

V = 60 MPH

TTST = 3% DUAL = 3%

FUNC CLASS =

RURAL LOCAL

SUB-REGIONAL TIER

PROFILE (HORIZONTAL)

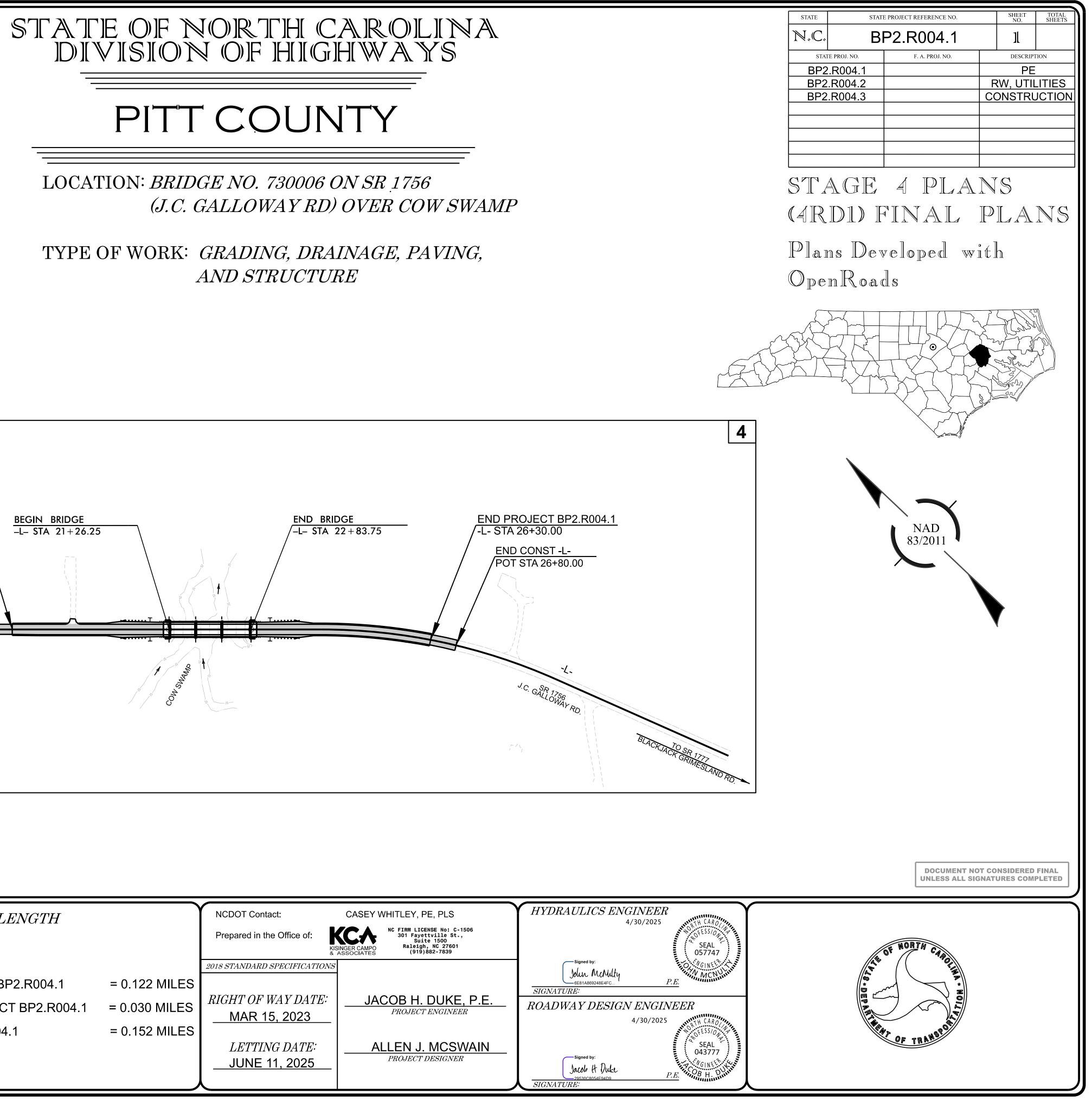
PROFILE (VERTICAL)

LENGTH OF ROADWAY PROJECT B LENGTH OF STRUCTURES PROJE TOTAL LENGTH PROJECT BP2.R004

SR 1756 J.C. GALLOWAY RD.



AND STRUCTURE



PROJECT LENGTH		KIS	CASEY WHITLEY, PE, PLS CASEY WHITLEY, PE, PLS NC FIRM LICENSE No: C-1506 301 Fayettville St., Suite 1500 Raleigh, NC 27601 (919)882-7839	HYDRAULICS ENGINE 4/30/202
AY PROJECT BP2.R004.1 TURES PROJECT BP2.R004.1 DJECT BP2.R004.1	= 0.122 MILES = 0.030 MILES = 0.152 MILES	2018 STANDARD SPECIFICATIONS RIGHT OF WAY DATE: MAR 15, 2023 LETTING DATE: JUNE 11, 2025	JACOB H. DUKE, P.E. PROJECT ENGINEER ALLEN J. MCSWAIN PROJECT DESIGNER	Signed by: John McMulty 6E81A869248E4FC SIGNATURE: ROADWAY DESIGN ENC 4/30/2 Signed by: Jacob H Duke 29530C8054F94D9 SIGNATURE:

INDE	X OF SHEETS	GENERAL N
SHEET NUMBER	SHEET	
1	TITLE SHEET	GRADING A
1A	INDEX OF SHEETS, GENERAL NOTES, AND STANDARD DRAWINGS	THE G
1B	CONVENTIONAL SYMBOLS	SURF/ ADJUS
2A-1	PAVEMENT SCHEDULE AND TYPICAL SECTIONS	ENGIN
3B-1	ROADWAY SUMMARIES	CLEARING:
3D-1	DRAINAGE SUMMARIES	CLEAI METH
3G-1	GEOTECHNICAL SUMMARIES	SUPERELE
4	PLAN SHEET	ALL C
5	PROFILE SHEET	STD. I SUPE
RW-01 THRU RW-05	5 RIGHT OF WAY PLANS	SECTI
EC-1 THRU EC-9	EROSION CONTROL PLANS	SHOULDER
RF-1	REFORESTATION PLANS	ASPH, SUPE
UO-1 THRU UO-2	UTILITIES BY OTHERS PLANS	SUBSURFA
X-1 THRU X-9	CROSS-SECTIONS	SUBS
S-1 THRU S-20	STRUCTURE PLANS	LOCA
		DRIVEWAY
		DRIVE USING

/EWAYS SHALL BE CONSTRUCTED IN ACCORDANCE WITH STD. 848.02 USING 3 FOOT RADII OR RADII AS SHOWN ON THE PLANS. LOCATIONS OF DRIVES WILL BE AS SHOWN ON THE PLANS OR AS DIRECTED BY THE ENGINEER.

THE GUARDRAIL LOCATIONS SHOWN ON THE PLANS MAY BE ADJUSTED DURING CONSTRUCTION AS DIRECTED BY THE ENGINEER. THE CONTRACTOR SHOULD CONSULT WITH THE ENGINEER PRIOR TO ORDERING GUARDRAIL MATERIAL

END BENTS:

THE ENGINEER SHALL CHECK THE STRUCTURE END BENT PLANS, DETAILS, AND CROSS-SECTION PRIOR TO SETTING OF THE SLOPE STAKES FOR THE EMBANKMENT OR EXCAVATION APPROACHING A BRIDGE.

UTILITIES:

UTILITY OWNERS ON THIS PROJECT ARE

Century Link

Greenville Utilities Commission

Eastern Pines Water Corp

RIGHT-OF-WAY MARKERS:

ALL RIGHT-OF-WAY MARKERS ON THIS PROJECT SHALL BE PLACED BY OTHERS.

NOTES:

2024 SPECIFICATIONS EFFECTIVE: January 2024 **REVISED**:

GAND SURFACING:

E GRADE LINES SHOWN DENOTE THE FINISHED ELEVATION OF THE PROPOSED RFACING AT GRADE POINTS SHOWN ON THE TYPICAL SECTIONS. GRADE LINES MAY BE USTED AT THEIR BEGINNING AND ENDING AND AT STRUCTURES AS DIRECTED BY THE SINEER IN ORDER TO SECURE A PROPER TIE-IN.

ARING ON THIS PROJECT SHALL BE PERFORMED TO THE LIMITS ESTABLISHED BY HOD II.

EVATION:

CURVES ON THIS PROJECT SHALL BE SUPERELEVATED IN ACCORDANCE WITH NO. 225.04 USING THE RATE OF SUPERELEVATION AND RUNOFF SHOWN ON THE PLANS. PERELEVATION IS TO BE REVOLVED ABOUT THE GRADE POINTS SHOWN ON THE TYPICAL TIONS.

ER CONSTRUCTION:

HALT, EARTH, AND CONCRETE SHOULDER CONSTRUCTION ON THE HIGH SIDE OF PERELEVATED CURVES SHALL BE IN ACCORDANCE WITH STD. NO. 560.01

FACE DRAINS:

BSURFACE DRAINS SHALL BE CONSTRUCTED IN ACCORDANCE WITH STD. NO. 815.02 AT CATIONS DIRECTED BY THE ENGINEER.

YS:

GUARDRAIL:

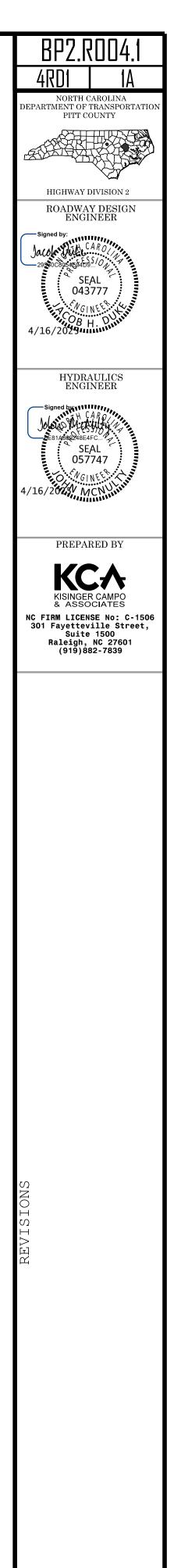
EFF. January 2024 REV.

The following Roadway Standards as appear in "Roadway Standard Drawings" Highway Design Branch - N. C. Department of Transportation - Raleigh, N. C., Dated January, 2018 are applicable to this project and by reference hereby are considered a part of these plans:

TITLE STD.NO. **DIVISION 2 - EARTHWORK** 200.03 Method of Clearing - Method III 225.02 Guide for Grading Subgrade - Secondary and Local 225.04 Method of Obtaining Superelevation - Two Lane Pavement DIVISION 3 - PIPE CULVERTS 300.01 Method of Pipe Installation 310.02 Parallel Pipe End Section - Precast Concrete Section for 15" to 24" Pipe **DIVISION 4 - MAJOR STRUCTURES** 423.01 Bridge Approach Fills - Type I Approach Fill for Bridge Abutment DIVISION 5 - SUBGRADE, BASES AND SHOULDERS 560.01 Method of Shoulder Construction - High Side of Superelevated Curve - Method I **DIVISION 8 - INCIDENTALS** 815.02 Subsurface Drain 840.00 Concrete Base Pad for Drainage Structures 840.25 Anchorage for Frames - Brick or Concrete or Precast 840.29 Frames and Narrow Slot Flat Grates 840.35 Traffic Bearing Grated Drop Inlet - for Double Frame and Grates 840.66 Drainage Structure Steps 846.01 Concrete Curb, Gutter and Curb & Gutter 846.04 Drop Inlet Installation in Shoulder Berm Gutter 862.01 Guardrail Placement 862.02 Guardrail Installation 862.03 Structure Anchor Units

2024 ROADWAY ENGLISH STANDARD DRAWINGS

876.02 Guide for Rip Rap at Pipe Outlets



Note: Not to Scale

BOUNDARIES AND PROPERTY:

State Line	
County Line	
Township Line —	
City Line ———	
Reservation Line	· · ·
Property Line ————	
Existing Iron Pin (EIP)	- <u>O</u> EIP
Computed Property Corner	- ×
Existing Concrete Monument (ECM)	_
Parcel / Sequence Numbe r	- (23)
Existing Fence Line	
Proposed Woven Wire Fence	· 0
Proposed Chain Link Fence	
Proposed Barbed Wire Fence	
Existing Wetland Boundary	- — — — WLB— — — —
Proposed Wetland Boundary	WLB
Existing Endangered Animal Boundary	EAB
Existing Endangered Plant Boundary ———	ЕРВ
Existing Historic Property Boundary	нрв
Known Contamination Area: Soil	·- 😿 — s — 😿 — s —
Potential Contamination Area: Soil	·- X - s - X - s -
Known Contamination Area: Water	·-∞-w-∞-w-
Potential Contamination Area: Water	·- X w M w-
Contaminated Site: Known or Potential	
BUILDINGS AND OTHER CULTU	7 RE:
Gas Pump Vent or U/G Tank Cap	- O
Sign —	- O s
Well	- 🖓
Small Mine —	- X
Foundation —	
Area Outline	
Cemetery	
Building —	

Church Dam -

HYDROLOGY:

School

JS •••
——— BZ 1 ———
——— BZ 2 ———
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RAILROADS:

Standard Gauge	CSX TRANSPORTATION
RR Signal Milepost	⊙ MILEPOST 35
Switch —	Switch
RR Abandoned	+++_
RR Dismantled	

(Rebar and Cap) (Concrete) \Diamond \bigotimes (Rebar and Cap) \wedge A \bigcirc _____(<u>Ĉ</u>)_____

RIGHT OF WAY & PROJECT CONTROL: Primary Horiz Control Point Primary Horiz and Vert Control Point ——— Secondary Horiz and Vert Control Point — Vertical Benchmark — Existing Right of Way Monument— Proposed Right of Way Monument — Proposed Right of Way Monument— Existing Permanent Easement Monument Proposed Permanent Easement Monument Existing C/A Monument —— Proposed C/A Monument (Rebar and Cap) — Proposed C/A Monument (Concrete) Existing Right of Way Line -Proposed Right of Way Line-Existing Control of Access Line— Proposed Control of Access Line-Proposed ROW and CA Line — Existing Easement Line-Proposed Temporary Construction Easement Proposed Permanent Drainage/Utility Easement _______ Proposed Permanent Utility Easement _____ PUE _____ Proposed Temporary Utility Easement ______ _____ Proposed Aerial Utility Easement _____ ____ ROADS AND RELATED FEATURES: Existing Edge of Pavement _____ ____ Existing Curb —————

Proposed Slope Stakes Cut **Proposed Slope Stakes Fill** Proposed Curb Ramp — Existing Metal Guardrail — Proposed Guardrail —— Existing Cable Guiderail Proposed Cable Guiderail Equality Symbol Pavement Removal **VEGETATION:**

Single Tree	
Single Shrub	
Hedge ——	

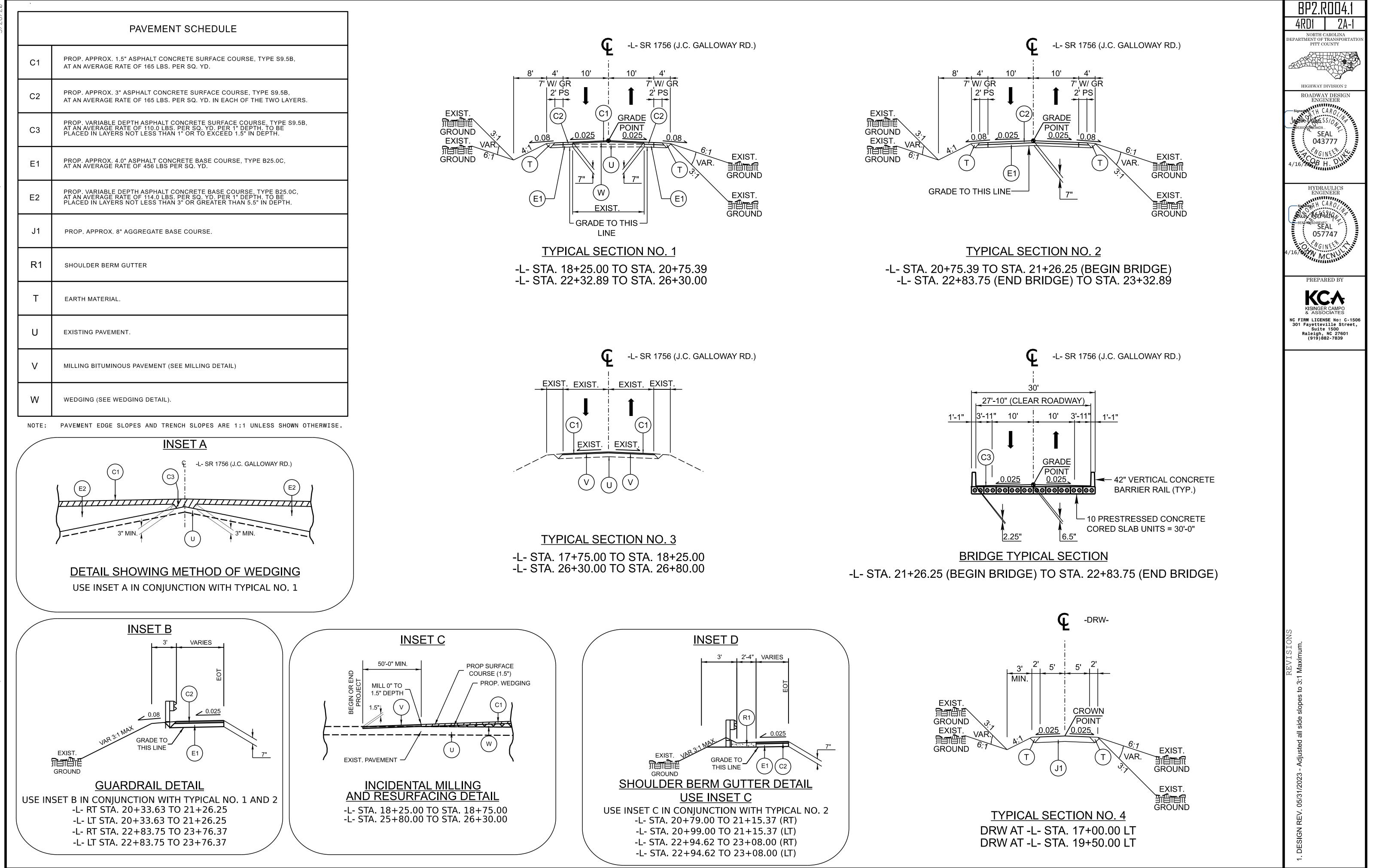
STATE OF NORTH CAROLINA, DIVISION OF HIGHWAYS CONVENTIONAL PLAN SHEET SYMBOLS

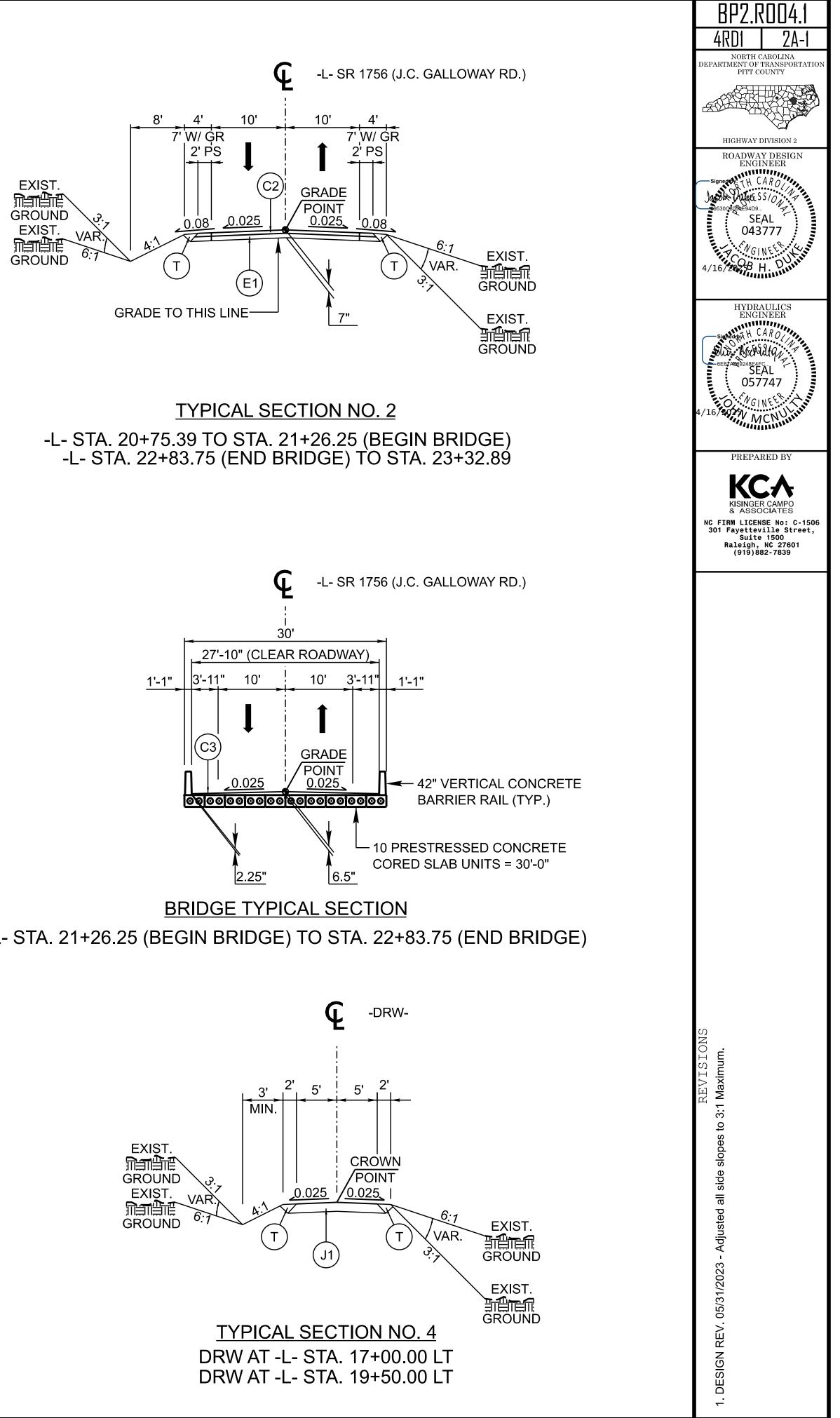
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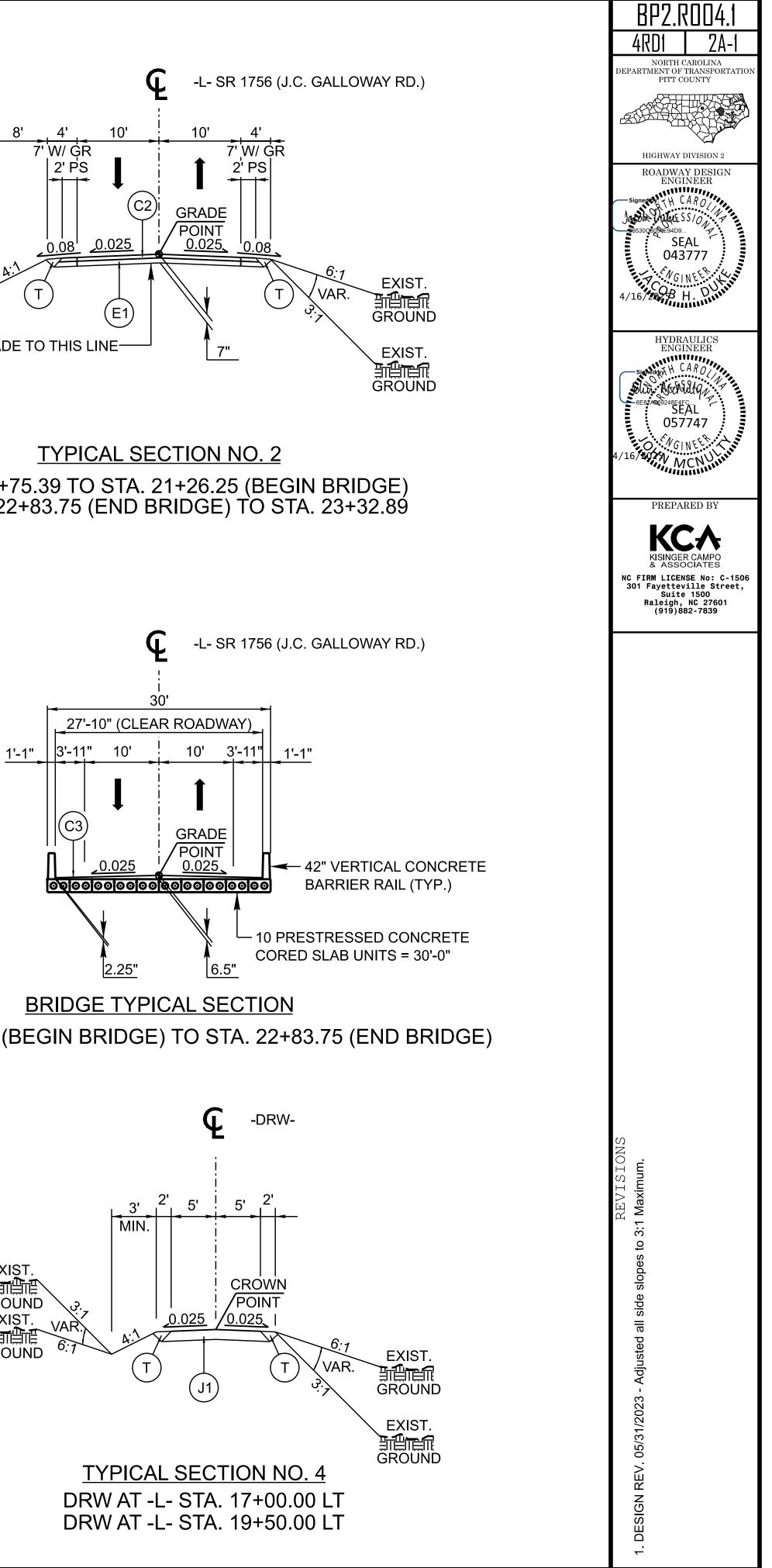
Woods Line	
Orchard	- & & & & &
Vineyard	- Vineyard
EXISTING STRUCTURES:	
MAJOR:	
Bridge, Tunnel or Box Culvert	CONC
Bridge Wing Wall, Head Wall and End Wall - MINOR:	-) CONC WW (
Head and End Wall	CONC HW
Pipe Culvert	
Footbridge —	≻≺
Drainage Box: Catch Basin, DI or JB	СВ
Paved Ditch Gutter	
Storm Sewer Manhole	S
Storm Sewer	s
UTILITIES:	
* SUE - Subsurface Utility Engineering LOS - Level of Service - A,B,C or D (A POWER:	Accuracy)
Existing Power Pole	•
Proposed Power Pole	-
Existing Joint Use Pole	
Proposed Joint Use Pole	
Power Manhole —	•
Power Line Tower —	
Power Transformer	
U/G Power Cable Hand Hole ———	
H-Frame Pole	
U/G Power Line Test Hole (SUE - LOS A)* — U/G Power Line (SUE - LOS B)* ———	
U/G Power Line (SUE - LOS C)*	P
U/G Power Line (SUE - LOS D)*	
TELEPHONE:	•
Existing Telephone Pole	
Proposed Telephone Pole	-0-
Telephone Manhole	
Telephone Pedestal	
Telephone Cell Tower	
U/G Telephone Cable Hand Hole	
U/G Telephone Test Hole (SUE - LOS A)* — U/G Telephone Cable (SUE - LOS B)* ——	
U/G Telephone Cable (SUE - LOS C)*	
U/G Telephone Cable (SUE - LOS D)*	
U/G Telephone Conduit (SUE - LOS B)*	
U/G Telephone Conduit (SUE - LOS C)*	
U/G Telephone Conduit (SUE - LOS D)*	
U/G Fiber Optics Cable (SUE - LOS B)* ——	
U/G Fiber Optics Cable (SUE - LOS C)* ——	
U/G Fiber Optics Cable (SUE - LOS D)*	T F0

	<u>41(D1 10</u>
WATER:	
Water Manhole ————	\mathfrak{W}
Water Meter	0
Water Valve	\otimes
Water Hydrant ———	¢
U/G Water Line Test Hole (SUE - LOS A)* —	٢
U/G Water Line (SUE - LOS B)*	
U/G Water Line (SUE - LOS C)*	
U/G Water Line (SUE - LOS D)*	
Above Ground Water Line	A/G Water
TV:	_
TV Pedestal	
TV Tower —	\otimes
U/G TV Cable Hand Hole	нн
U/G TV Test Hole (SUE - LOS A)*	
U/G TV Cable (SUE - LOS B)*	— — — TV— — — -
U/G TV Cable (SUE - LOS C)*	Tv
U/G TV Cable (SUE - LOS D)*	Tv
U/G Fiber Optic Cable (SUE - LOS B)* ——	— — — TV FO— — —
U/G Fiber Optic Cable (SUE - LOS C)* ——	— _TV FO—
U/G Fiber Optic Cable (SUE - LOS D)* ——	
GAS:	
Gas Valve	\diamond
Gas Meter	\diamond
U/G Gas Line Test Hole (SUE - LOS A)* —	٢
U/G Gas Line (SUE - LOS B)*	c
U/G Gas Line (SUE - LOS C)*	
U/G Gas Line (SUE - LOS D)*	
Above Ground Gas Line	A/G Gas
SANITARY SEWER:	
Sanitary Sewer Manhole	⊕
Sanitary Sewer Cleanout	÷
U/G Sanitary Sewer Line	
Above Ground Sanitary Sewer	A/G Sanitary Sewer
SS Force Main Line Test Hole (SUE - LOS A)*	
	— — — — FSS — — — -
SS Force Main Line (SUE - LOS C)*	
SS Force Main Line (SUE - LOS D)*	FSS
MISCELLANEOUS:	
Utility Pole	•
Utility Pole with Base	·
Utility Located Object	\odot
Utility Traffic Signal Box	S
Utility Unknown U/G Line (SUE - LOS B)* —	7UTL
U/G Tank; Water, Gas, Oil	
Underground Storage Tank, Approx. Loc. ——	(UST)
A/G Tank; Water, Gas, Oil	
Geoenvironmental Boring	
Abandoned According to Utility Records —	AATUR
End of Information	E.O.I.

Docusign Envelope ID: 5AFCAF88-B8D6-4DAB-96DB-15B94E4DE0D3







SUMMARY OF EARTHWORK

IN CUBIC YARDS

Station	Station	Uncl. Excav.	Embank. +%	Borrow	Waste
-L- 18+25.00	-L- 21+26 .25	67	243	176	
Bri	dge				
-L- 22+83.75	-L- 26+30.00	37	216	179	
PROJECT	TOTALS:	104	459	355	
Waste in Lie	u of Borrow				
Replace Topsoil o	on Borrow Pit (5%)			18	
GRAND TOTALS:		104	459	373	
SA	Y:	110		380	

NOTE:

APPROXIMATE QUANTITIES ONLY. UNCLASIFIED EXCAVATION, BORROW EXCAVATION, FINE GRADING, CLEARING AND GRUBBING, AND REMOVAL OF EXISTING PAVEMENT WILL BE PAID FOR BY THE CONTRACT LUMP SUM PRICE FOR "GRADING"

ALL EARTHWORK QUANTITIES WERE DERIVED FROM ORD QUANTITIES BY NAMED BOUNDARY REPORT(S) AS DESCRIBED IN THE ORD QUICKSTART TRAINING.

	RIGHT OF WAY AREA DATA						
PARCEL NO.	PROPERTY OWNER NAMES	TOTAL ACREAGE	AREA TAKEN	AREA REMAINING	R/W	CONST. EASE.	DRAIN UTIL. EASE
1	GEORGE SUTTON LIFE ESTATE						
2	ANDRE GOMEZ HERNANDEZ					0.0176	
3	LEON RAYMOND HARDEE, JR.					0.0335	
4	GRIMES BUILT CONSTRUCTION, LLC					0.0765	
5	LINDA IPOCK HARDEE LIVING TRUST C/O LEON RAYMOND HARDEE					0.0541	
6	KENSIL JOHN EVANS					0.0723	

"N" = DISTANCE FROM EDGE OF LANE TO FACE OF GUARDRAIL TOTAL SHOULDER WIDTH = DISTANCE FROM EDGE OF TRAVEL LANE TO SHOULDER BREAK POINT. FLARE LENGTH = DISTANCE FROM LAST SECTION OF PARALLEL GUARDRAIL TO END OF GUARDRAIL W = TOTAL WIDTH OF FLARE FROM BEGINNING OF TAPER TO END OF GUARD RAIL LENGTH WARRANT POINT "N" TOTAL FLARE SURVEY DIST. SHOUL APPROAC SHOP DOUBLE APPROACH TRAILING BEG. STA. LOCATION END STA. LINE CURVED FACED FROM WIDTH END STRAIGHT END END 21+26.25 3'-11" 6'-11" 20+32.50 21+26.25 LT 93.75 -L-20+32.50 21+26.25 RT 93.75 21+26.25 3'-11" 6'-11" 75 -L-22+83.75 3'-11" 6'-11" 23+77.50 22+83.75 93.75 -L-LT 75 -L-22+83.75 23+77.50 RT 93.75 22+83.75 3'-11" 6'-11" SUBTOTAL: 375.00 Less GREU TL-3 @ 50' Each 200 Less Type III @ 18.75 Each 75 PROJECT TOTALS: 100.00 SAY: 100.00

SHOULDER BERM GUTTER SUMMARY IN LINEAR FEET

LINE	STATION	STATION	LENGTH
-L- (RT)	20+78.93	21+15.36	36.43
-L- (LT)	20+99.42	21+15.36	15.94
-L- (RT)	22+94.63	23+08.78	14.15
-L- (LT)	22+94.63	23+08.76	14.13
		TOTAL:	80.65
		SAY:	81

PERM. PERM. TEMP. IN. DRAIN. UTIL. UTIL. EASE. EASE. EASE. 0.0064 0.0074 0.0060 0.0073

PAVEMENT REMOVAL SUMMARY

IN SQUARE YARDS

			14				
SURVEY LINE	Station	Station	LOCATION LT/RT/CL	ASPHALT REMOVAL	ASPHALT BREAKUP	CONCRETE REMOVAL	CONCRETE BREAKUP
-L-	20+75.39	21+26.25	CL	135.63			
-L-	22+83.75	23+32.89	CL	131.04			
		TOTAL:		266.67			
		SAY:		270.00			

		GUAI	RDR.	4 <i>IL</i>	S	UM	(M	AR)	Y										G = GATING IMPACT ATTENUATOR TYPE 350 NG = NON-GATING IMPACT ATTENUATOR TYPE 350
ARE L	ENGTH	w						ANCHORS	\$							SINGLE	REMOVE	REMOVE &	
DACH ID	TRAILING END	APPROACH END	TRAILING END	XI MOD	хі	GREU TL-3	M-350	TYPE III	CAT-1	VI MOD	BIC	AT-1	ADDITIONAL GUARDRAIL POSTS	G	NG	FACED CONCRETE	EXISTING GUARDRAIL	STOCKPILE EXISTING	REMARKS
	75		1			1		1											
5		1				1		1											
5		1				1		1											
	75		1			1		1											
						4		4					5						
						4		4					5						
						4		4					5						

BP2.R004.1 4RD1 3B-1 NORTH CAROLINA EPARTMENT OF TRANSPORTATION PITT COUNTY HIGHWAY DIVISION 2 PREPARED BY KCA KISINGER CAMPO & ASSOCIATES NC FIRM LICENSE No: C-1506 301 Fayetteville Street, Suite 1500 Raleigh, NC 27601 (919)882-7839

S

Note: Invert Elevations indicated are for Bid Purposes only and shall not be used for project construction stakeout.

LIST OF PIPES, ENDWALLS, ETC. (FOR PIPES 48 INCHES & UNDER)

STATION	LOCATION (LT, RT, OR CL)'	STRUCTURE NO		TOP ELEVATION	INVERT ELEVATION	INVERT ELEVATION	SLOPE CRITICAL			(F	RCP,	D CSP,			PIPE		C)						C.S.	PIPE	:				
SIZE	LOCATIO							12''	15''	18''	24''	30''	36''	42''	48''	СР	SP	AP	PE	12''	15''	18''	24''	30''	36''	42''	48''	12''	Ī
THICKNESS OR GAUGE		FROM	TO													DO NOT USE RCP	DO NOT USE CSP	DO NOT USE CAAP	DO NOT USE HDPE	.064	.064	.064	.064	670.	670.	.109	.109		
21+03 -L-	12.96 LT	401		25.9																									
	12.96 LT	401	402		22.4	19.4			24							X													T
20+83 -L-	12.95 RT	403		25.7																									Ī
	12.95 RT	403	404		22.2	20.4			24							Х													Ī
23+05 -L-	12.89 LT	405		25.8																									
	12.89 LT	405	406		23.0	20.2			20							X													
23+05 -L-	12.89 RT	407		25.8																									L
	12.89 RT	407	408		23.0	18.8			24							X													╞
SHEET TOTALS							┢		92																				┢

NORTH CAROLINA DEPARTMENT OF TRANSPORTATION DIVISION OF HIGHWAYS

			C. PIF Ass I								C. PIF Ass I					RACTOR DESIGN	CONTRACTOR DESIGN			REINFO ENDW/ STD. 8 838.1 ⁴ STD. 8 (UNL) NOT OTHERI	ALLS 38.01 1 OR 38.80 ESS ED	QUANTITIES FOR DRAINAGE		<pre>*TOTAL L.F. FOR PAY QUANTITY SHALL BE COL. 'A' + (1.3 X COL.'B')</pre>	2	GF ANI STA	RAMI RATE D HO MDA 340.03	S, OD RD	CONCRETE TRANSITIONAL	SECTION		G.D.I. (N.S. FLAT) FRAME W/ 2 GRATES STD 840.29		
																ONTF	ONTF				,			FT.	40.02							W 2 0	15"	2
15''	18''	24''	30''	36''	42''	48''	12''	15''	18''	24''	30''	36''	42''	48''	(۸ s	TS, C(TS, C(CU. YA	RDS	5.0')	Α	В	STD. 840.02							AME	SMOR	
															" R.C. PIPE (CLASS V)	' RC PIPE CULVERTS, CONTRACTOR DESIGN	' RC PIPE CULVERTS,	15" SIDE DRAIN PIPE	18" SIDE DRAIN PIPE	R.C.P.	C.S.P.	R EACH (0' THRU 5.0')	5.0' THRU 10.0'	10.0' AND ABOVE	STD. 840.01 OR		'PE C GRATI		DROP INLET	CATCH BASIN	T.B.D.I STD 840.35	D.I. (N.S. FLAT) FR	DRAINAGE PIPE EI BOWS 15"	
															24	***		15'	18'			PER	5.0	10.	C.B.	E	F	G	DR	CA		ິບ		
																						1									1	1	2	2
																															4	_		_
																						1									1	1	2	
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																						1									1	1	2	2
																						4									4	4	8	}

				ABBREVIATIONS
CONC & RDICK DIDE DI LIG C V STD 840.74	CONC. COLLARS CL. "B" C.Y. STD. 840.72	PIPE REMOVAL LIN. FT.	C.B. N.D.I. D.I. G.D.I. G.D.I.(N.S.) J.B. M.H. T.B.D.I. T.B.J.B.	CATCH BASIN NARROW DROP INLET DROP INLET GRATED DROP INLET (NARROW SLOT) JUNCTION BOX MANHOLE TRAFFIC BEARING DROP INLET TRAFFIC BEARING JUNCTION BOX
	CON	PIPE		REMARKS

BP2.R004.1

NORTH CAROLINA EPARTMENT OF TRANSPORTATION PITT COUNTY

HIGHWAY DIVISION 2 PREPARED BY

KCA

KISINGER CAMPO & ASSOCIATES

NC FIRM LICENSE No: C-1506 301 Fayetteville Street, Suite 1500 Raleigh, NC 27601 (919)882-7839

3D-1

4RD1

COMPUTED BY: Tyler C. Bottoms DATE: 3/23/2022 CHECKED BY: Thein Tun Zan DATE: 5/1/2023	

(2-3-23)

STATE OF NORTH CAROLINA DIVISION OF HIGHWAYS

SUMMARY OF SUBSURFACE DRAINAGE

LINE	Station	Station	Location LT/RT/CL	Drain Type* UD/BD/SD	LF
	CONTING	GENCY	-	SD	200
				TOTAL LF:	200

*UD = Underdrain *BD = Blind Drain

*SD = Subsurface Drain

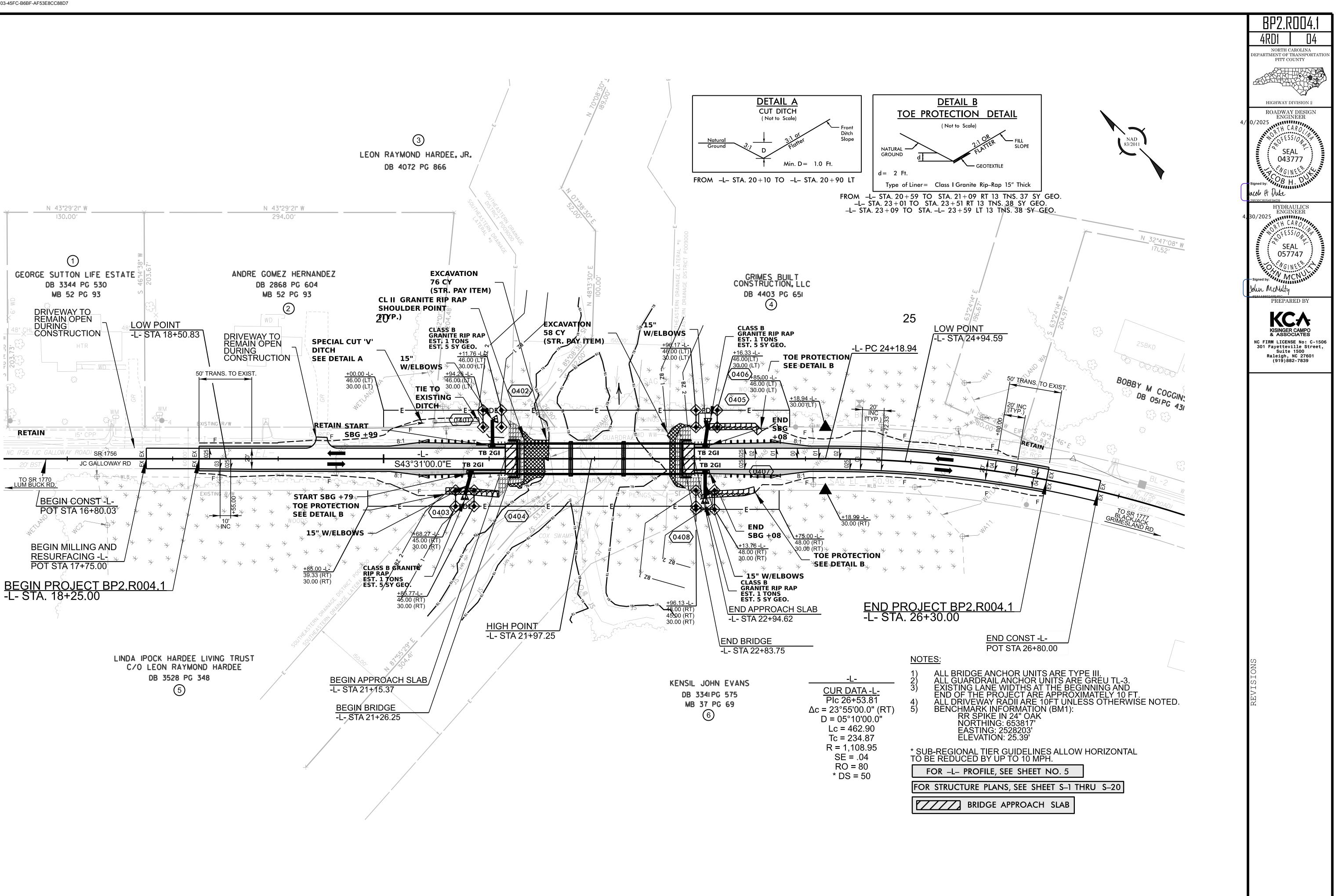
			BP2.R004.1
			4RD1 3G-1
PROJECT NO.	SHEET NO.		NORTH CAROLINA DEPARTMENT OF TRANSPORTATION PITT COUNTY
BP2.R004.1 (SF-730006)	3G-1		
			- Alana
			HIGHWAY DIVISION 2 PREPARED BY
			КС∧
			KISINGER CAMPO & ASSOCIATES NC FIRM LICENSE No: C-1506
			301 Fayetteville Street, Suite 1500 Raleigh, NC 27601 (919)882-7839
			(919)882-7839
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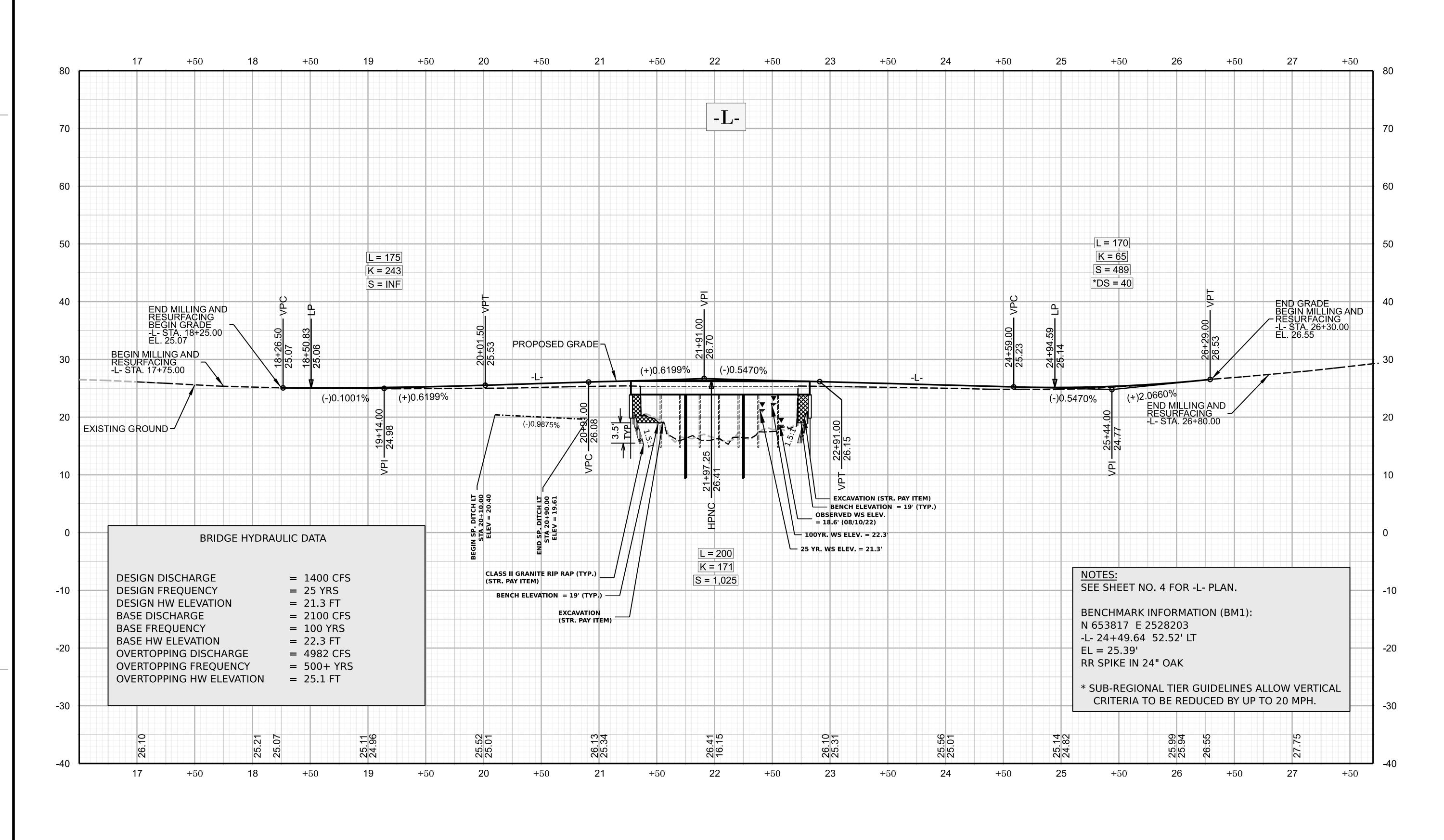
130.00'

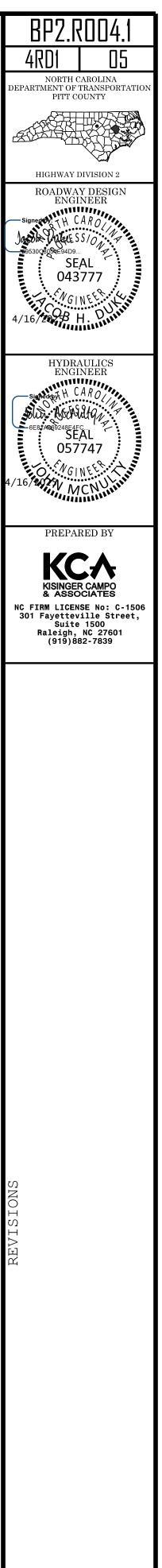
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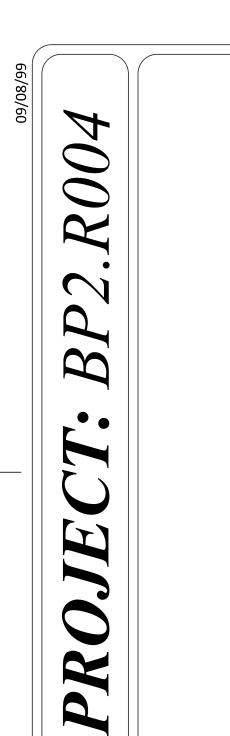
RETAIN

TO SR 1770 LUM BUCK RD.



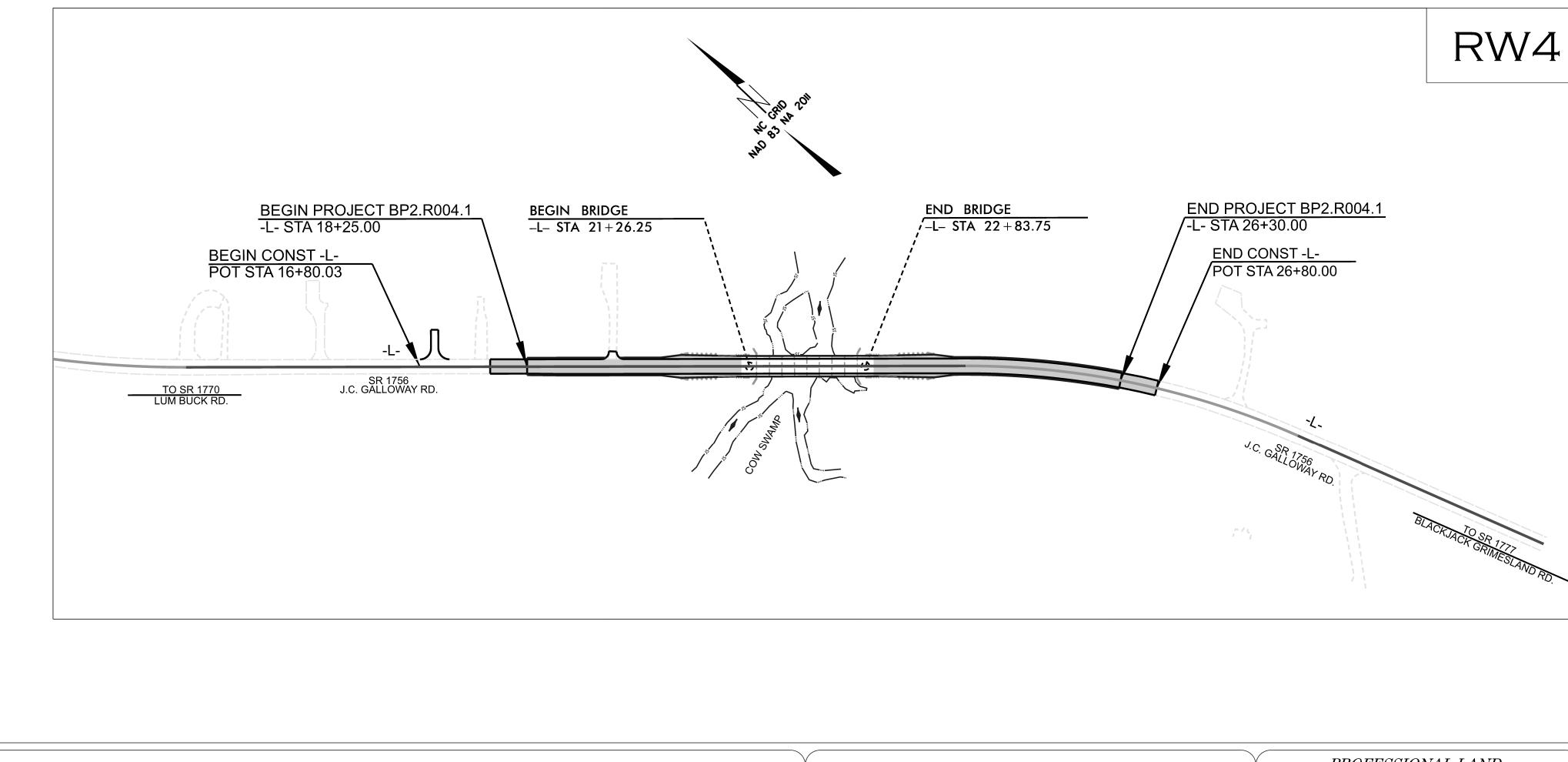


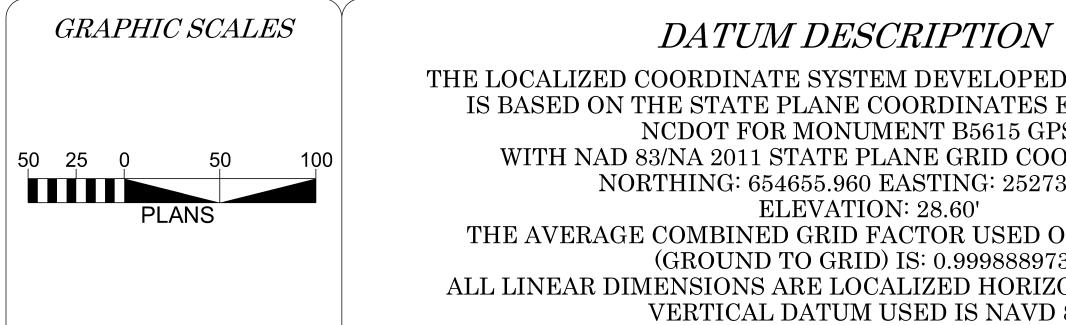




TIP







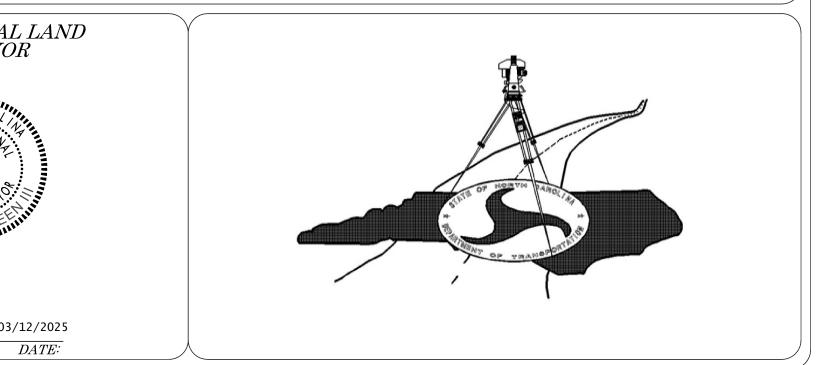
STATE OF NORTH CAROLINA DIVISION OF HIGHWAYS

PITT COUNTY

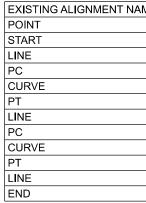
	Prepared in th	e Office of:	PRC	DFESSIONAT SURVEYO
) FOR THIS PROJECT ESTABLISHED BY	Divisio Location and			HOR CESS 10
S-2 ORDINATES OF 315.059	2708 Neu Office New Bern, I	No.7		SEAL L-5082
ON THIS PROJECT 30	2018 STANDARD	SPECIFICATIONS	_	
ONTAL DISTANCES 88	<i>RIGHT OF WAY DATE:</i> <u>MARCH 15, 2023</u>	<i>LETTING DATE:</i> <i>FEBRUARY 2, 2029</i>		03,

STATE	STATE PROJECT REFERENCE NO.	SHEET NO.	TOTAL SHEETS
$\mathbb{N}.\mathbb{C}.$	BP2.Roog	RWOI	6

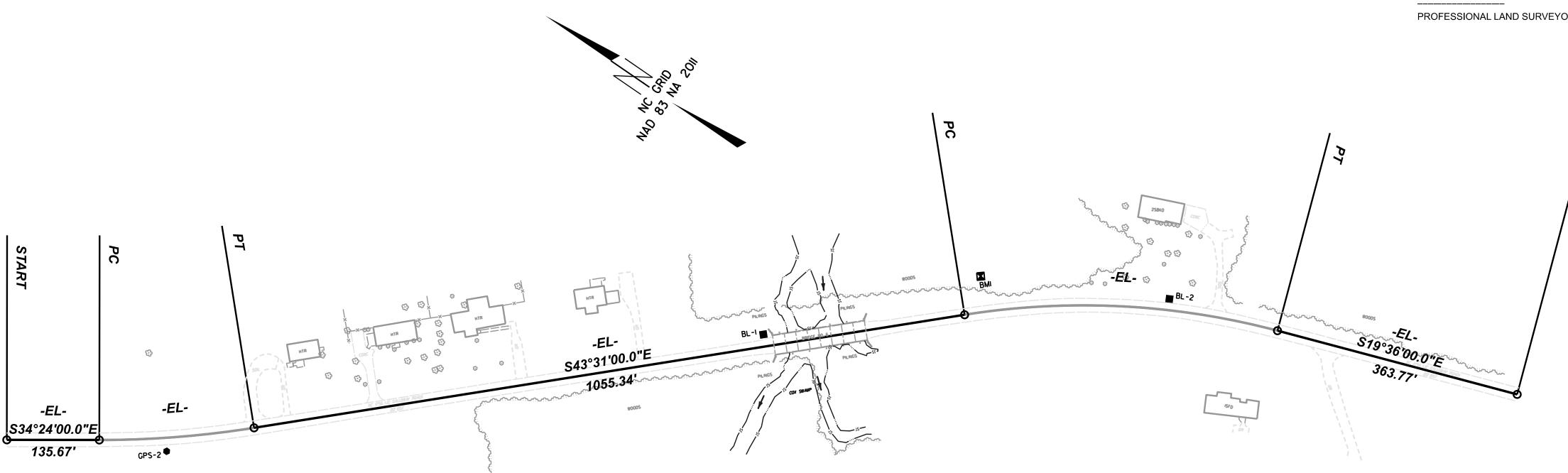




SURVEY CONTROL SHEET W/EXISTING CENTERLINE ALIGNMENTS PRIOR TO CONSTRUCTION



Control				
POINT	DESC	NORTH	EAST	ELEVATION
BL-1	4020	654031.390	2527951.756	24.78'
BL-2	4020	653569.845	2528331.753	28.02'
GPS-1	4017	655679.641	2526751.316	35.42'
GPS-2	4017	654655.960	2527315.059	28.60'



50 25 0 50

AME: EL									
	NORTHING	EASTING	BEARING	DIST	DELTA	D	L	Т	R
	654858.737	2527196.196							
			S34°24'00"E	135.674					
	654746.791	2527272.847							
					09°07'00" Left	04°00'00"	227.917	114.199	1432.394
	654569.749	2527415.999							
			S43°31'00"E	1055.345					
	653804.440	2528142.673							
					23°55'00" Right	05°10'00"	462.903	234.872	1108.951
	653412.854	2528383.186							
			S19°36'00"E	363.772					
	653070.160	2528505.214							

BENCHMARK TABLE				
BENCHMARK	NORTHING	EASTING	ELEVATION	DESCRIPTION
BM1	653816.981	2528202.576	25.39'	RR SPIKE SET IN 24" OAK

NOTES:

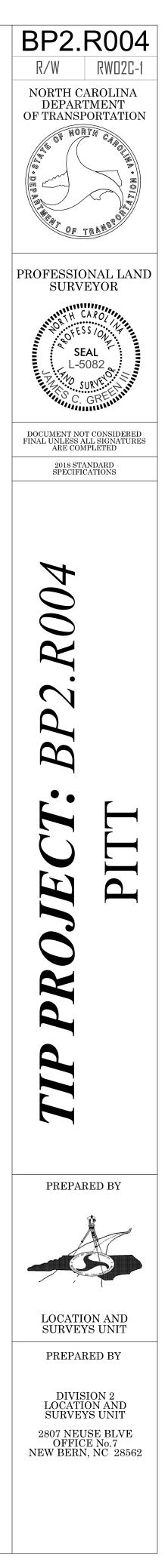
1. PROJECT CONTROL WAS ESTABLISHED USING GNSS, THE GLOBAL NAVIGATION SATELLITE SYSTEM. 2. THE PROPOSED ALIGNMENT CONTROL DATA FOR THIS PROJECT HAS BEEN COMPILED FROM VARIOUS SOURCES. IF FURTHER INFORMATINO REGARDING PROJECT CONTROL IS NEEDED, PLEASE CONTACT THE LOCATION AND SURVEYS UNIT.

I, JAMES C. GREEN, III, PLS, CERTIFY THAT THE PROJECT CONTROL WAS VERIFIED UNDER MY SUPERVISION FROM AN ACTUAL GPS SURVEY MADE UNDER MY SUPERVISION AND THE FOLLOWING INFORMATION WAS USED TO PERFORM THE SURVEY:

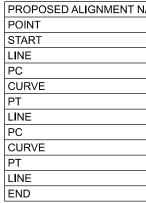
CLASS OF SURVEY: AA TYPE OF GPS FIELD PROCEDURE: RTN DATES OF SURVEY: JANUARY 2017 DATUM/EPOCH: NAD 83/NA 2011 PUBLISHED/FIXED-CONTROL USE: N/A LOCALIZED AROUND: GPS-2 NORTHING: 654655.960 EASTING: 2527315.059 COMBINED GRID FACTOR: 0.9998889730 GEOID MODEL: G12NC UNITS: US SURVEY FOOT

I ALSO CERTIFY THAT THE BASELINE CONTROL FOR THIS PROJECT WAS VERIFIED UNDER MY DIRECT AND RESPONSIBLE CHARGE FROM AN ACTUAL SURVEY MADE UNDER MY SUPERVISION; THAT ALL HORIZONTAL CLOSURES HAD A MINIMUM RATIO OF PRECISION OF 1:20,000 (CLASS AA) AND VERTICAL ACCURACY TO CLASS A. FIELD WORK WAS PERFORMED FROM JANUARY 2017, AND ALL COORDINATES ARE BASED ON NAD 83/NA 2011 AND ALL ELEVATIONS ARE BASED ON NAVD 88; THAT THIS SURVEY WAS PERFORMED TO MEET THE REQUIREMENTS OF 21NCAC 56.1600 AS APPLICABLE.

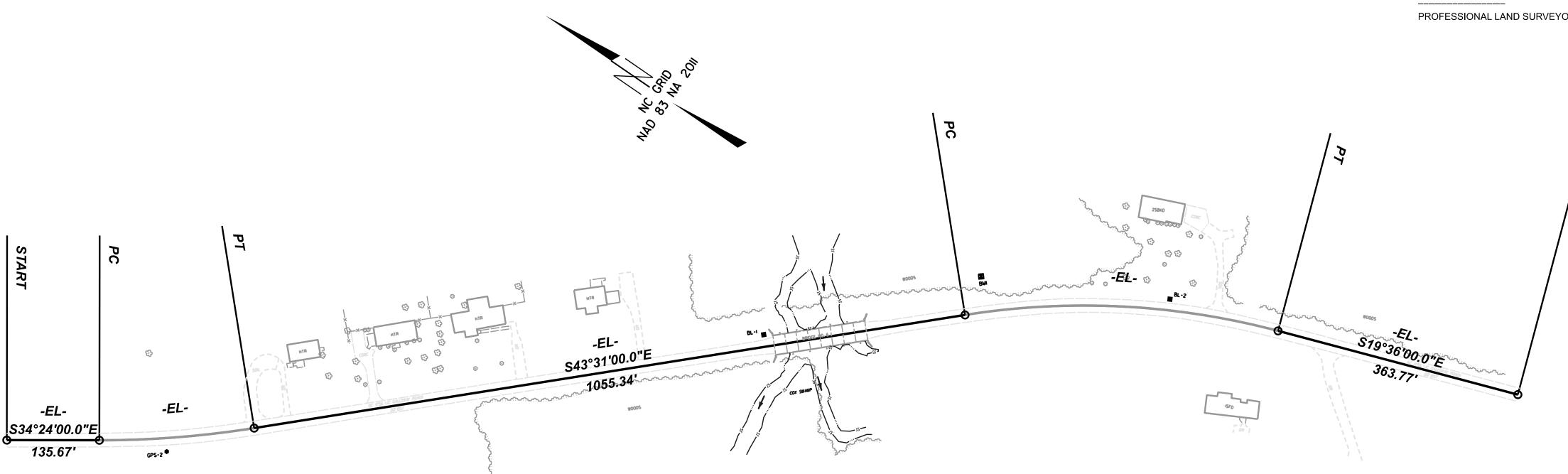
THIS 28 DAY OF AUGUST, 2023.



SURVEY CONTROL SHEET W/EXISTING CENTERLINE ALIGNMENTS PRIOR TO CONSTRUCTION



XX				
POINT	DESC	NORTH	EAST	ELEVATION
GPS-1	4017	654031.390	2527951.756	35.42'
GPS-2	4017	653569.845	2528331.753	28.60'
BL-1	4020	655679.641	2526751.316	24.78'
BL-2	4020	654655.960	2527315.059	28.02'



50 25 0 50

NAME:L									
	NORTHING	EASTING	BEARING	DIST	DELTA	D	L	Т	R
	654858.737	2527196.196							
			S34°24'00"E	135.674					
	654746.791	2527272.847							
					09°07'00" Left	04°00'00"	227.917	114.199	1432.394
	654569.749	2527415.999							
			S43°31'00"E	1055.345					
	653804.440	2528142.673							
					23°55'00" Right	05°10'00"	462.903	234.872	1108.951
	653412.854	2528383.186							
			S19°36'00"E	363.772					
	653070.160	2528505.214							

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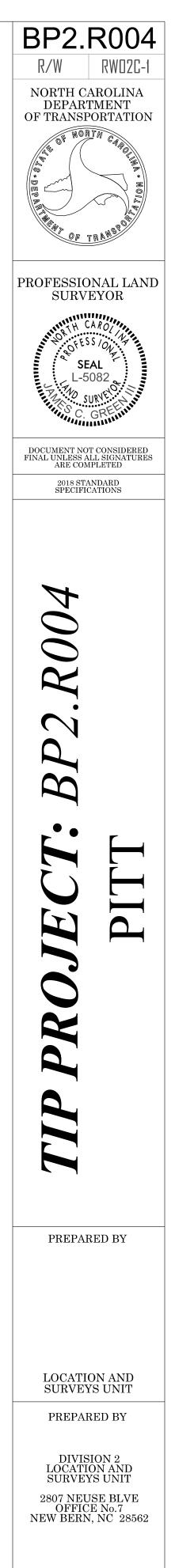
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CLASS OF SURVEY: AA TYPE OF GPS FIELD PROCEDURE: RTN DATES OF SURVEY: JANUARY 2017 DATUM/EPOCH: NAD 83/NA 2011 PUBLISHED/FIXED-CONTROL USE: N/A LOCALIZED AROUND: GPS-2 NORTHING: 654655.960 EASTING: 2527315.059 COMBINED GRID FACTOR: 0.9998889730 GEOID MODEL: G12NC UNITS: US SURVEY FOOT

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THIS 28 DAY OF AUGUST, 2023.



PROPOSED ALIGNMENT NAME:L									
POINT	NORTHING	EASTING	BEARING	DIST	DELTA	D	L	Т	R
START	654858.737	2527196.196							
LINE			S34°24'00"E	135.674					
PC	654746.791	2527272.847							
CURVE					09°07'00" Left	04°00'00"	227.917	114.199	1432.394
PT	654569.749	2527415.999							
LINE			S43°31'00"E	1055.345					
PC	653804.440	2528142.673							
CURVE					23°55'00" Right	05°10'00"	462.903	234.872	1108.951
PT	653412.854	2528383.186							
LINE			S19°36'00"E	363.772					
END	653070.160	2528505.214							

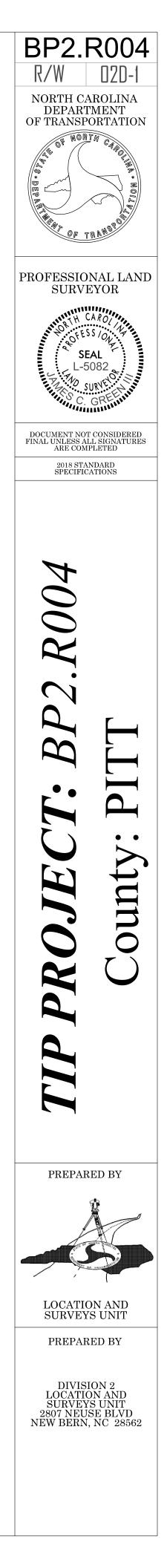
PROPOSED ALIGNMENT CONTROL SHEET

NOTES:

1. PROJECT CONTROL WAS ESTABLISHED USING GNSS, THE GLOBAL NAVIGATION SATELLITE SYSTEM. 2. THE PROPOSED ALIGNMENT CONTROL DATA FOR THIS PROJECT HAS BEEN COMPILED FROM VARIOUS SOURCES. IF FURTHER INFORMATINO REGARDING PROJECT CONTROL IS NEEDED, PLEASE CONTACT THE LOCATION AND SURVEYS UNIT.

I, JAMES C. GREEN,III, PLS, CERTIFY THAT THE DATA COMPILED CAME FROM AVAILABLE SURVEYS/MAPPING PERFORMED BY OTHERS AND PROVIDED TO ME BY NCDOT AND DO NOT CERTIFY TO THE ACCURACY OR QUALITY OF THE INDIVIDUAL DATA SOURCES.

THIS 15 DAY OF AUGUST, 2023.



PERMANENT ROW MARKER IRON PIN AND CAP: -L-			
STATION	OFFSET	NORTH	EAST
24+18.94	30.000	653783.783	2528120.918
24+18.94	-30.000	653825.097	2528164.429

RIGHT OF WAY CONTROL SHEET

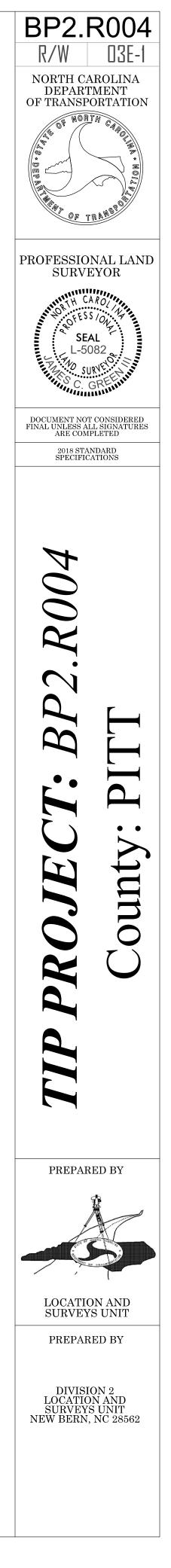
PERMANENT EASEMENT MARKER IRON PIN AND CAP: -L-			
STATION	OFFSET	NORTH	EAST
20+68.27	45.000	654027.747	2527868.586
20+68.27	30.000	654038.075	2527879.464
20+85.77	45.000	654015.056	2527880.636
20+85.77	30.000	654025.385	2527891.513
20+94.26	-46.001	654071.563	2527952.470
20+94.26	-30.000	654060.546	2527940.866
21+11.76	-30.000	654047.855	2527952.916
21+11.76	-46.001	654058.873	2527964.520
22+96.13	48.000	653860.444	2528023.306 NOT SET (UNDER WATER)
22+96.13	45.000	653862.510	2528025.481 NOT SET (UNDER WATER)_
22+96.13	30.000	653872.838	2528036.359 NOT SET (UNDER WATER)
22+96.17	-46.001	653925.138	2528091.503
22+96.17	-30.000	653914.120	2528079.900
23+13.76	30.000	653860.057	2528048.495 NOT SET (UNDER WATER)
23+13.76	48.000	653847.663	2528035.442 NOT SET (UNDER WATER)
23+16.33	-46.001	653910.524	2528105.379
23+16.33	-29.955	653899.475	2528093.744

NOTES:

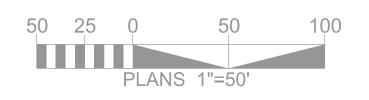
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I , JAMES C. GREEN, III , CERTIFY THAT THE RIGHT OF WAY AND PERMANENT EASEMENT MONUMENTATION FOR THIS PROJECT SHOWN HEREIN WAS COMPLETED UNDER MY DIRECT AND RESPONSIBLE CHARGE FROM AN ACTUAL SURVEY MADE UNDER MY SUPERVISION; THAT ALL HORIZONTAL CLOSURES HAD A MINIMUM RATIO OF PRECISION OF 1:10,000 (CLASS A). FIELD WORK WAS PERFORMED ON AUGUST 7th, 2023, AND ALL COORDINATES ARE BASED ON NAD83/2011; THAT THIS SURVEY WAS PERFORMED TO MEET THE REQUIREMENTS OF 21NCAC 56.1600 AS APPLICABLE.

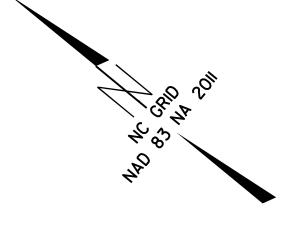
THIS 15th DAY OF AUGUST, 2023.







004



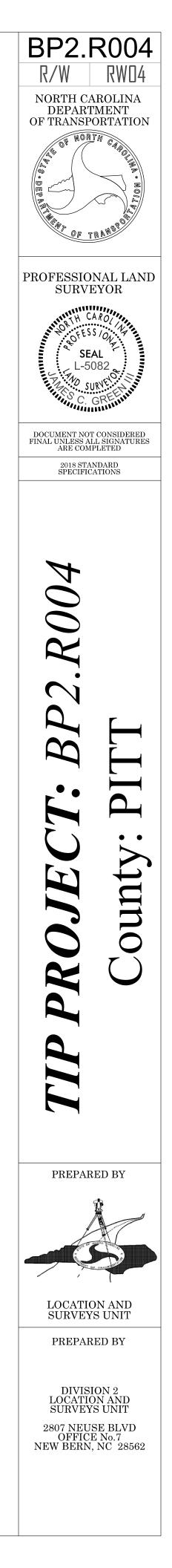
LEON RAYMOND HARDEE, JR. DB 4072 PG 866

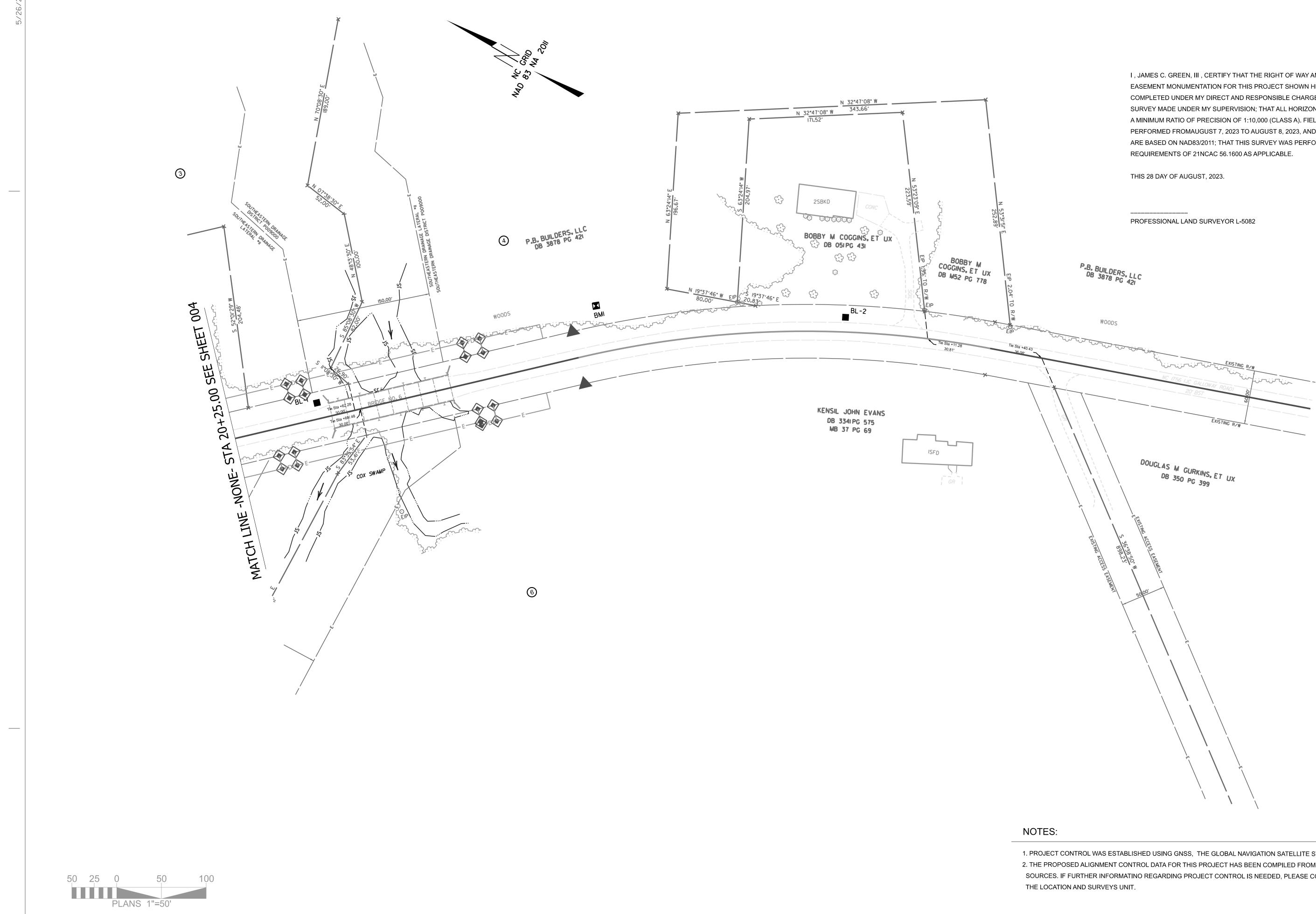
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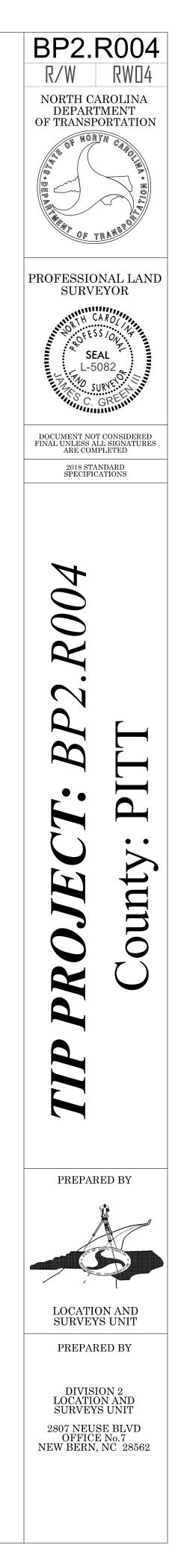
THIS 28 DAY OF AUGUST, 2023.

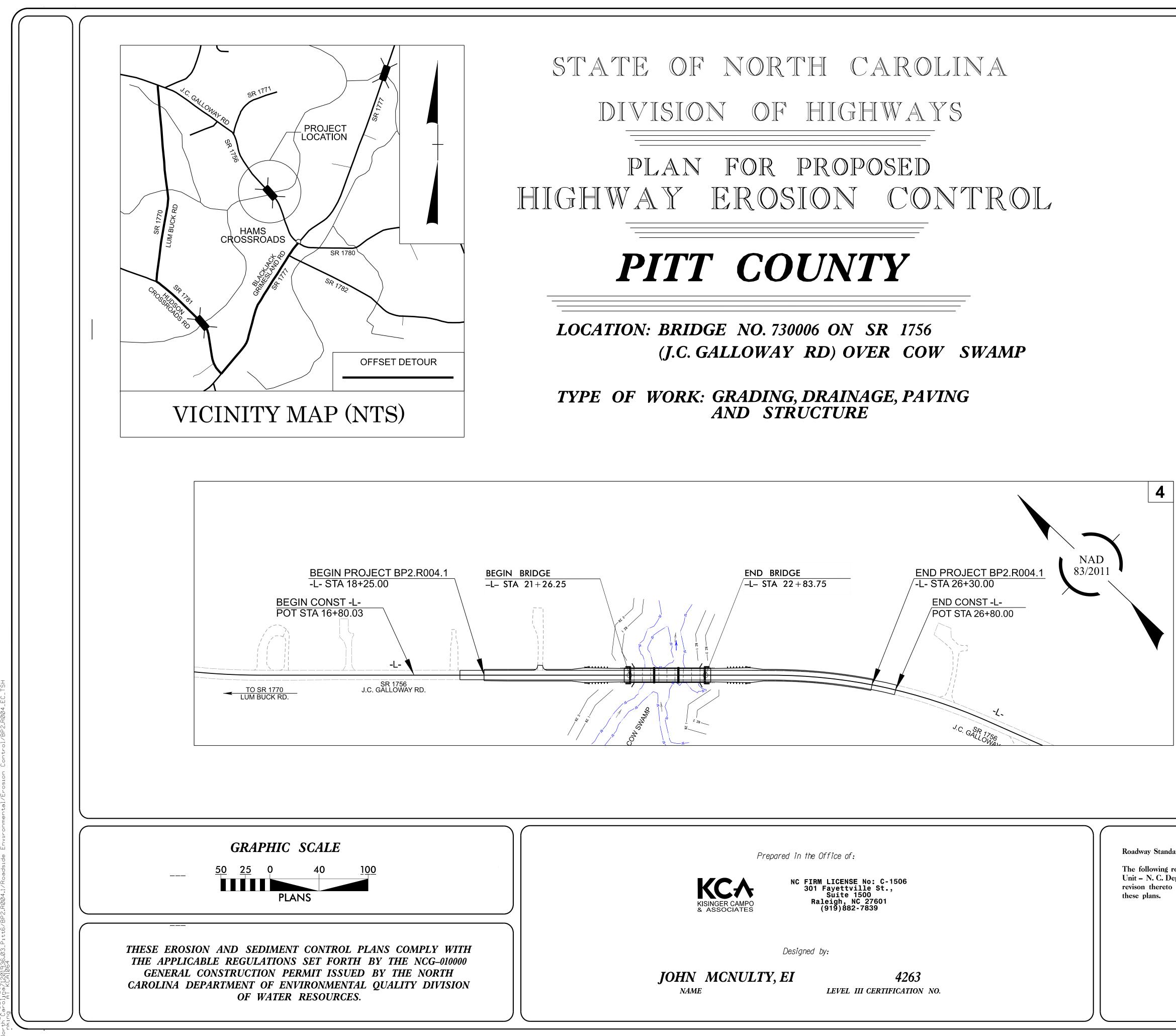




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Prepared in the Office of:
KISINGER CAMPO & ASSOCIATES NC FIRM LICENSE No: C-1506 301 Fayettville St., Suite 1500 Raleigh, NC 27601 (919)882-7839
Designed by:
JOHN MCNULTY, EI 4263 NAME LEVEL III CERTIFICATION NO.

STATE	STATE P	ROJECT REFERENCE NO.	SHEET NO.	TOTAL SHEETS
N.C.	B	P2.R004	EC-1	
STATE P	ROJ. NO.	F. A. PROJ. NO.	DESCRIPTI	ON

THIS PROJECT CONTAINS EROSION CONTROL PLANS FOR CLEARING AND **GRU33ING PHASE OF** CONSTRUCTION.

ENVIRONMENTALLY **SENSITIVE AREA(S) EXIST ON THIS PROJECT**

Refer To E. C. Special Provisions for Special Considerations.

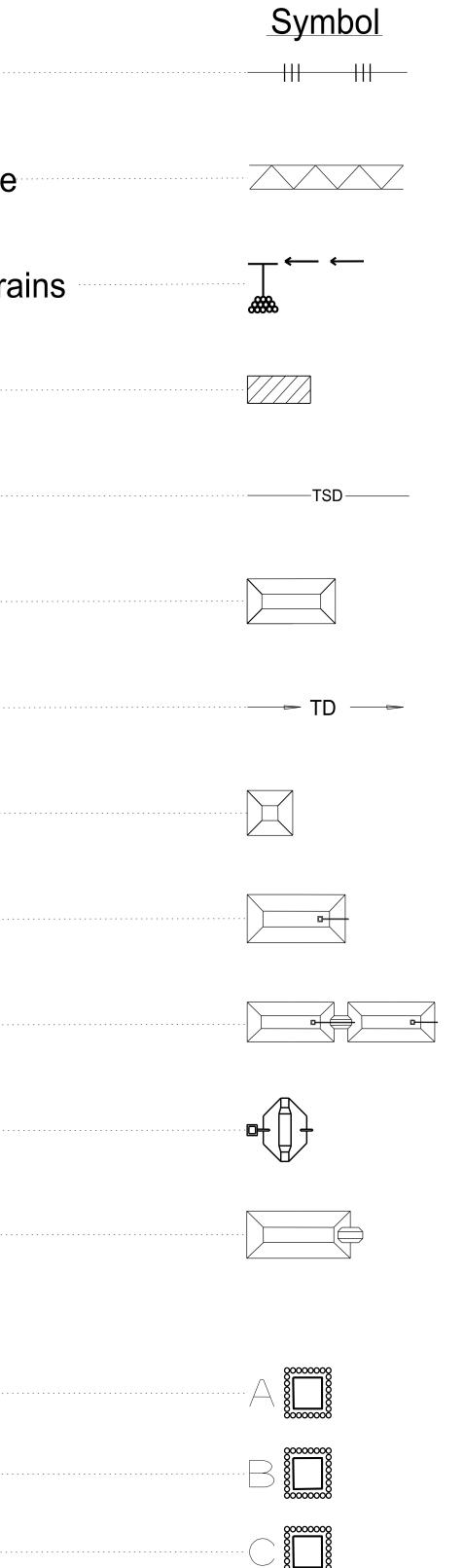
Roadway Standard Drawings

The following roadway english standards as appear in "Roadway Standard Drawings"– Roadway Design Unit – N. C. Department of Transportation – Raleigh, N. C., dated January 2024 and the latest revison thereto are applicable to this project and by reference hereby are considered a part of

EROSION & SEDIMENT CONTROL LEGE

<u>Std. #</u>	Description
1605.01	Temporary Silt Fence
1606.01	Special Sediment Control Fence
1622.01	Temporary Berms and Slope Dra
1630.02	Silt Basin Type B
1630.03	Temporary Silt Ditch
1630.04	Stilling Basin
1630.05	Temporary Diversion
1630.06	Special Stilling Basin
1630.07	Skimmer Basin
1630.08	Tiered Skimmer Basin
1630.09	Earthen Dam with Skimmer
	Infiltration Basin
1632.01	Rock Inlet Sediment Trap: Type A
1632.02	Туре В
1632.03	Туре С

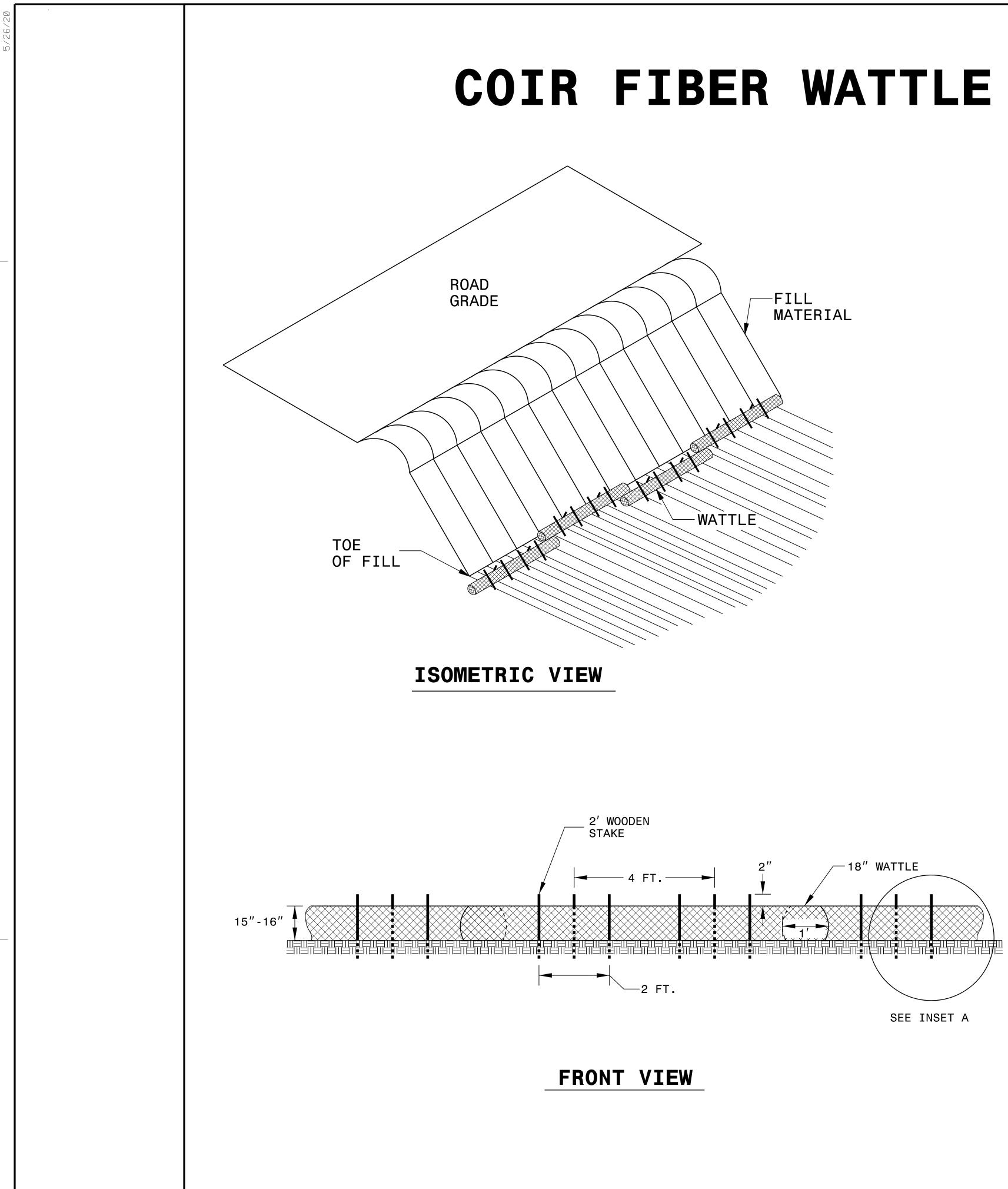
DIVISION OF HIGHWAYS STATE OF NORTH CAROLINA



•	
Std. #	Description
1633.01	Temporary Rock Silt Check Ty
1633.02	Temporary Rock Silt Check Ty
1633.03	Temporary Rock Silt Check Ty Excelsior Matting and Floccula
1634.01	Temporary Rock Sediment Da
1634.02	Temporary Rock Sediment Da
1635.01	Rock Pipe Inlet Sediment Trap
1635.02	Rock Pipe Inlet Sediment Trap
1636.01	Excelsior Wattle Check
1636.01	Excelsior Wattle Check with Fl
1636.01	Coir Fiber Wattle Check
1636.01	Coir Fiber Wattle Check with F
1636.02	Silt Fence Excelsior Wattle Bre
	Silt Fence Coir Fiber Wattle Br
1636.03	Excelsior Wattle Barrier
1636.03	Coir Fiber Wattle Barrier

	PROJECT REFERENCE N	IO. SHEET NO.
	BP2.R004	EC-2
	ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER
EGEND		
t Check Type A	<u>Symbol</u>	
t Check Type B		
t Check Type A with nd Flocculant		
diment Dam Type A		
diment Dam Type B		
iment Trap Type A		
iment Trap Type B		
eck		
eck with Flocculant		
neck		
neck with Flocculant		
Wattle Break		
r Wattle Break	CFW	
rier	EW-EW-EW-	

CFW—CFW—CFW—



COIR FIBER WATTLE BARRIER DETAIL

NOTES:

USE MINIMUM 18 IN. NOMINAL DIAMETER COIR FIBER (COCONUT) WATTLE AND LENGTH OF 10 FT.

EXCAVATE A 2 TO 3 INCH TRENCH FOR WATTLE TO BE PLACED.

DO NOT PLACE WATTLES ON TOE OF SLOPE.

CROSS SECTION.

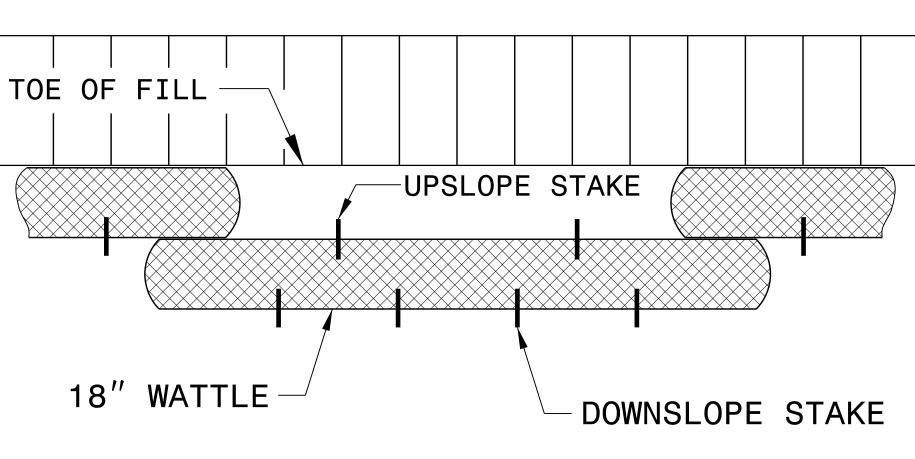
INSTALL A MINIMUM OF 2 UPSLOPE STAKES AND 4 DOWNSLOPE STAKES AT AN ANGLE TO WEDGE WATTLE TO GROUND.

INSTALL STAPLES APPROXIMATELY EVERY 1 LINEAR FOOT ON BOTH SIDES OF WATTLE AND AT EACH END TO SECURE IT TO THE SOIL.

FOR BREAKS ALONG LARGE SLOPES, USE MAXIMUM SPACING OF 25 FT.

FILL SLOPE

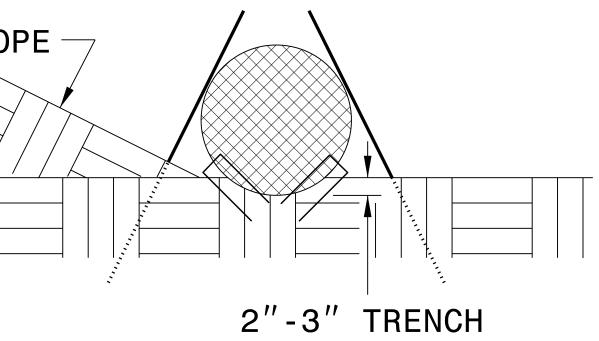




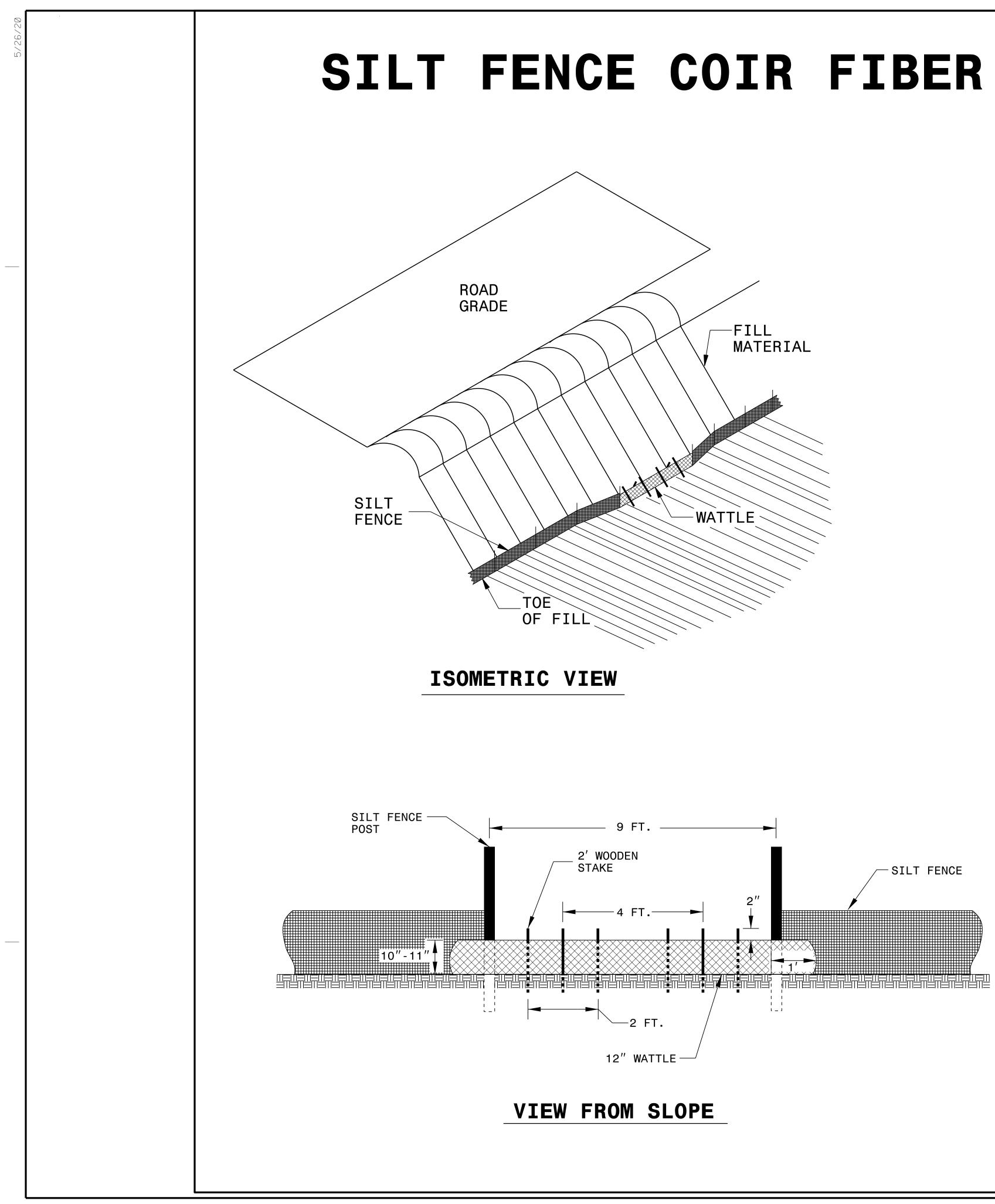
PROJECT REFERENCE NO	D. SHEET NO.
BP2.R004	EC-2A
R/W SHEET N	10.
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER

USE 2 FT. WOODEN STAKES WITH A 2 IN. BY 2 IN. NOMINAL

PROVIDE STAPLES MADE OF 0.125 IN. DIAMETER STEEL WIRE FORMED INTO A U SHAPE NOT LESS THAN 12" IN LENGTH.



TOP VIEW



SILT FENCE COIR FIBER WATTLE BREAK DETAIL



LENGTH OF 10 FT.

EXCAVATE A 1 TO 2 INCH TRENCH FOR WATTLE TO BE PLACED.

DO NOT PLACE WATTLE ON TOE OF SLOPE.

USE 2 FT. WOODEN STAKES WITH A 2 IN. BY 2 IN. NOMINAL CROSS SECTION.

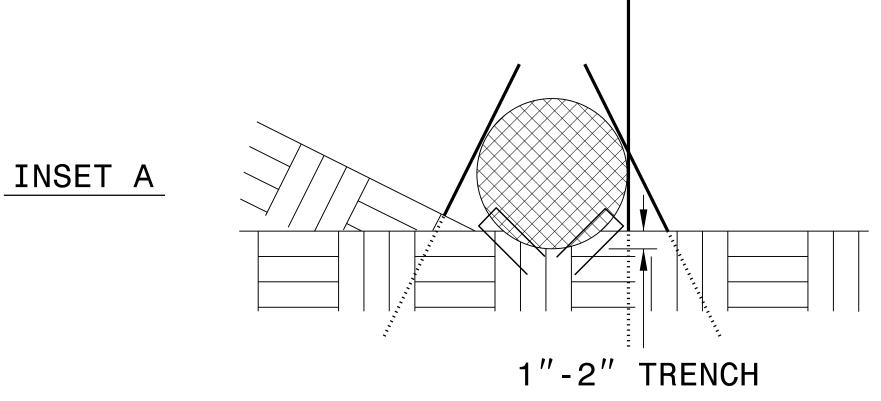
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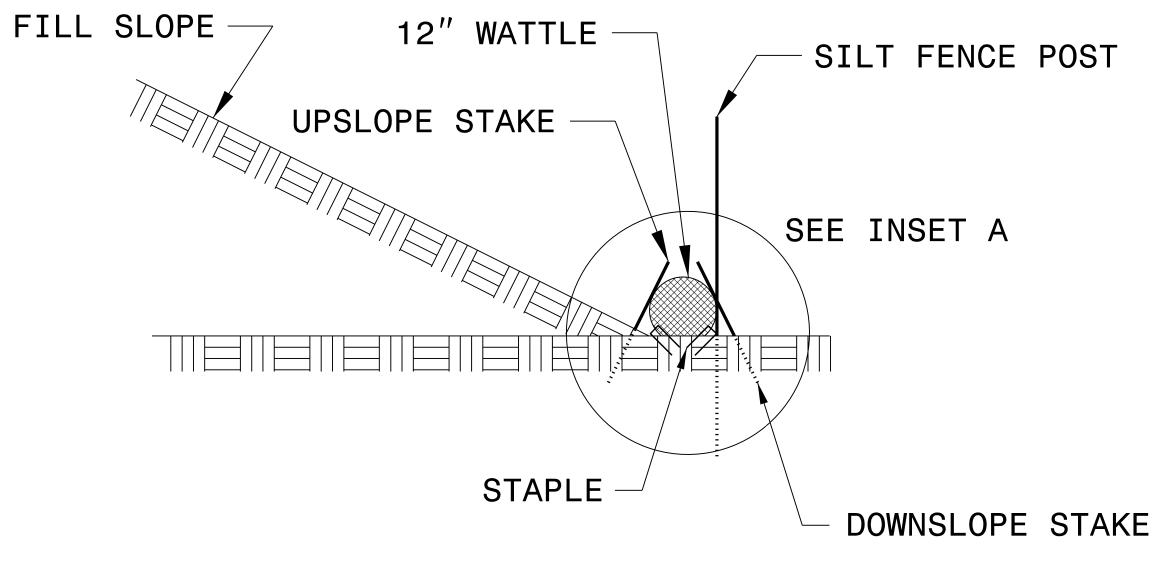
PROVIDE STAPLES MADE OF 0.125 IN. DIAMETER STEEL WIRE FORMED INTO A U SHAPE NOT LESS THAN 12" IN LENGTH.

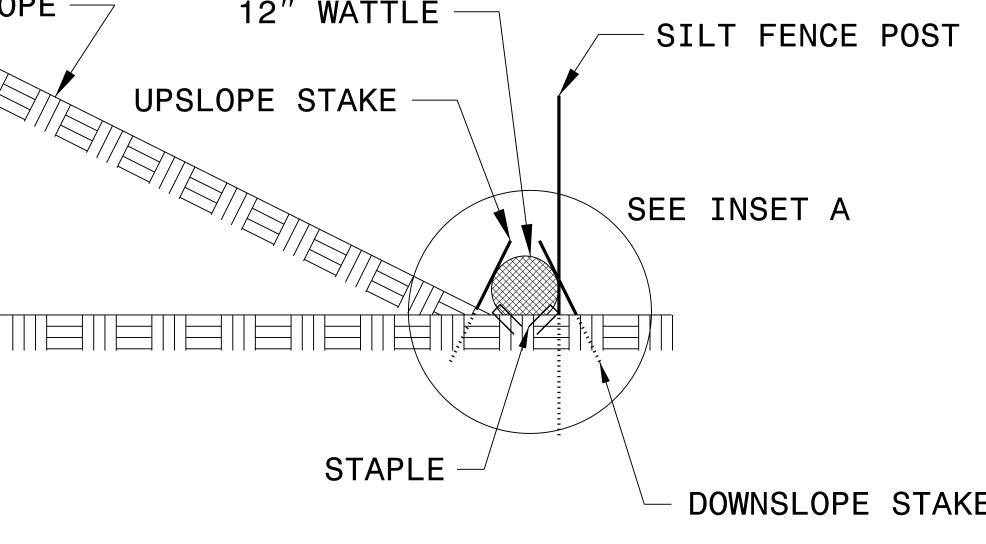
INSTALL STAPLES APPROXIMATELY EVERY 1 LINEAR FOOT ON BOTH SIDES OF WATTLE AND AT EACH END TO SECURE IT TO THE SOIL.

WATTLE INSTALLATION CAN BE ON OUTSIDE OF THE SILT FENCE AS DIRECTED.

INSTALL TEMPORARY SILT FENCE IN ACCORDANCE WITH SECTION 1605 OF THE STANDARD SPECIFICATIONS.





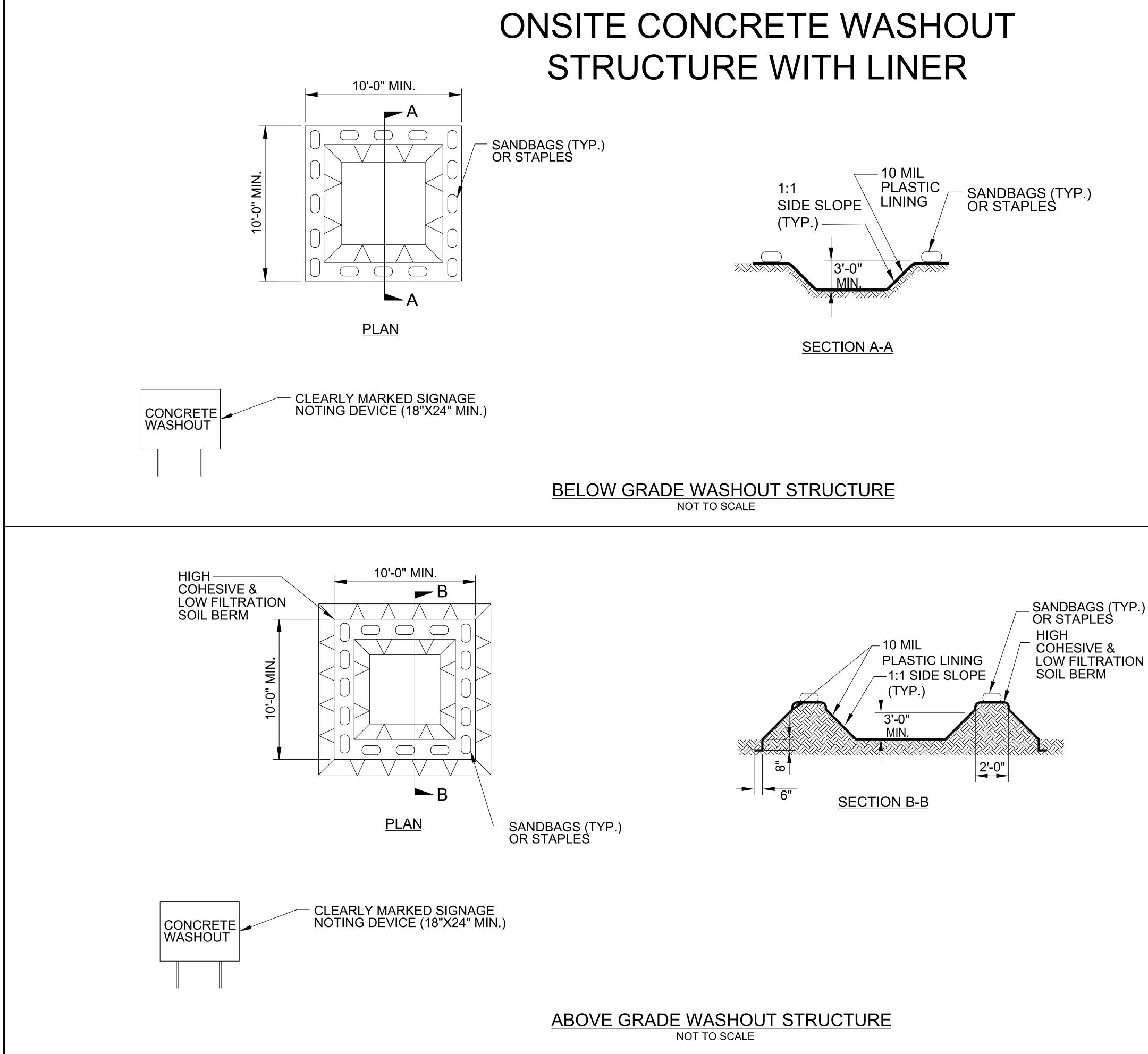


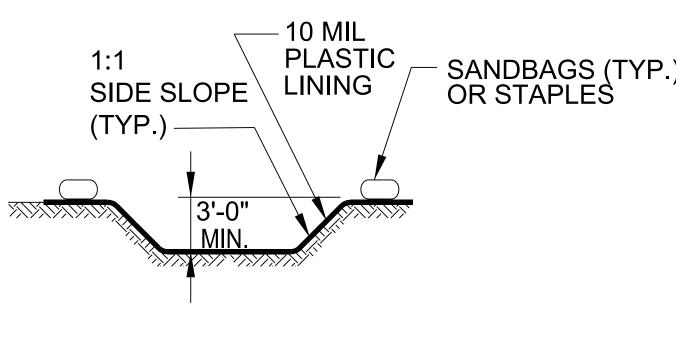


PROJECT REFERENCE NC	D. SHEET NO.
BP2.R004	EC-2B
R/W SHEET N	IO.
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER

USE MINIMUM 12 IN. DIAMETER COIR FIBER (COCONUT FIBER) WATTLE AND

SIDE VIEW





PROJECT REFERENCE NO	D. SHEET NO.
BP2.R004	EC-2C
R/W SHEET N	10.
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER

NOTES: 1. ACTUAL LOCATION DETERMINED IN FIELD

2. THE CONCRETE WASHOUT STRUCTURES SHALL BE MAINTAINED WHEN THE LIQUID AND/OR SOLID REACHES 75% OF THE STRUCTURES CAPACITY TO PROVIDE ADEQUATE HOLDING CAPACITY WITH A MINIMUM 12 INCHES OF FREEBOARD.

3.CONCRETE WASHOUT STRUCTURE NEEDS TO BE CLEARY MARKED WITH SIGNAGE NOTING DEVICE.

NOTES: 1. ACTUAL LOCATION DETERMINED IN FIELD

2. THE CONCRETE WASHOUT STRUCTURES SHALL BE MAINTAINED WHEN THE LIQUID AND/OR SOLID REACHES 75% OF THE STRUCTURES CAPACITY TO PROVIDE ADEQUATE HOLDING CAPACITY WITH A MINIMUM 12 INCHES OF FREEBOARD.

3.CONCRETE WASHOUT STRUCTURE NEEDS TO BE CLEARY MARKED WITH SIGNAGE NOTING DEVICE.

SITE DESCRIPTION

PERIMETER DIKES, SWALES, DITCHES AND

HIGH QUALITY WATER (HQW) ZONES

SLOPES STEEPER THAN 3:1

SLOPES 3:1 TO 4:1

ALL OTHER AREAS WITH SLOPES FLATTE

DIVISION OF HIGHWAYS STATE OF NORTH CAROLINA

SOIL STABILIZATION TIMEFRAMES

	STABILIZATION TIME	7,
) SLOPES	7 DAYS	NONE
	7 DAYS	NONE
	7 DAYS	IF SLOPE Not ste
		7 DAYS Length
I4 DAIS	I4 DAYS	7 DAYS Perimet
ter than 4:1	14 DAYS	7 DAYS Perimeti

PROJECT REFERENCE NC). SHEET NO.
BP2.R004	EC-3A
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER

IMEFRAME EXCEPTIONS

PES ARE 10' OR LESS IN LENGTH AND ARE FEEPER THAN 2:1, 14 DAYS ARE ALLOWED.

FOR SLOPES GREATER THAN 50' IN WITH SLOPES STEEPER THAN 4:1.

FOR PERIMETER DIKES, SWALES, DITCHES TER SLOPES, AND HQW ZONES

FOR PERIMETER DIKES, SWALES, DITCHES TER SLOPES, AND HQW ZONES

PERMANENT SOIL REINFORCEMNT MAT

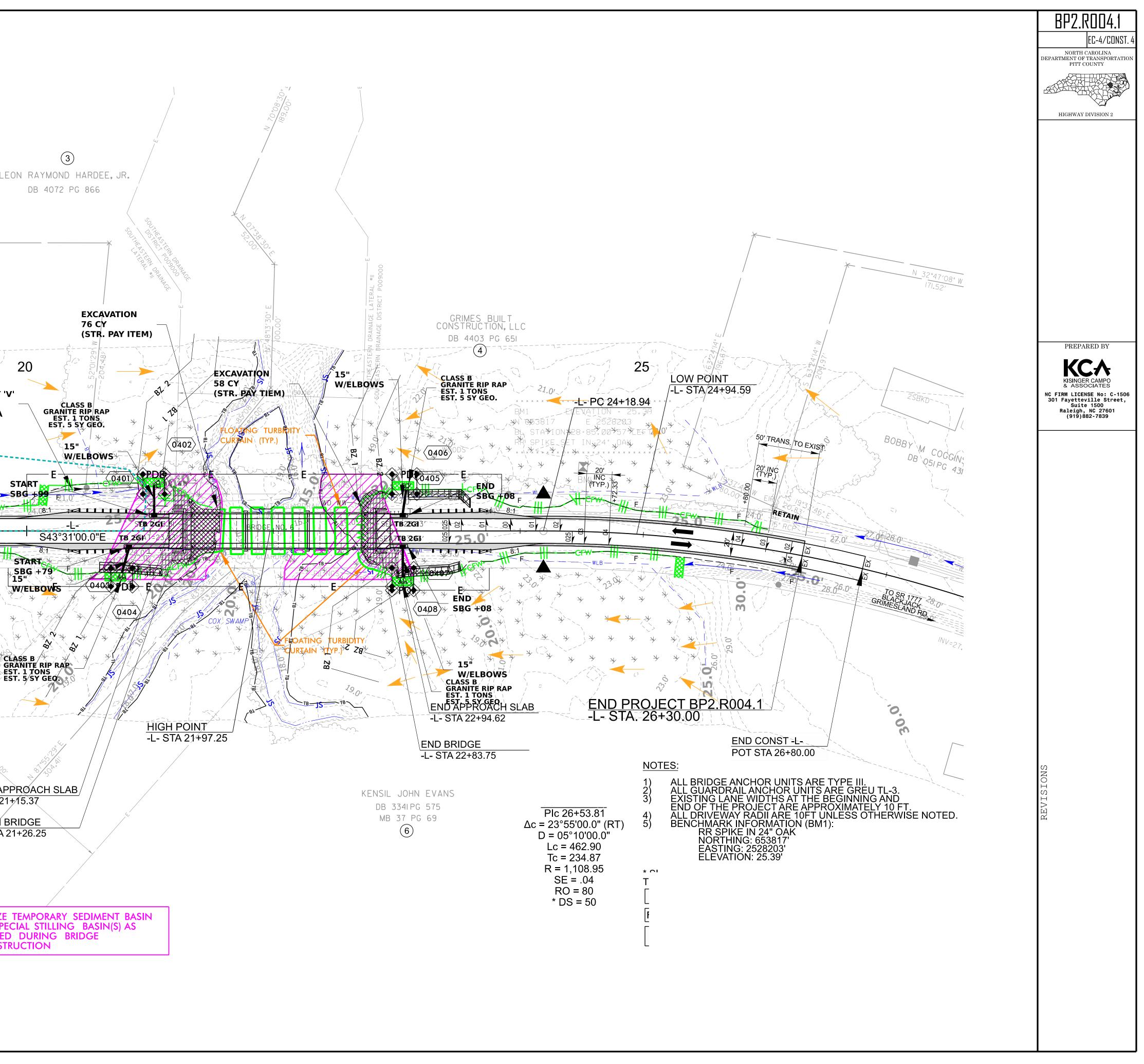
· · · · · · · · · · · · · · · · · · ·		1								-	
CONST SHEET NO.	LINE	FROM STATION	TO STATION	SIDE	ESTIMATE (SY)	CONST SHEET NO.	LINE	FROM STATION	TO STATION	SIDE	ESTIMATE (SY)
4						4		20+10	20+90	LT	185
				STOTAL	0			SUBTOTAL			185
	ADDITIONAL	PSKM TO	BE INST		0	N	VISCELLANEOUS	MATTING TO	O BE INS		2000
				TOTAL	0					TOTAL	2185
				SAY	0					SAY	2185

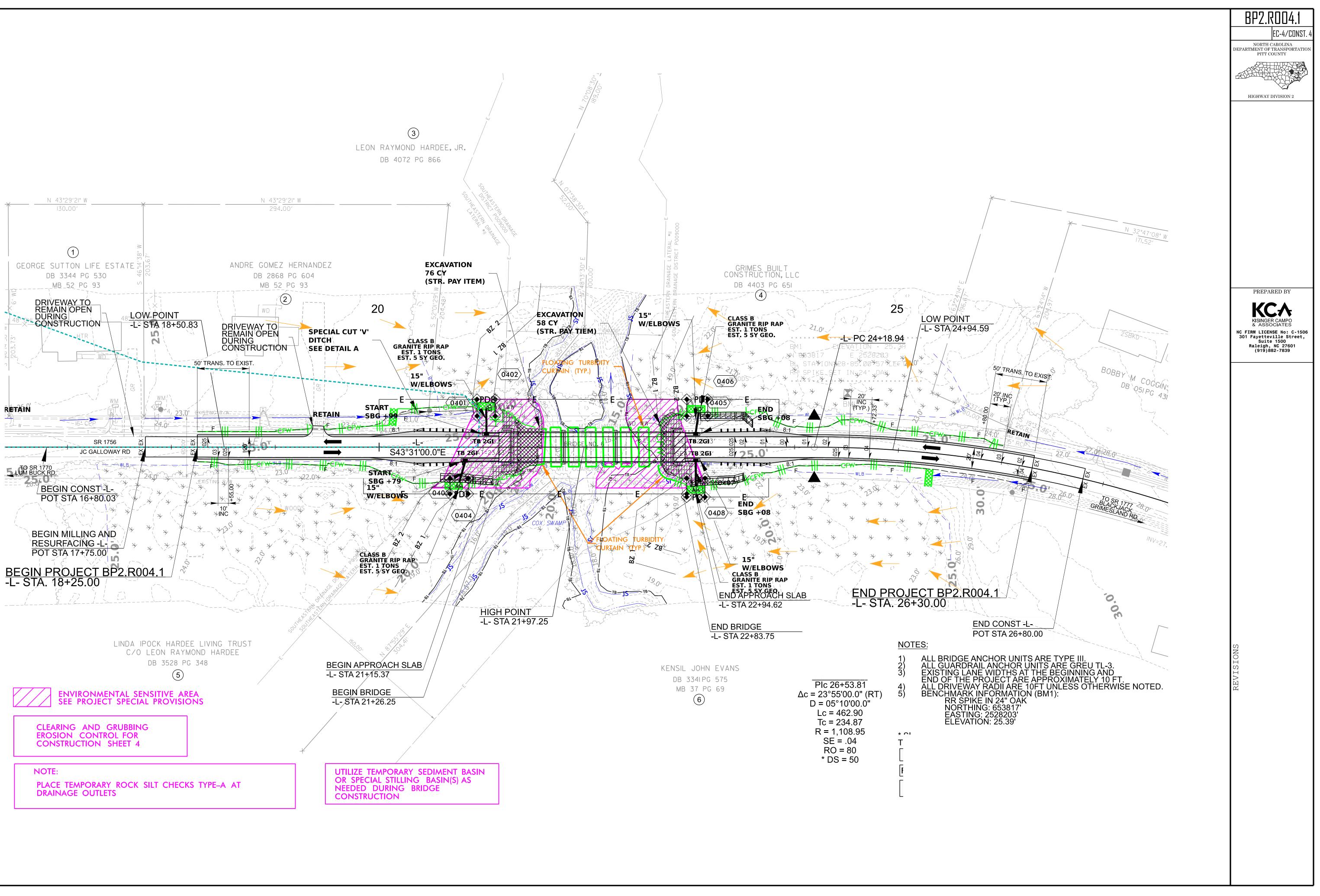
DIVISION OF HIGHWAYS STATE OF NORTH CAROLINA

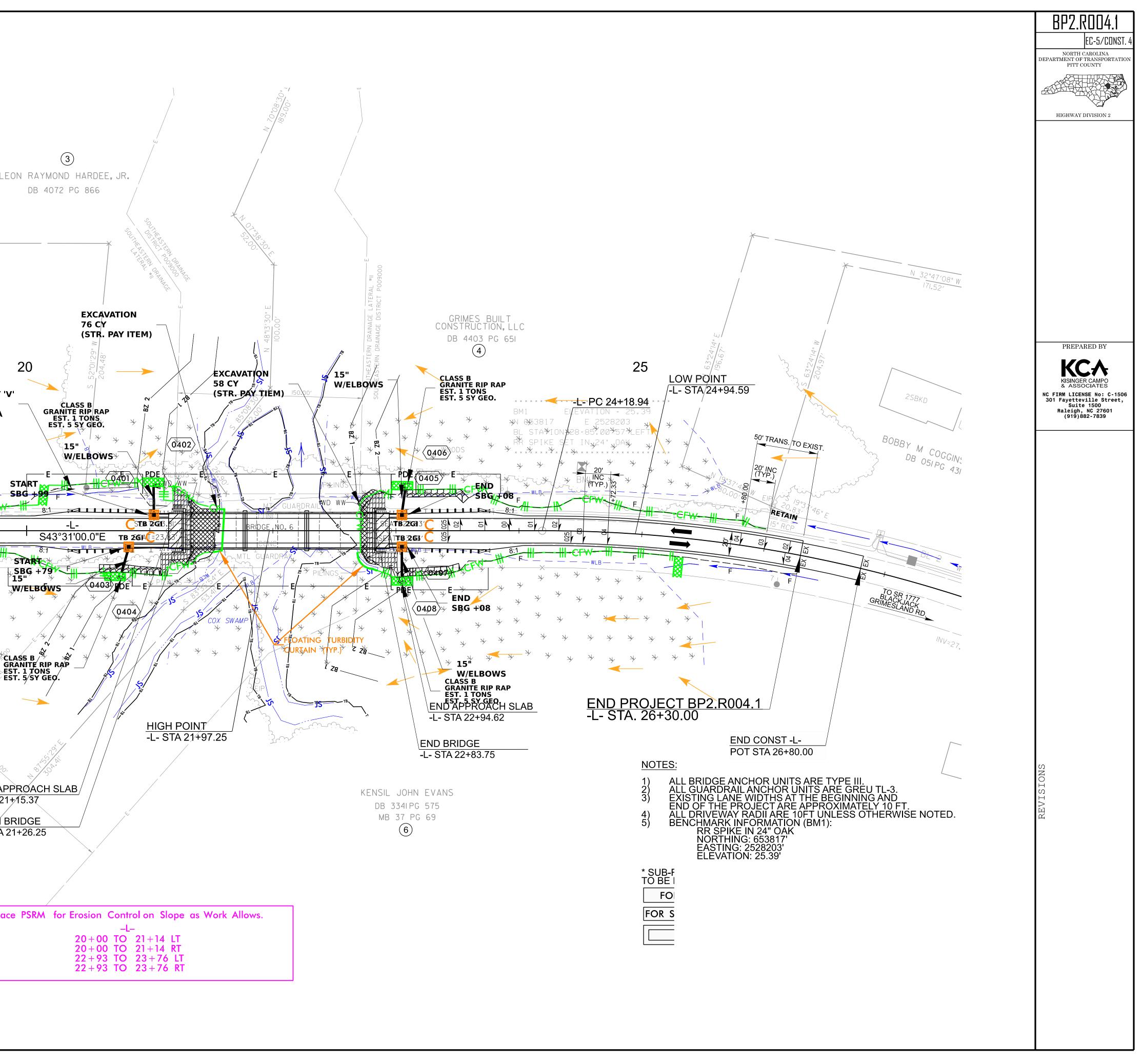
SOIL STABILIZATION SUMMARY SHEET

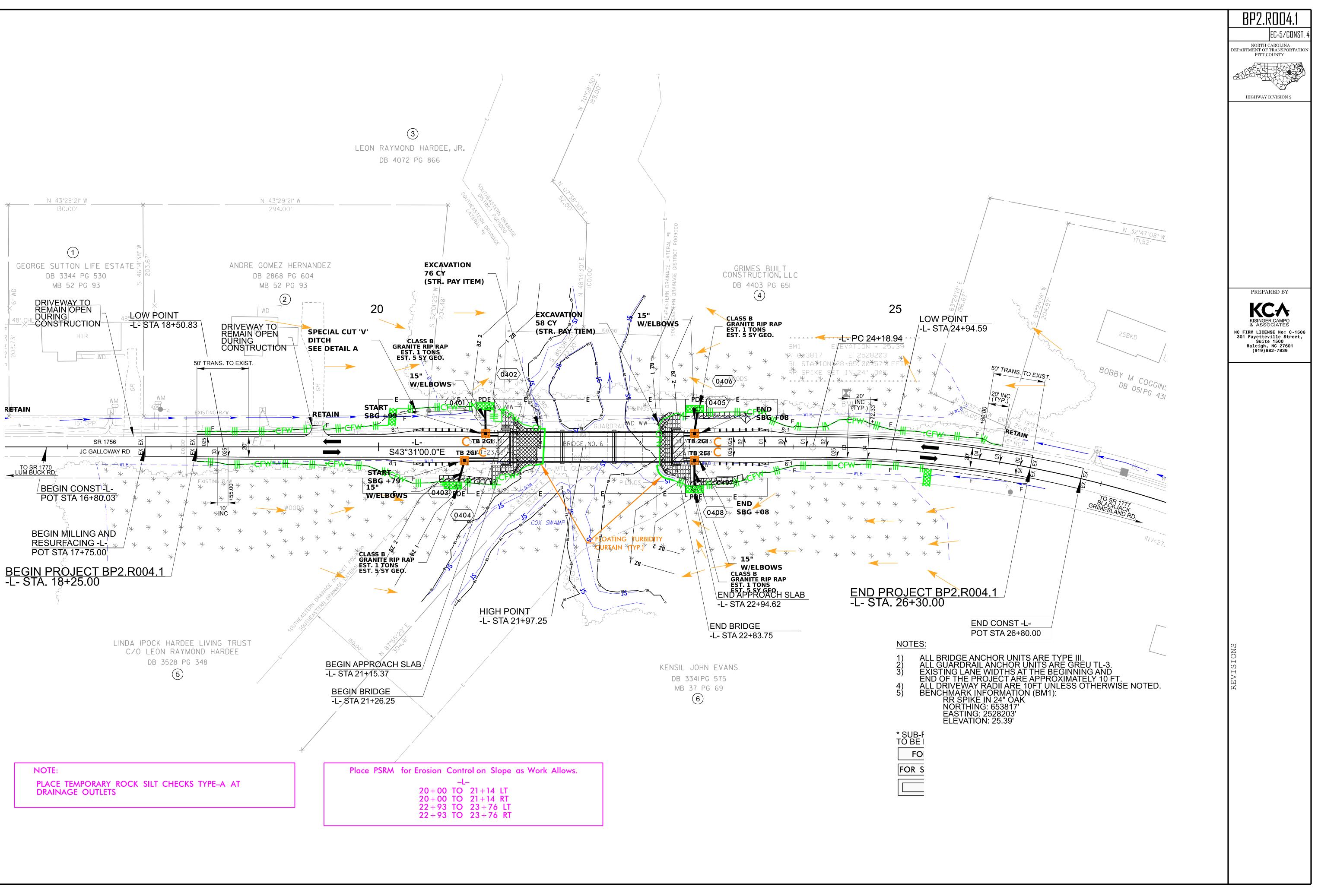
MATTING FOR EROSION CONTROL (STRAW)

PROJECT REFERENCE NC	D. SHEET NO.
BP2.R004	EC-3B
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER



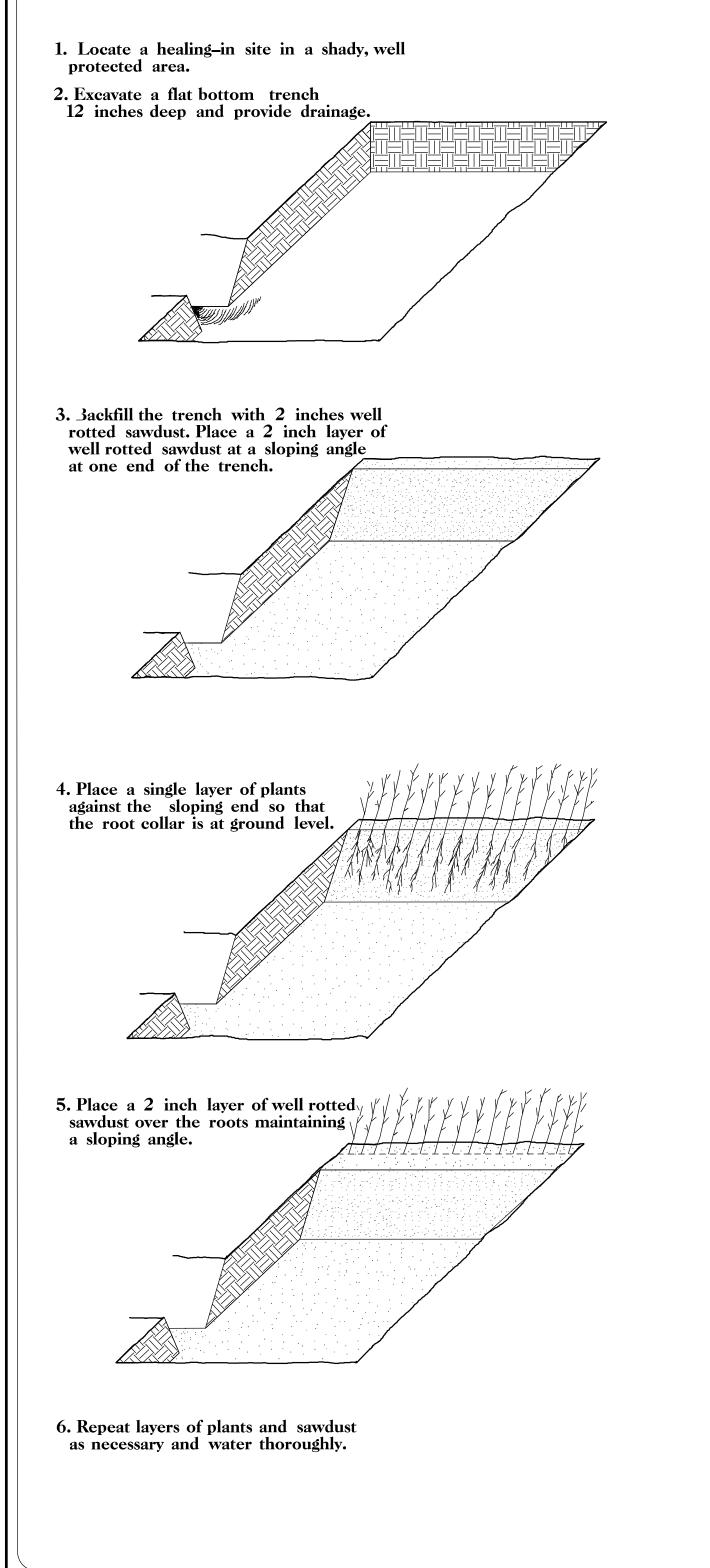


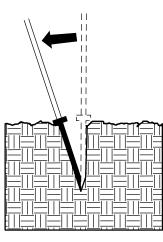




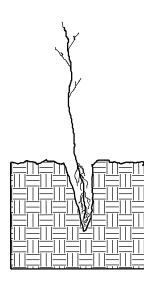
PLANTING DETAILS SEEDLING / LINER JAREROOT PLANTING DETAIL

HEALING IN

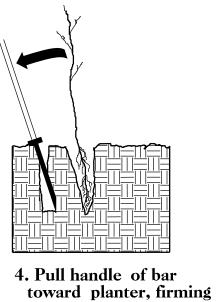




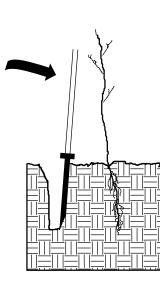
1. Insert planting bar as shown and pull handle toward planter.



2. Remove planting bar and place seedling at correct depth.



soil at bottom.



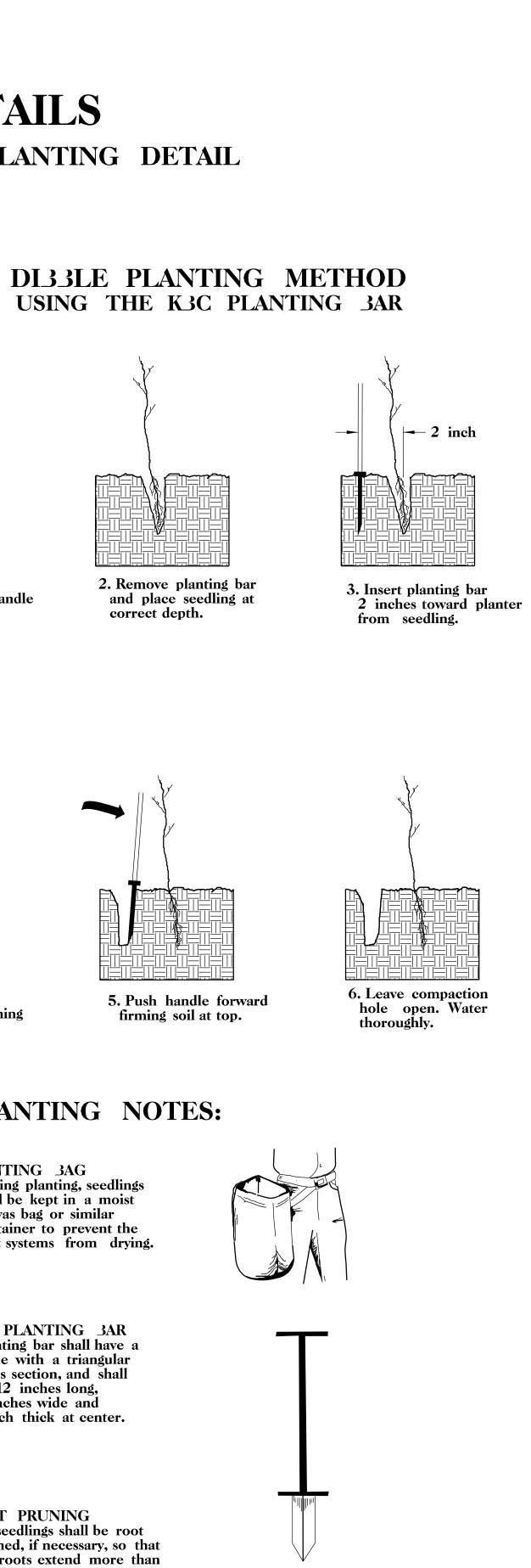
5. Push handle forward firming soil at top.

PLANTING NOTES:

PLANTING JAG During planting, seedlings shall be kept in a moist canvas bag or similar container to prevent the root systems from drying.

K3C PLANTING 3AR Planting bar shall have a blade with a triangular cross section, and shall be 12 inches long, 4 inches wide and 1 inch thick at center.

ROOT PRUNING All seedlings shall be root pruned, if necessary, so that no roots extend more than 10 inches below the root collar.



REFORESTATION

□ TREE REFORESTATION SHALL JE PLANTED 6 FT. TO 10 FT. ON CENTER, RANDOM SPACING, AVERAGING 8 FT. ON CENTER, APPROXIMATELY 680 PLANTS PER ACRE.

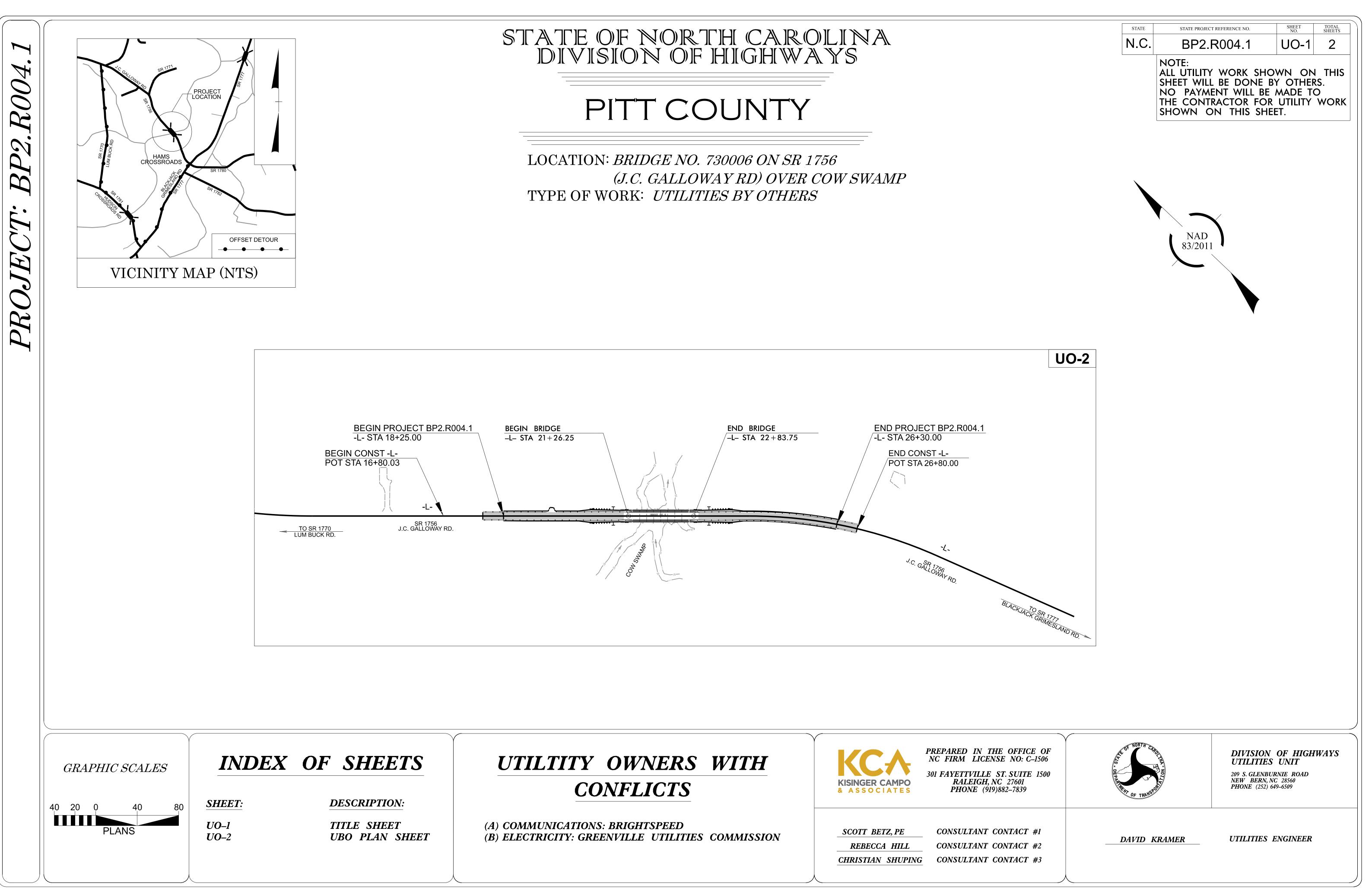
MIXTURE, TYPE, SIZE, AND FURNISH SHALL	CONFORM TO THE FOLLOWING	8
25% LIRIODENDRON TULIPIFERA	TULIP POPLAR	12 in – 18 in <i>3</i> R
25% PLATANUS OCCIDENTALIS	AMERICAN SYCAMORE	12 in – 18 in J R
25% FRAXINUS PENNSYLVANICA	GREEN ASH	12 in – 18 in <i>3</i> R
25% JETULA NIGRA	RIVER JIRCH	12 in – 18 in 3 R

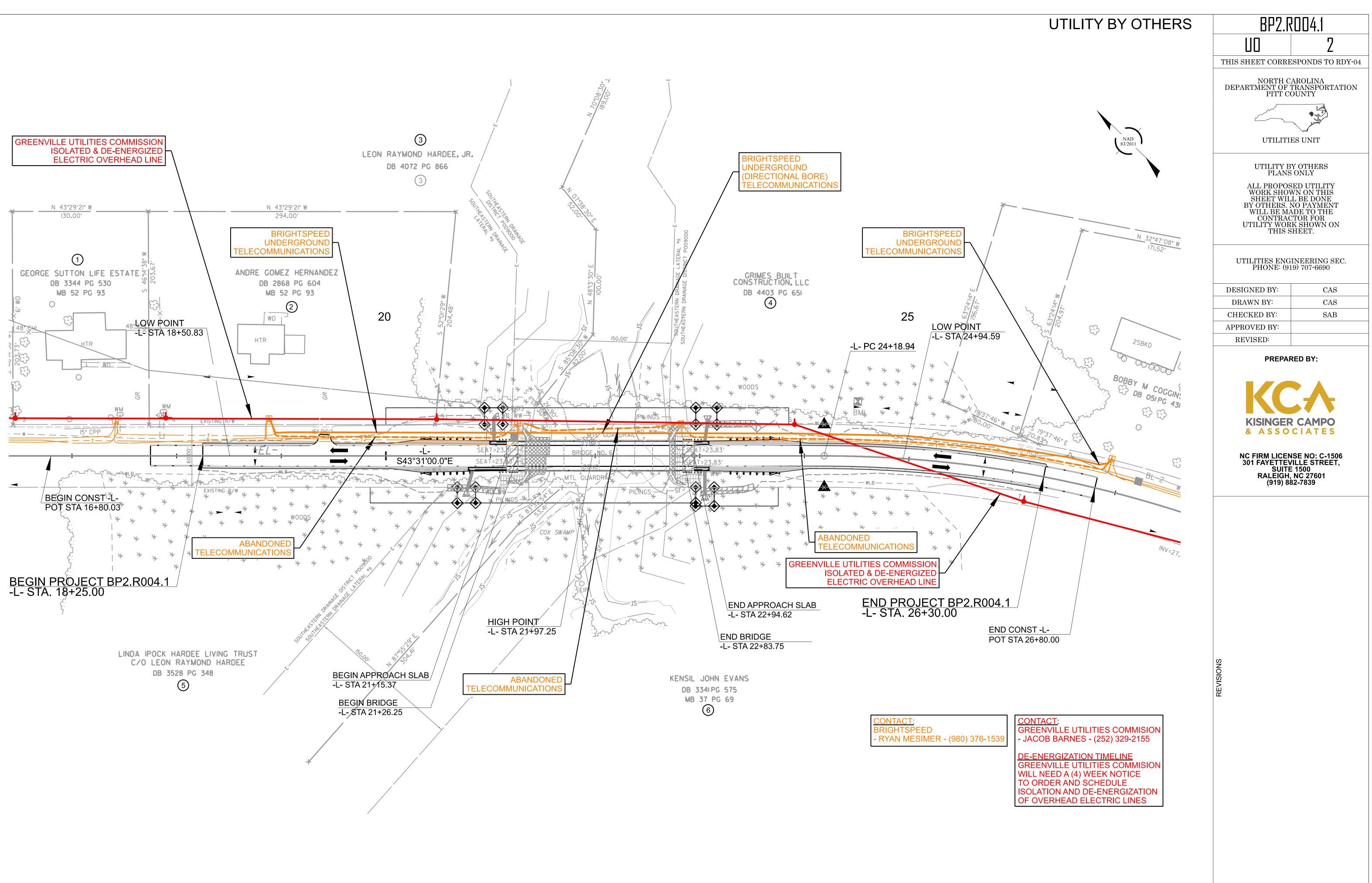


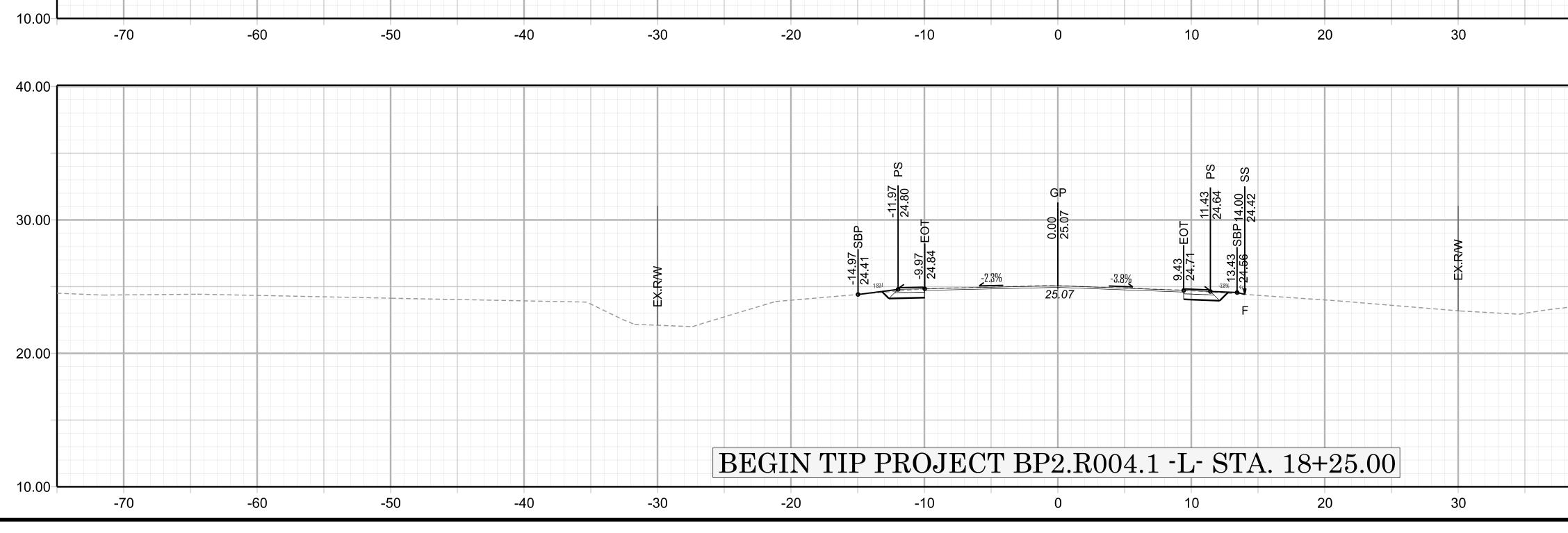
STATE	STATE	SHEET NO.	TOTAI SHEET	
N.C.	BF	P2.R004	RF-1	
STATE PROJ. NO.		F. A. PROJ. NO.	DESCRIPT	ION

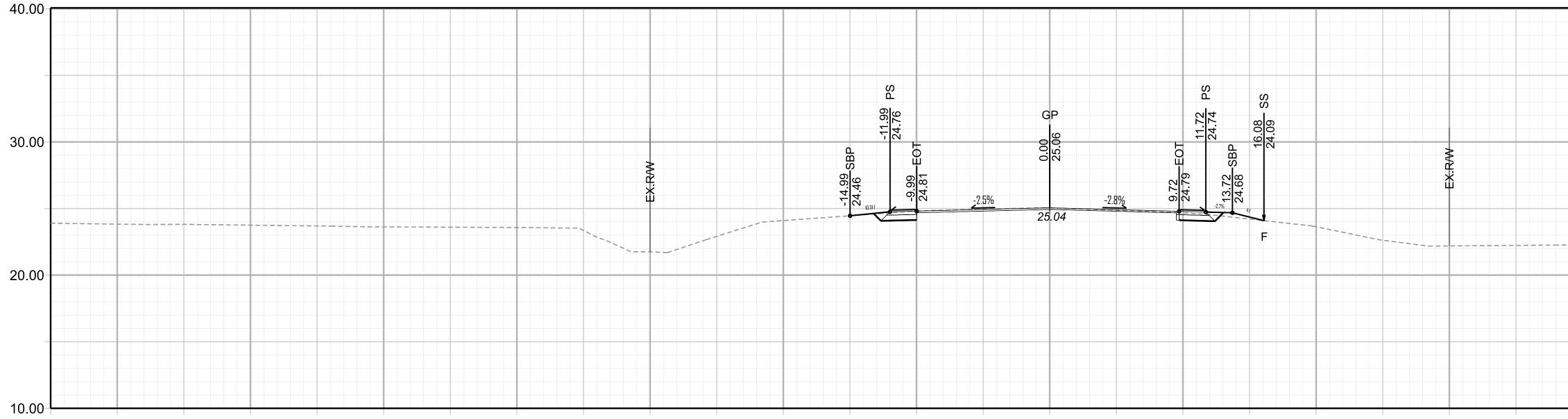
REFORESTATION DETAIL SHEET

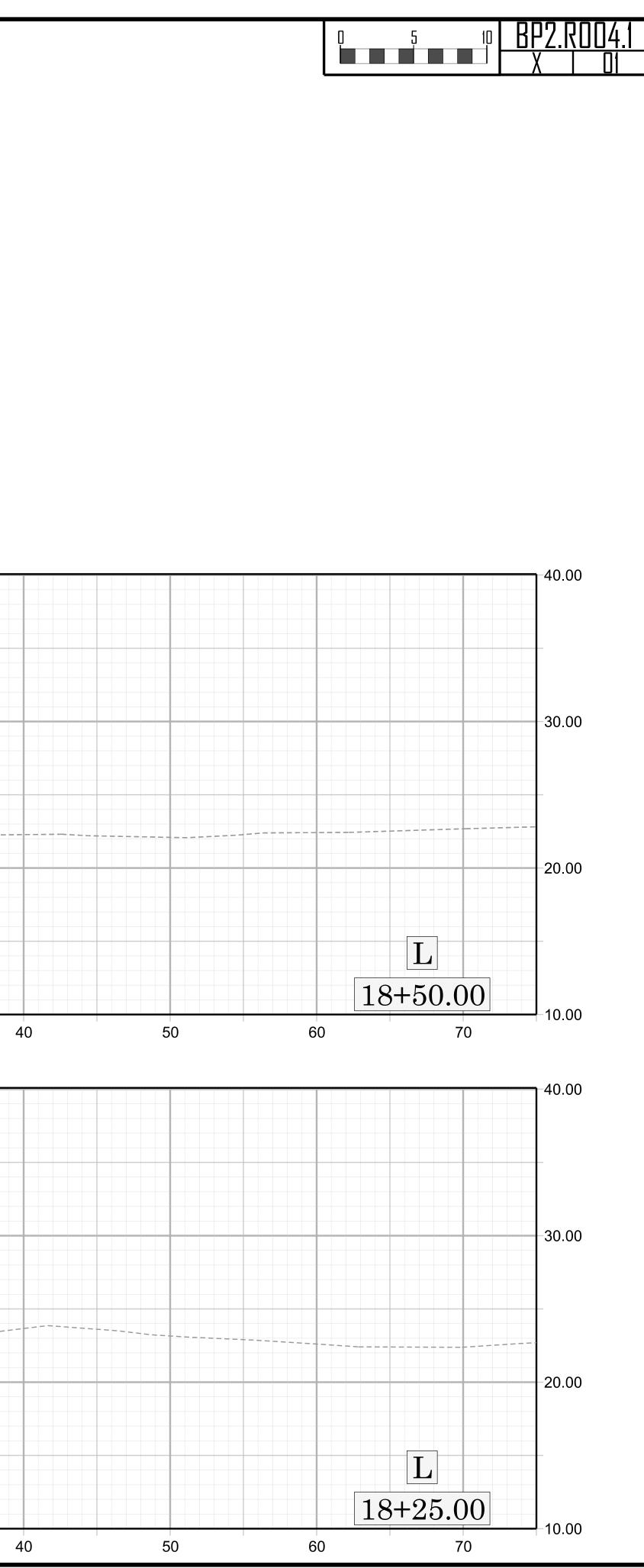
N.C.D.O.T. - ROADSIDE ENVIRONMENTAL UNIT

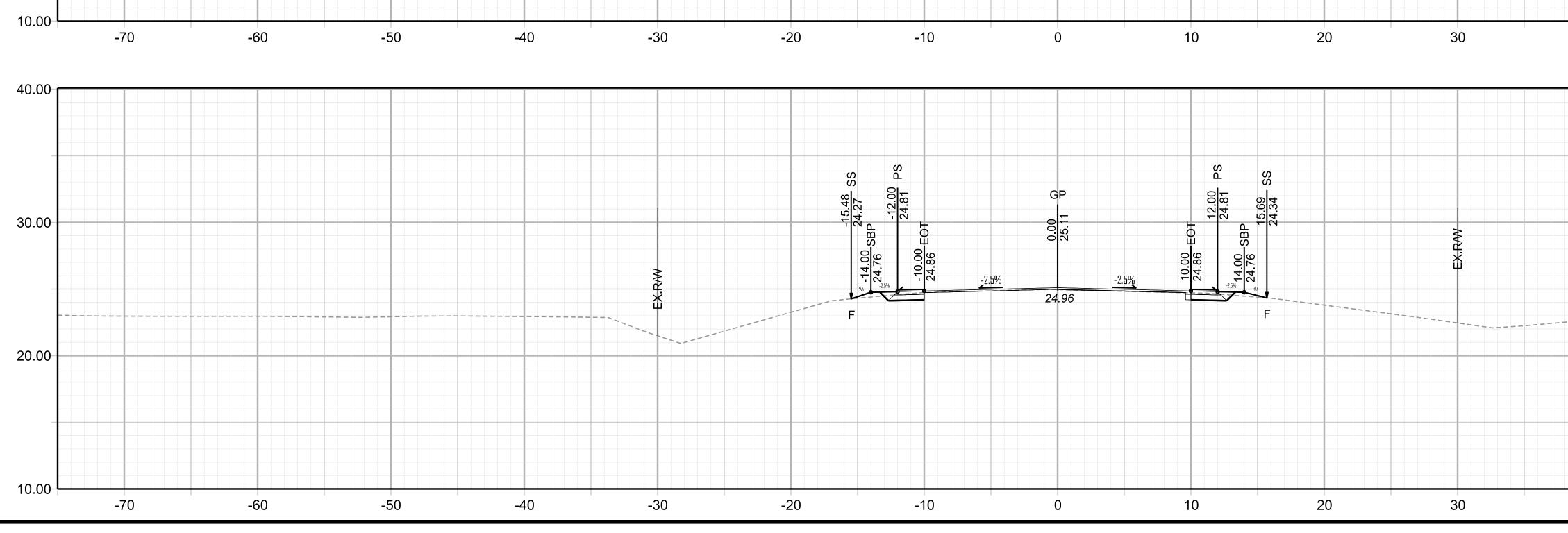


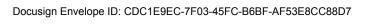










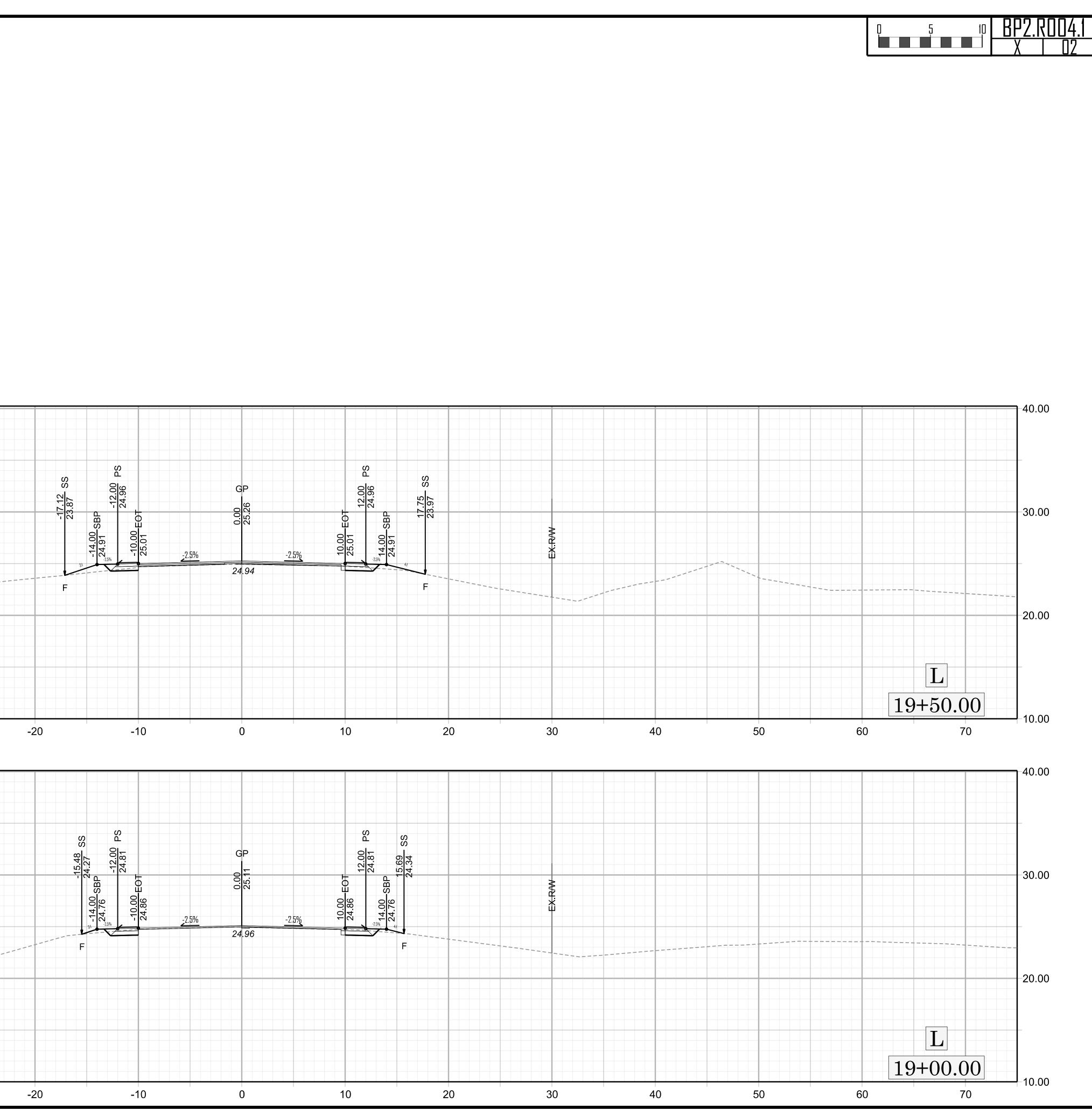


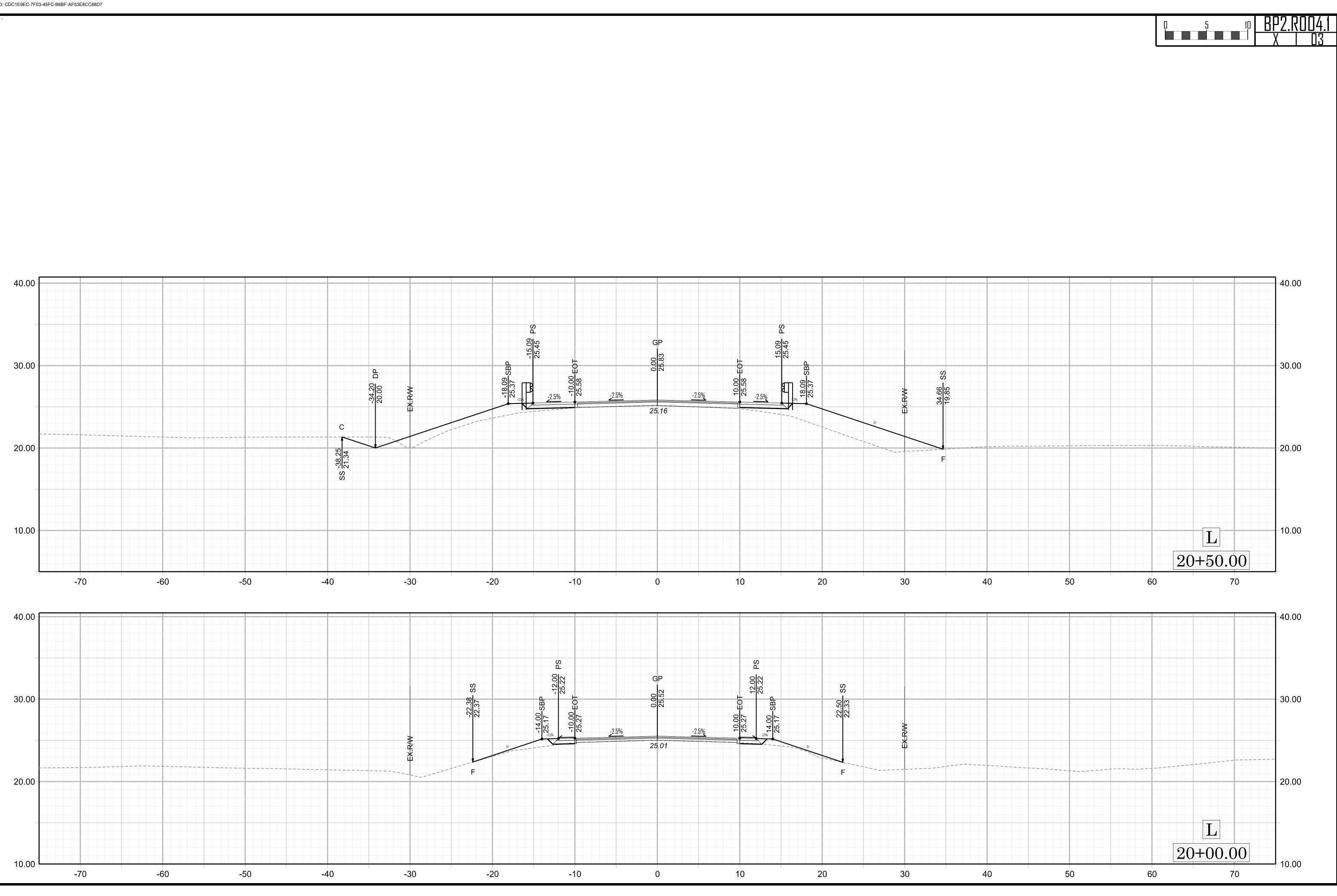
40.00 **f**

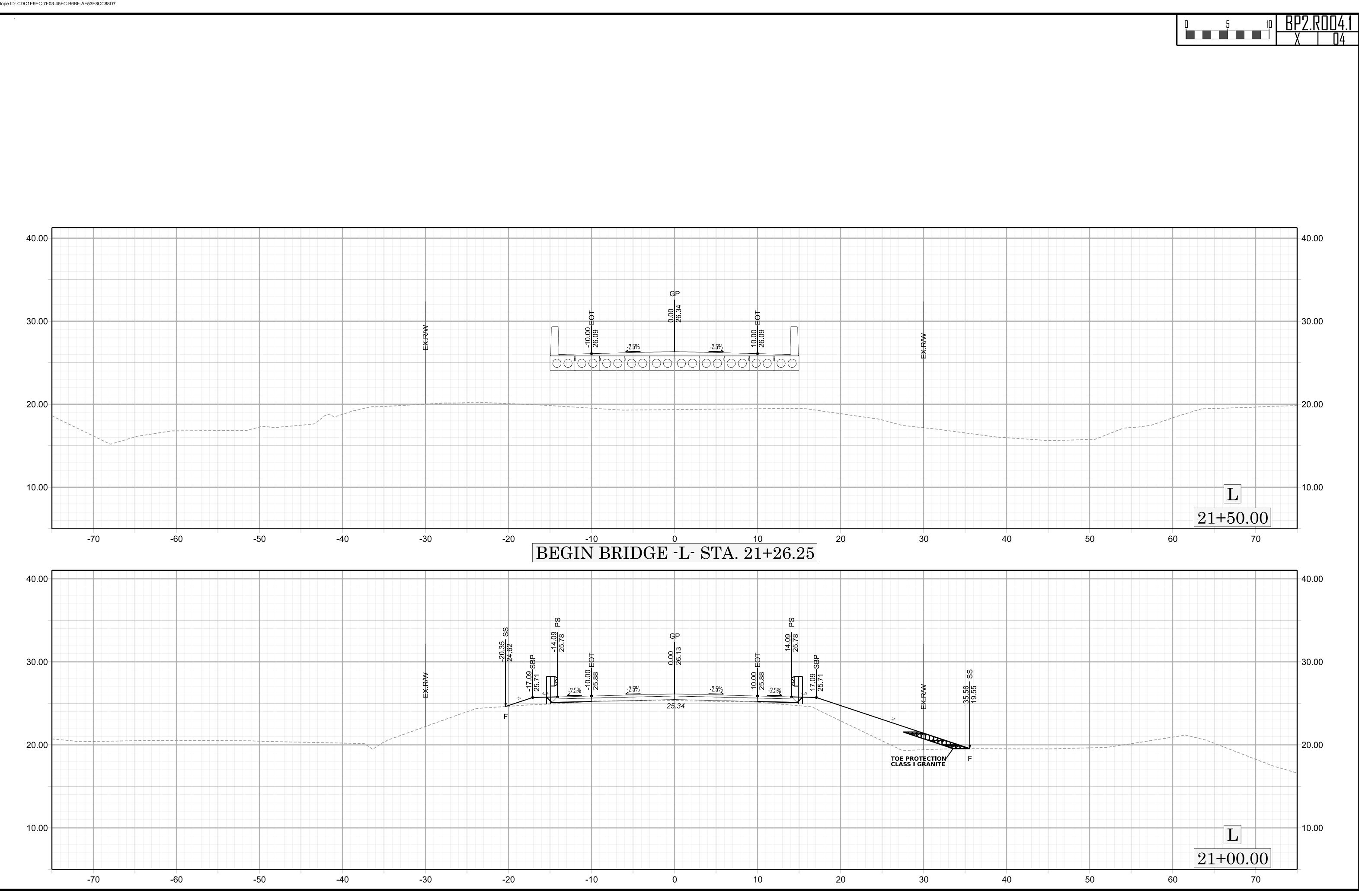
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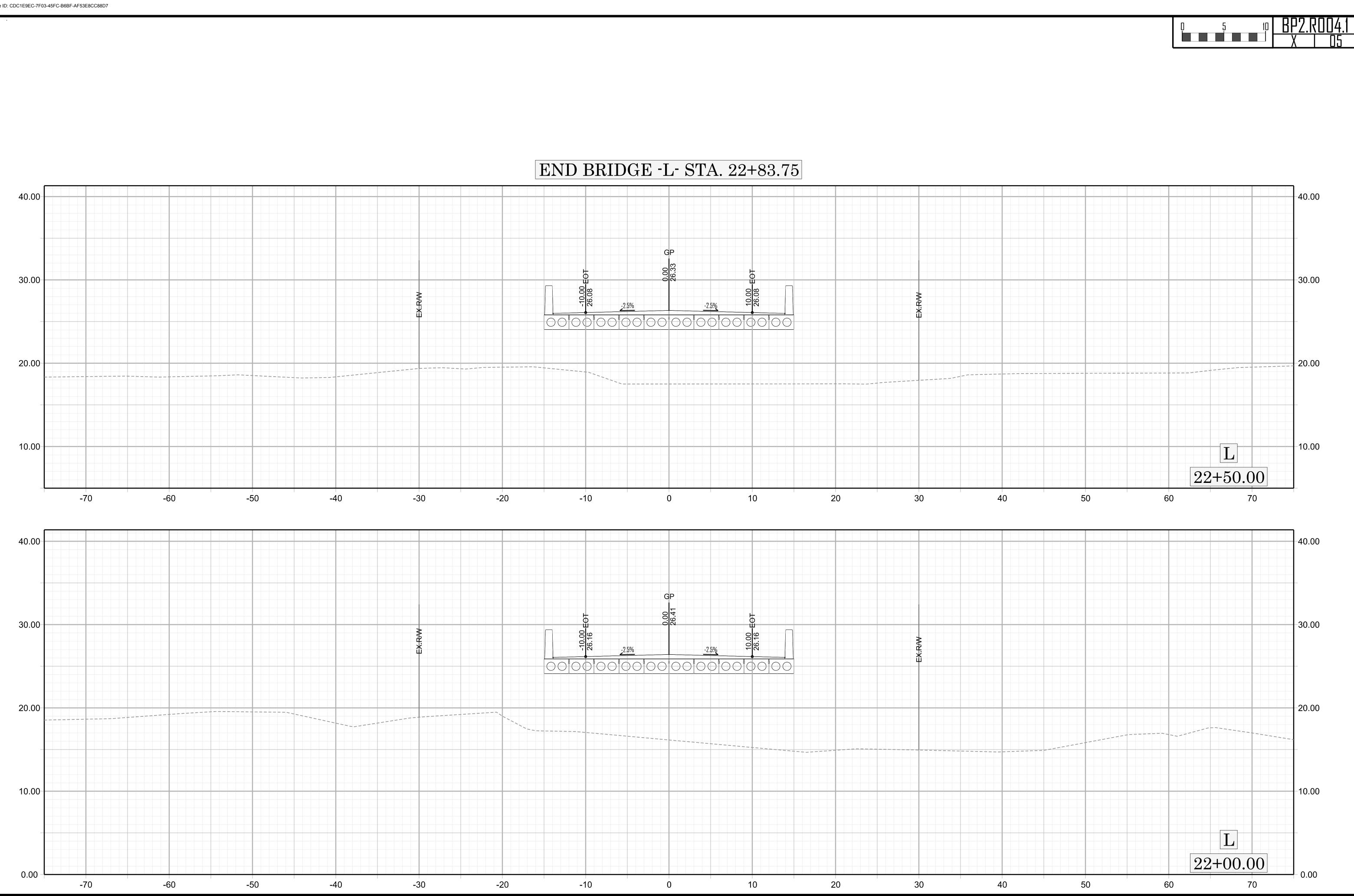
20.00

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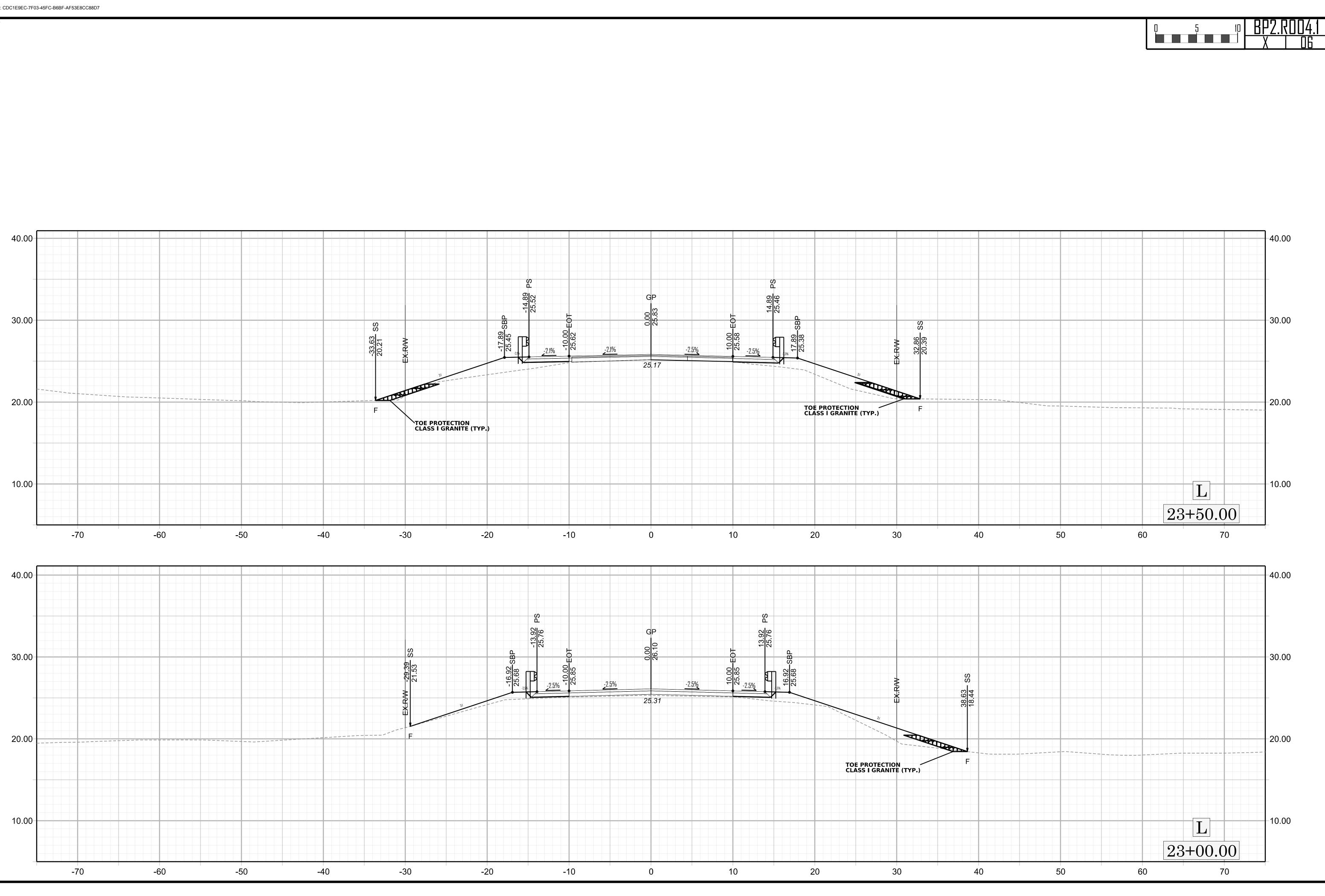


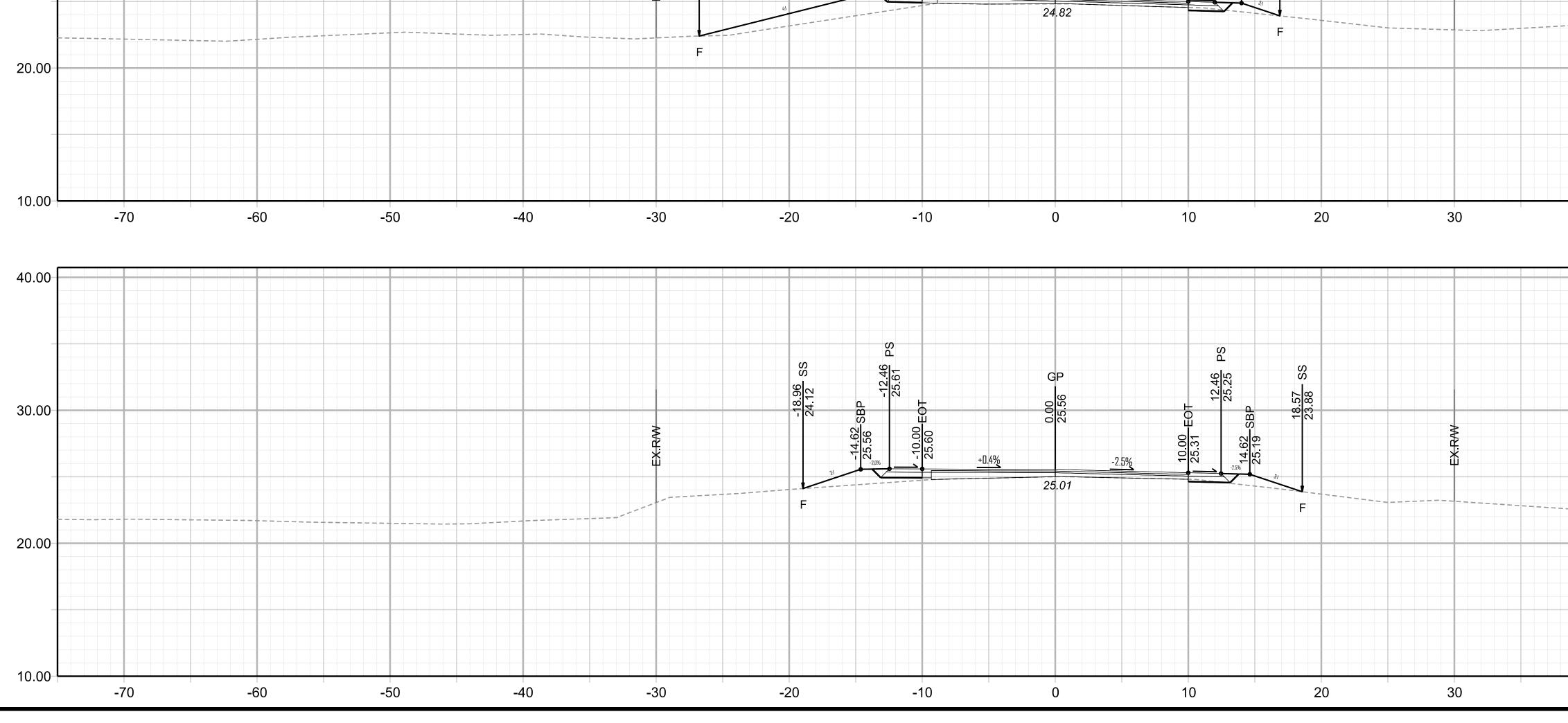












PS

12.00 25.63

<u>-14.00</u> 25.59 +2.9%

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<u>12.00</u> 24.94

SBI

24.88 24.88

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<u>10.00</u> 25.00

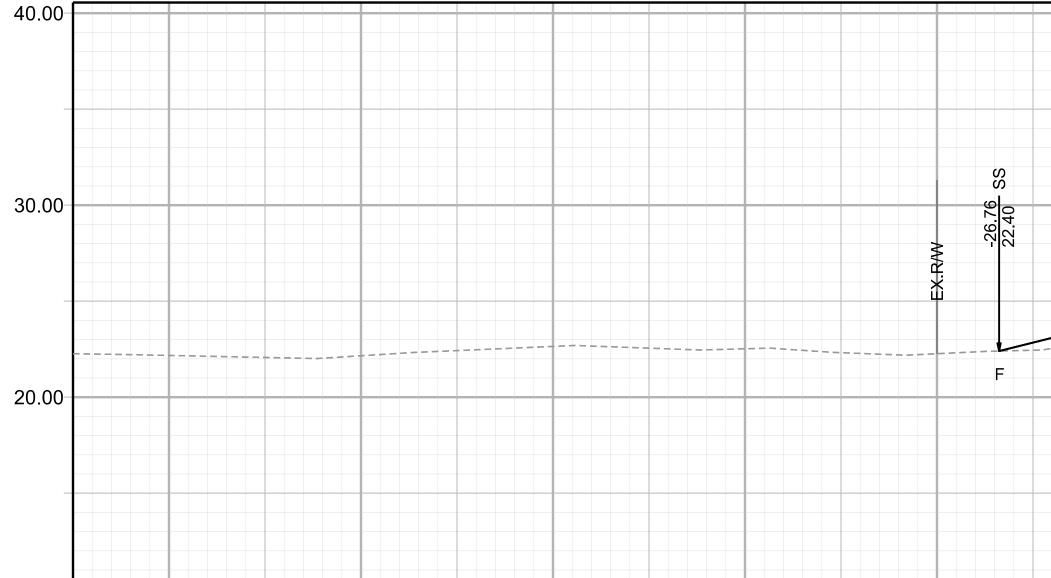
-2.9%

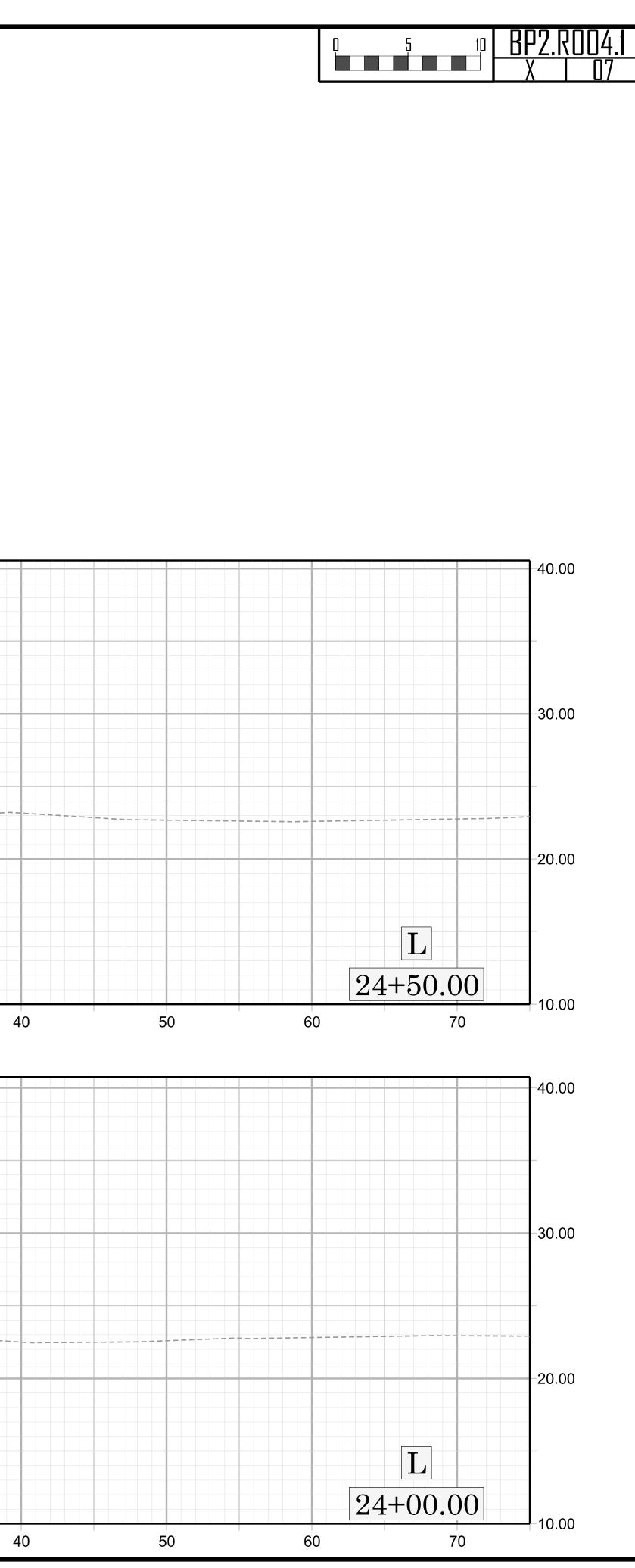
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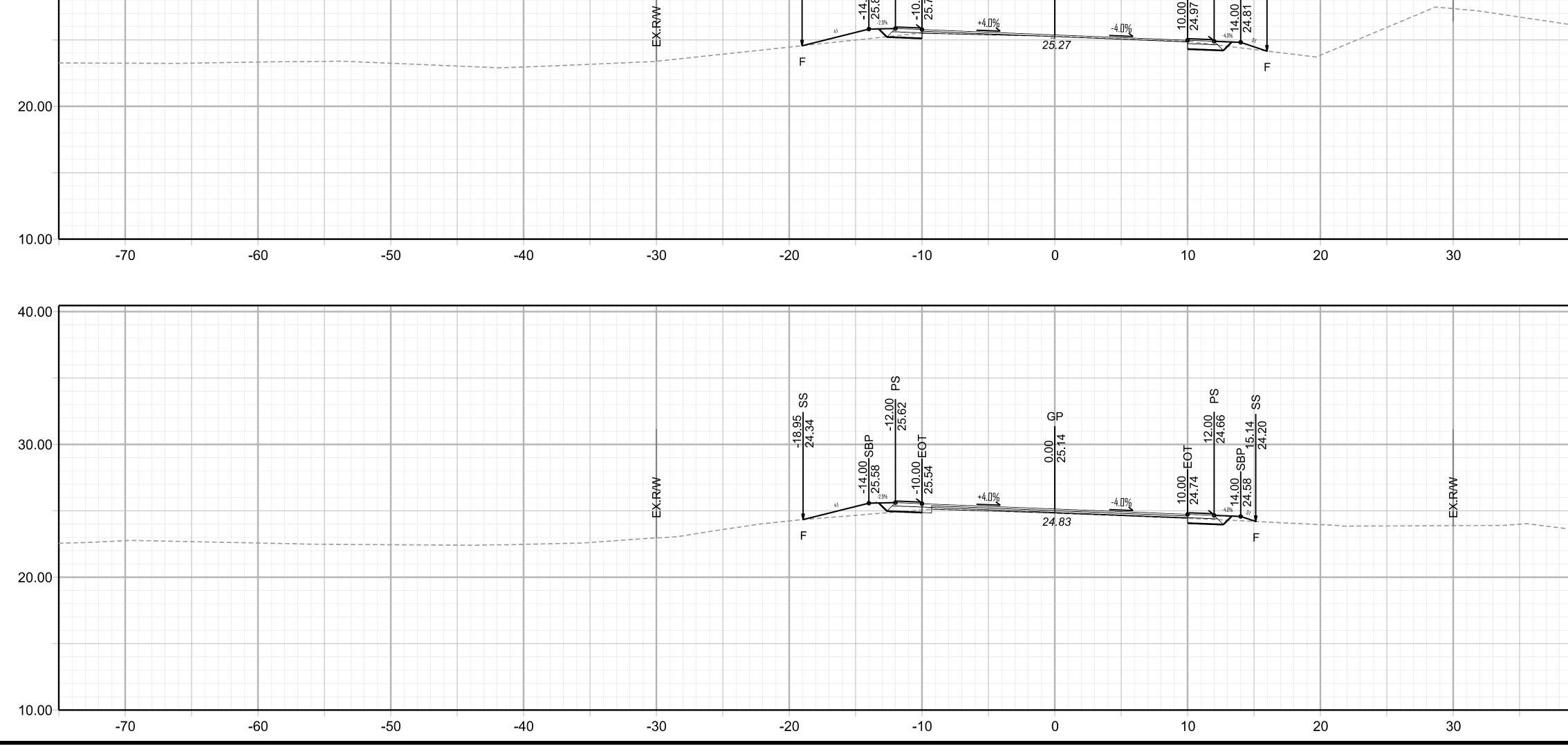
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PS

5.85

4.00 5.81

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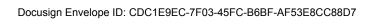
9.03 1.56 S

12.00 24.89

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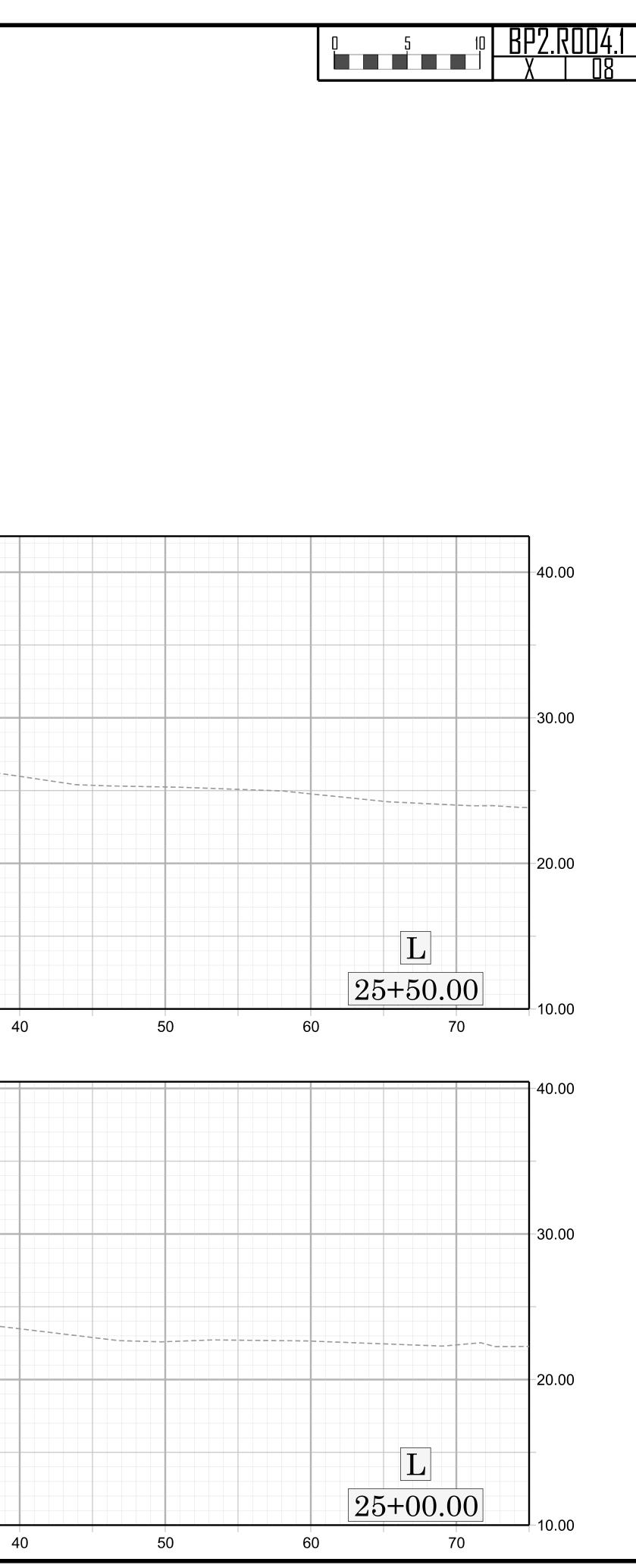
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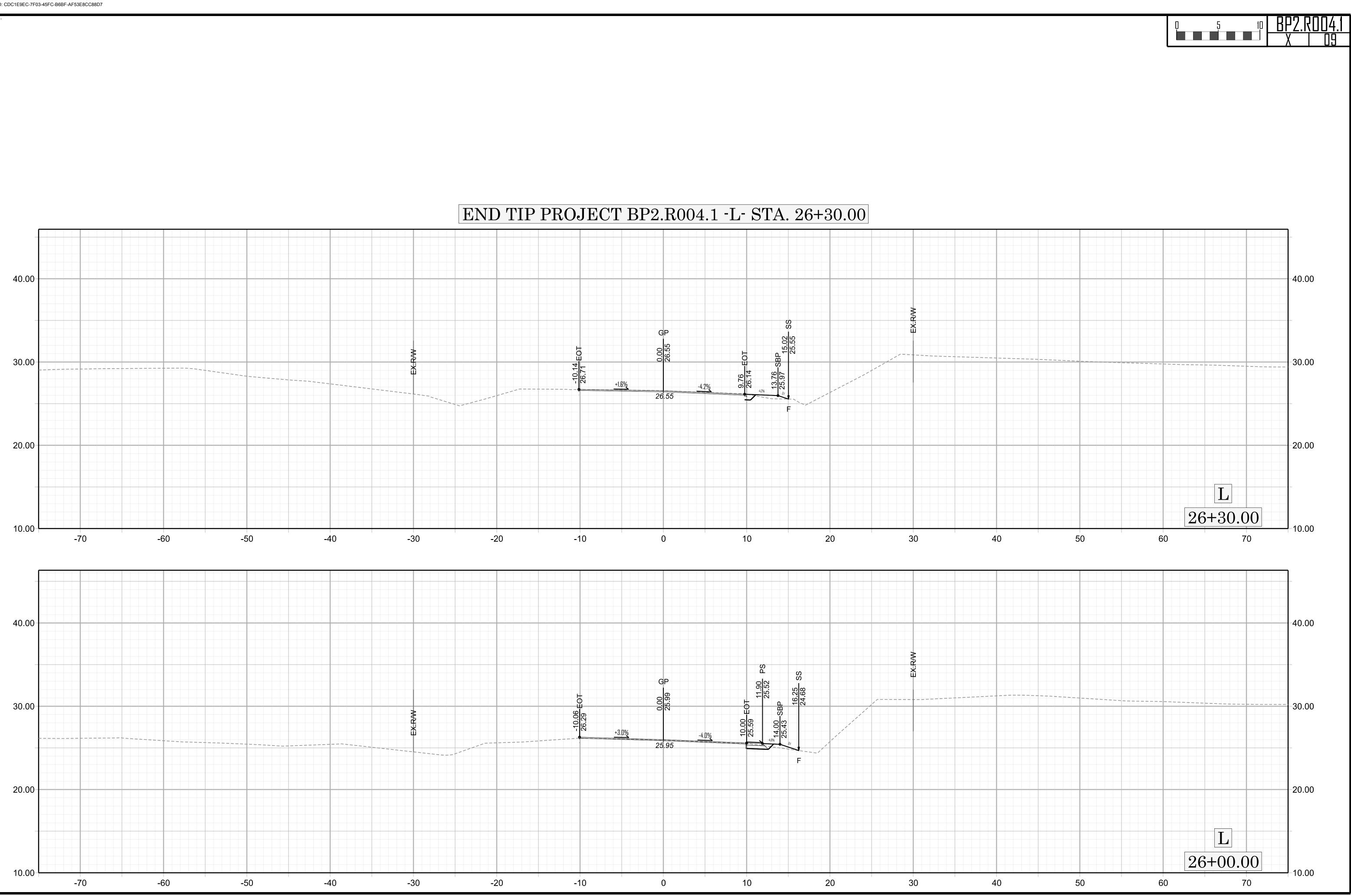
5.98 4.15

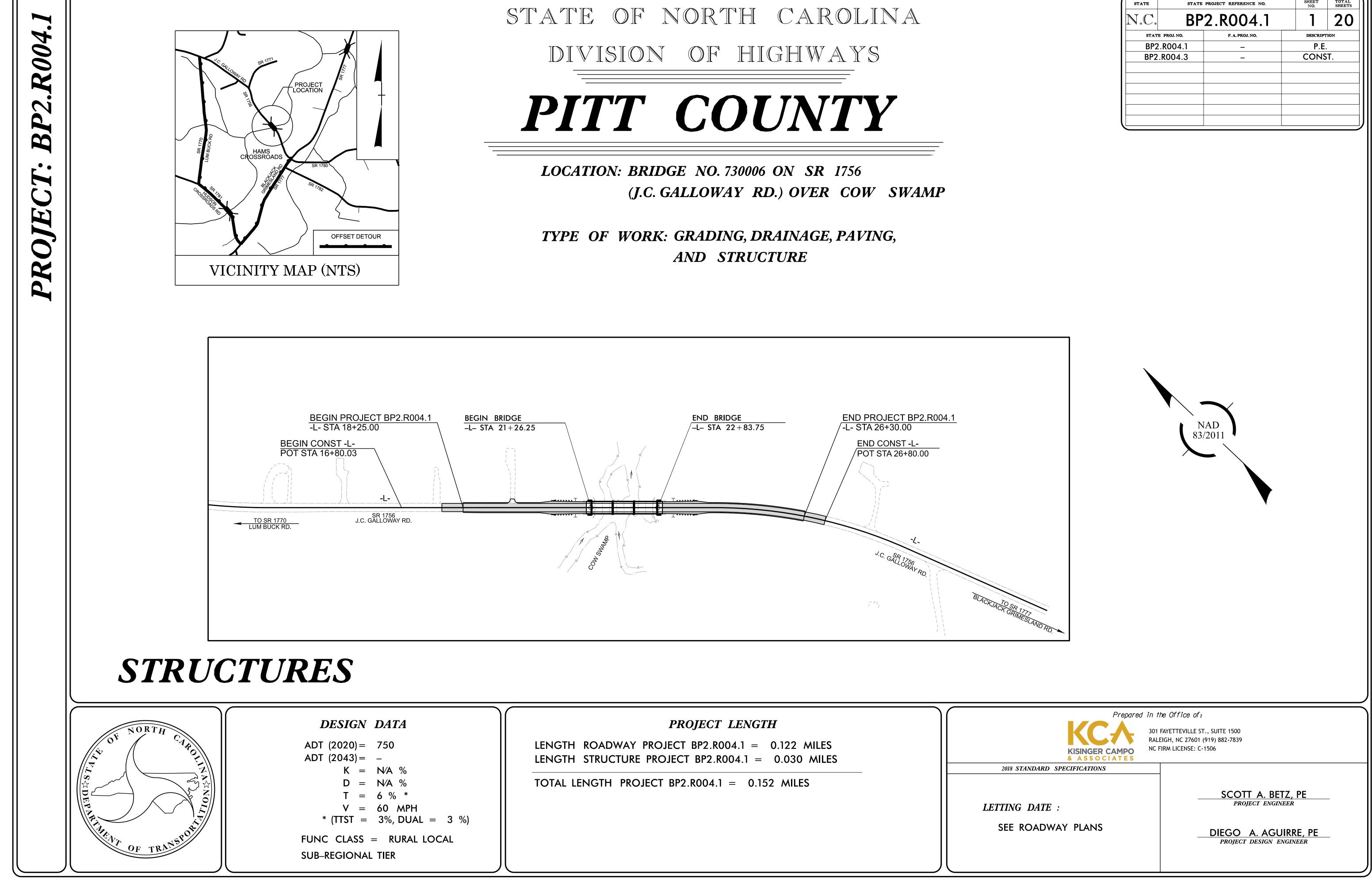


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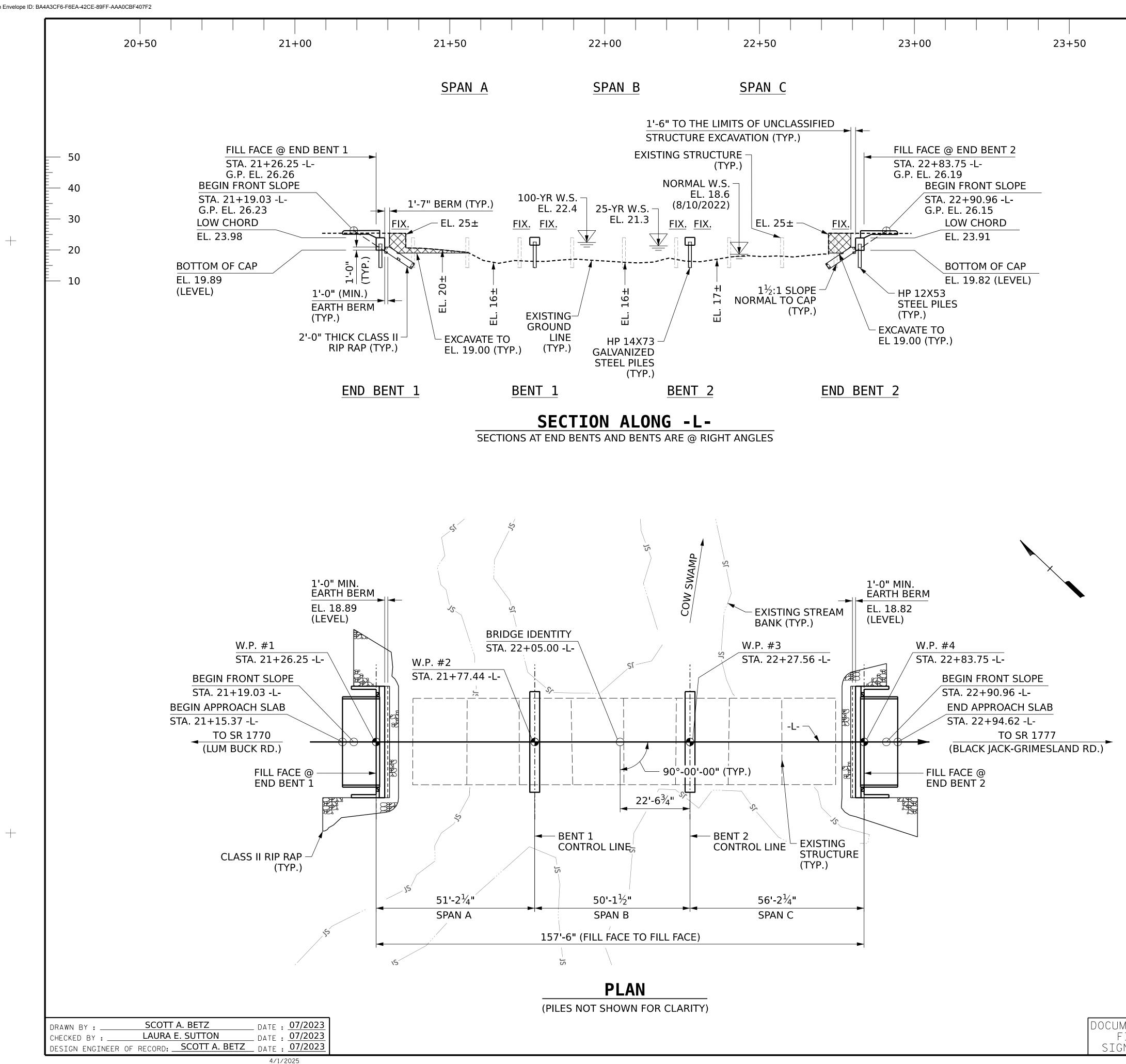






PROJECT LENGTH	
LENGTH ROADWAY PROJECT BP2.R004.1 = 0.122 MILES LENGTH STRUCTURE PROJECT BP2.R004.1 = 0.030 MILES	
TOTAL LENGTH PROJECT BP2.R004.1 = 0.152 MILES	2018
	SEE

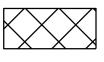
STATE	STATE	PROJECT REFERENCE NO.	SHEET NO.					
N.C.	BP	2.R004.1	1	20				
STATE F	PROJ. NO.	F. A. PROJ. NO.	DESCRIPT	ION				
BP2.F	2004.1	_	P.E.					
BP2.R	.004.3	_	CONS	T.				



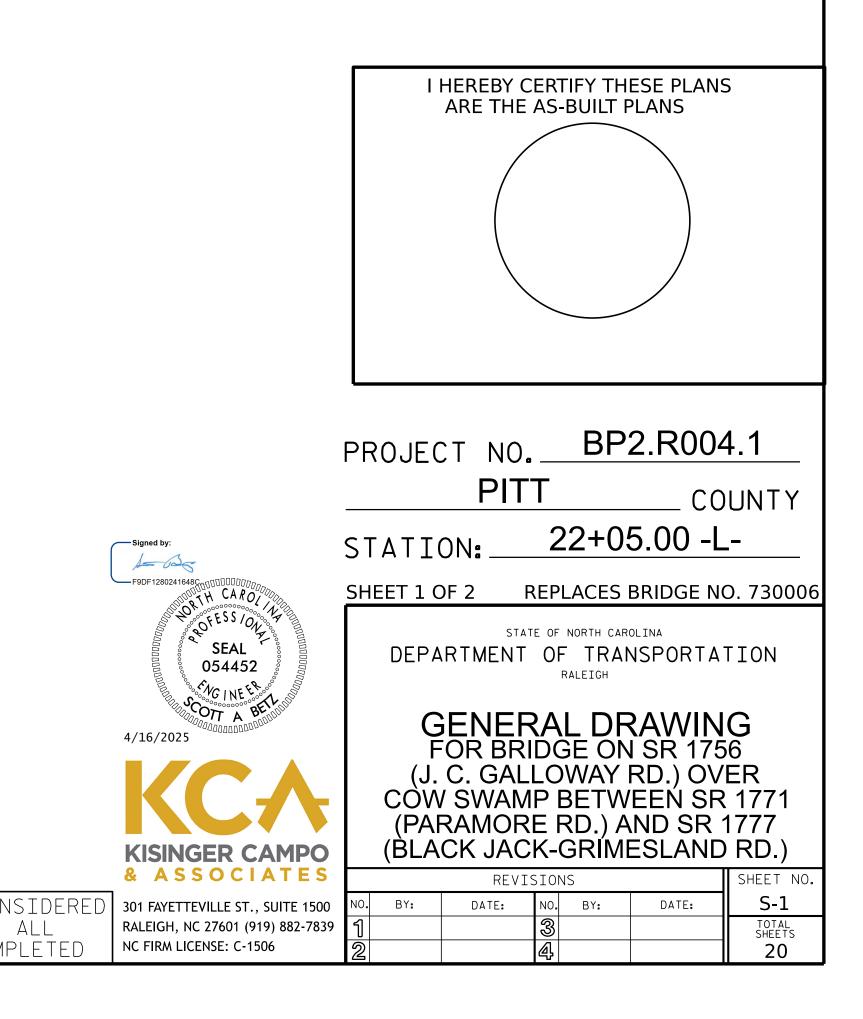
DOCUMENT	NOT	CON	ISIDE
FINAL	UNL	ESS	ALL
SIGNATU	res	COM	PLET



VERTICAL CURVE DATA 21+91.00 -L-PI = 26.70 EL. = VC = 200'



UNCLASSIFIED STRUCTURE EXCAVATION



							Driven Piles			Predrilling for Piles **		Drilled-In Piles				
End Bent / Bent No, Pile(s) #(-#) (e.g., "Bent 1, Piles 1-5")	Number of Piles per Line	Factored Resistance per Pile KIPS	Pile Cut-Off (Top of Pile) Elevation FT	Estimated Pile Length per Pile FT	Scour Critical Elevation FT	Minimum Pile Tip (Tip No Higher Than) Elevation FT	Required Driving Resistance (RDR)* per pile KIPS	Pile Redrives Quantity EACH	Predrilling Length per Pile LIN FT	Predrilling Elevation (Elevation Not To Predrill Below) FT	Maximum Predrilling Diameter INCHES	Pile Excavation (Bottom of Hole) Elevation FT	Pile Excavation Not In Soil per Pile LIN FT	Pile Excavation In Soil per Pile LIN FT		
End Bent 1 (Piles 1-5)	5	170	21.89	65			290	2								
Bent 1 (Piles 1-7)	7	240	22.52	70	7.00	-15.00	410	4								
Bent 2 (Piles 1-7)	7	240	22.51	70	7.00	-15.00	410	4								
End Bent 2 (Piles 1-5)	5	170	21.82	65			290	2								
TOTAL QUANTITY:								12								

Factored Resistance + Factored Drag Load + Factored Dead Load + Nominal Drag Load Resistance + Nominal Resistance from Scourable Material * RDR Dynamic Resistance Factor

** Predrilling for Piles is required for end bents/bents with a predrilling length and at the Contractor's option for end bents/bents with predrilling information but no predrilling length.

PILE DESIGN INFORMATION

(Blank entries indicate item is not applicable to structure)

End Bent / Bent No, Pile(s) #(-#) (e.g., "Bent 1, Piles 1-5")	Factored Axial Load per Pile KIPS	Factored Drag Load per Pile KIPS	Factored Dead Load * per Pile KIPS	Dynamic Resistance Factor	Nominal Drag Resistance per Pile KIPS	Nominal Scour Resistance per Pile KIPS
End Bent 1 (Piles 1-5)	162			0.60		
Bent 1 (Piles 1-7)	236			0.60		8
Bent 2 (Piles 1-7)	236			0.60		4
End Bent 2 (Piles 1-5)	170			0.60		

* Factored Dead Load is factored weight of pile above the ground line.

NOTES:

1. The Pile Foundation Tables are based on the bridge substructure design and foundation recommendations sealed by a North Carolina Professional Engineer (Thein Tun Zan, #030943) on 03-26-2025. 2. Total Pile Driving Equipment Setup quantity (not shown in Pile Foundation Tables) equals the number of driven piles, i.e., the number of piles with a Required Driving Resistance.

3. The Engineer may adjust the quantity for DPT Testing and Pipe Pile Plates when necessary.

4. For Piles, see Section 450 of the Standard Specifications.

5. It has been estimated that a hammer with an equivalent rated energy in the range of 30,000 FT-LBS per blow to 60,000 FT-LBS per blow will be required to drive piles at End Bent No. 1, Bent No. 1, Bent No. 2, and End Bent No. 2. This estimated energy range does not release the contractor from providing driving equipment in accordance with Subarticle 450-3(D)(2) of the standard specifications.

SUMMARY OF PILE INFORMATION/INSTALLATION

(Blank entries indicate item is not applicable to structure)

SUMMARY OF DPT/PILE ORDER LENGTHS

(Blank entries indicate item is not applicable to structure)

Dynamic Pile Testing (DPT)										
End Bent / Bent No (e.g., "Bent 1 - Bent 3")	DPT Test Pile Length FT	DPT Testing Quantity EACH								
End Bent 1 (Piles 1-5)	70									
Bent 1 (Piles 1-7)	75	1								
Bent 2 (Piles 1-7)	75	1								
End Bent 2 (Piles 1-5)	70									
_										
TOTAL QUANTITY:		1								

Pile Order Lengths for C	concrete Piles							
End Bent / Bent No (e.g., "Bent 1 - Bent 3")	Pile Order Length Basis* EST or DPT							
EST = Pile order lengths from estimated pile lengths; DPT = Pile order lengths based on								

Dynamic Pile Testing. For groups of end bents/bents with pile order lengths based on DPT testing, the first end bent/bent no. listed for each group is the representative end bent/bent with the DPT.

PROJECT NO. <u>BP2.R004.1 (SF-730006)</u>

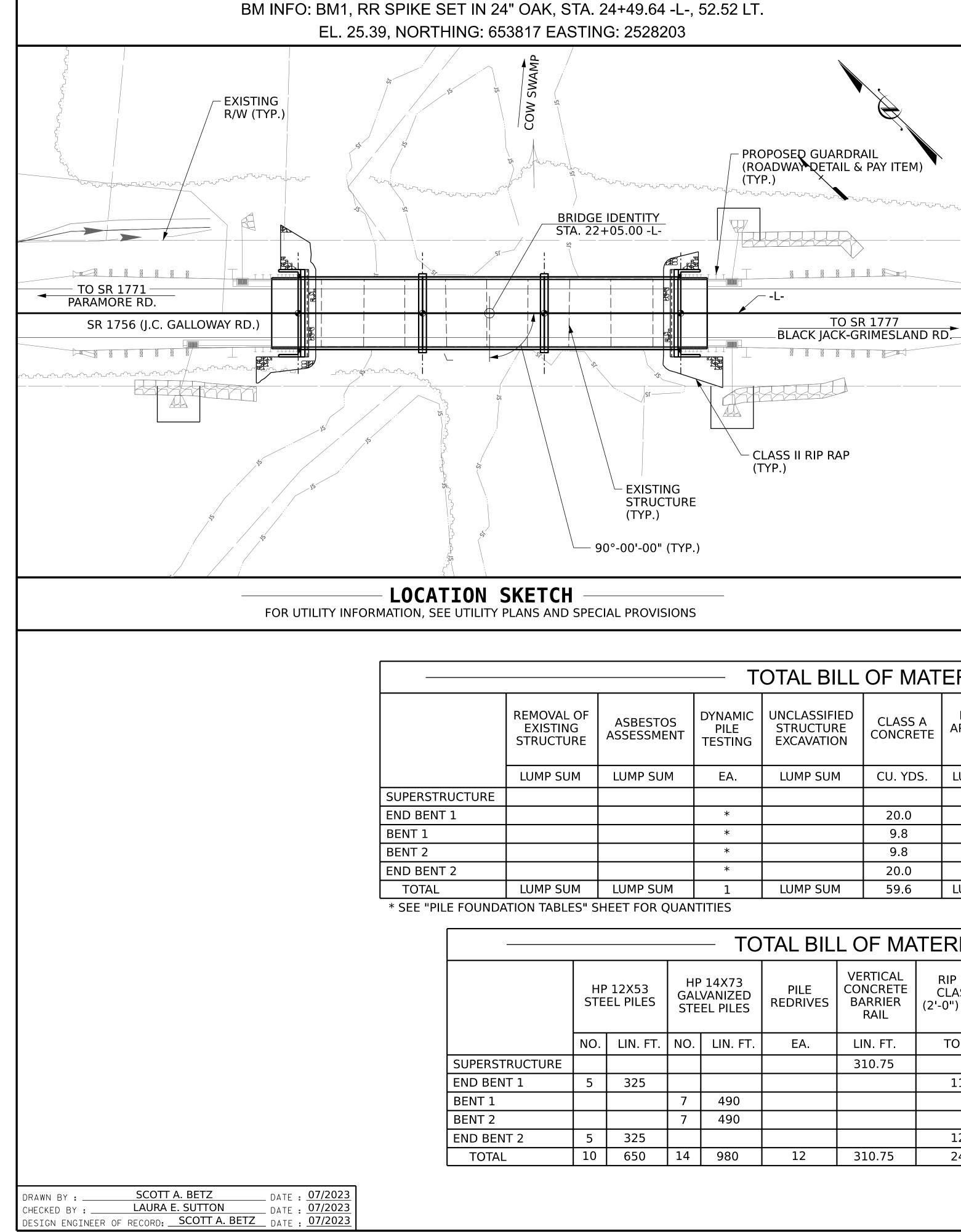
PITT

STATION:

22+05.00 -L-

COUNTY

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH SEAL PILE 054452 FOUNDATION OTT A TABLES -Signed by: A= 03g 4/16/2025 F9DF12802416480 DATE SIGNATURE SHEET NO. REVISIONS S-2 DOCUMENT NOT CONSIDERED NO. BY: DATE: NO. BY: DATE: TOTAL 3 SHEETS FINAL UNLESS ALL SIGNATURES COMPLETED 4 20



GENERAL NOTES

ASSUMED LIVE LOAD = HL-93 OR ALTERNATE LOADING

THIS BRIDGE HAS BEEN DESIGNED IN ACCORDANCE WITH THE **REQUIREMENTS OF THE AASHTO LRFD BRIDGE DESIGN** SPECIFICATIONS.

THIS BRIDGE IS LOCATED IN SEISMIC ZONE 1.

FOR OTHER DESIGN DATA AND GENERAL NOTES, SEE SHEET SN.

FOR SUBMITTAL OF WORKING DRAWINGS, SEE SPECIAL PROVISIONS.

FOR FALSEWORK AND FORMWORK, SEE SPECIAL PROVISIONS.

FOR CRANE SAFETY, SEE SPECIAL PROVISIONS.

FOR GROUT FOR STRUCTURES, SEE SPECIAL PROVISIONS.

FOR EROSION CONTROL MEASURES, SEE EROSION CONTROL PLANS.

THIS BRIDGE HAS BEEN DESIGNED IN ACCORDANCE WITH "HEC-18 -EVALUATING SCOUR AT BRIDGES".

THE MATERIAL SHOWN IN THE CROSS-HATCHED AREA ON SHEET S-1 SHALL BE EXCAVATED FOR A DISTANCE OF APPROXIMATELY 30 FT. EACH SIDE OF THE CENTERLINE ROADWAY AS DIRECTED BY THE ENGINEER. THIS WORK WILL BE PAID FOR AT THE CONTRACT LUMP SUM PRICE FOR UNCLASSIFIED STRUCTURE EXCAVATION. SEE SECTION 412 OF THE STANDARD SPECIFICATIONS.

THE EXISTING STRUCTURE CONSISTING OF 8 SPANS: 1 @ 17'-6", 3 @ 17'-0", 1 @ 17'-2", 1 @ 16'-8", 1 @ 17'-0" AND 1 @ 17'-7", CONSISTING OF A REINFORCED CONCRETE DECK ON TIMBER JOISTS WITH A CLEAR ROADWAY OF 28'-0" SHALL BE REMOVED. THE EXISTING BRIDGE IS PRESENTLY NOT POSTED FOR LOAD LIMIT. SHOULD THE STRUCTURAL INTEGRITY OF THE BRIDGE DETERIORATE DURING CONSTRUCTION OF THE PROPOSED BRIDGE, A LOAD LIMIT MAY BE POSTED AND MAY BE REDUCED AS FOUND NECESSARY DURING THE LIFE OF THE PROIECT.

	— Т	OTAL BILL	OF MAT	ERIAL -			
TOS MENT TESTING		UNCLASSIFIED STRUCTURE EXCAVATION	CLASS A CONCRETE BRIDGE APPROAC SLABS		REINFORCING STEEL	PILE DRIVING EQUIPMENT SETUP FOR HP 12X53 STEEL PILES	PILE DRIVING EQUIPMENT SETUP FOR HP 14X73 GALVANIZED STEEL PILES
5UM	EA.	LUMP SUM	CU. YDS.	LUMP SUM	LBS.	EA.	EA.
	*		20.0		2,449	5	
	*		9.8		1,932		7
	*		9.8		1,932		7
	*		20.0		2,449	5	
SUM	1	LUMP SUM	59.6	LUMP SUM	8,762	10	14
		-					

	— то	TAL BIL	L OF MA	TERIAL				
HP 14X73 GALVANIZED STEEL PILES		PILE REDRIVES	VERTICAL CONCRETE BARRIER RAIL	RIP RAP CLASS II (2'-0") THICK	GEOTEXTILE FOR DRAINAGE	ELASTOMERIC BEARINGS	PRES CO	" X 1'-9" STRESSED NCRETE ED SLABS
NO.	LIN. FT.	EA.	LIN. FT.	TONS	SQ. YDS.	LUMP SUM	NO.	LIN. FT.
			310.75			LUMP SUM	30	1,550
				114	233			
7	490							
7	490							
				127	258			
14	980	12	310.75	241	491	LUMP SUM	30	1,550

THE SUBSTRUCTURE OF THE EXISTING BRIDGE INDICATED ON THE PLANS IS FROM THE BEST INFORMATION AVAILABLE, THIS INFORMATION IS SHOWN FOR CONVENIENCE OF THE CONTRACTOR. THE CONTRACTOR SHALL HAVE NO CLAIM WHATSOEVER AGAINST THE DEPARTMENT OF TRANSPORTATION FOR ANY DELAYS OR ADDITIONAL COST INCURRED BASED ON DIFFERENCES BETWEEN THE EXISTING BRIDGE SUBSTRUCTURE SHOWN ON THE PLANS AND THE ACTUAL CONDITIONS AT THE PROJECT SITE.

REMOVAL OF THE EXISTING BRIDGE SHALL BE PERFORMED IN A MANNER THAT PREVENTS DEBRIS FROM FALLING INTO THE WATER. THE CONTRACTOR SHALL SUBMIT DEMOLITION PLANS FOR REVIEW AND REMOVE THE BRIDGE IN ACCORDANCE WITH ARTICLE 402-2 OF THE STANDARD SPECIFICATIONS.

FOR ASBESTOS ASSESSMENT, SEE SPECIAL PROVISIONS.

THE SCOUR CRITICAL ELEVATION FOR BENTS 1 AND 2 IS ELEVATION 7FT. SCOUR CRITICAL ELEVATIONS ARE USED TO MONITOR POSSIBLE SCOUR PROBLEMS DURING THE LIFE OF THE STRUCTURE.

PAVEMENT ALONG THE TRANSVERSE CENTERLINE OF ALL CAPS SHALL BE SAW CUT TO A DEPTH OF 3/4" ±, CLEANED, AND FILLED WITH AN APPROVED ASPHALT SEALANT IN ACCORDANCE WITH SECTION 1028 OF THE STANDARD SPECIFICATIONS. PAYMENT FOR THIS WORK SHALL BE INCIDENTAL TO THE CONTRACT UNIT PRICE FOR VARIOUS PAY ITEMS.

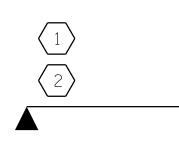
APPLY AN APPROVED EPOXY PROTECTIVE COATING TO THE TOPS OF ALL CAPS AND TO THE EXTERIOR FACES OF ALL EXTERIOR CORED SLAB UNITS. PAYMENT FOR THIS WORK SHALL BE INCIDENTAL TO OTHER PAY ITEMS IN THE CONTRACT AND NO SEPARATE PAYEMENT WILL BE MADE FOR THIS WORK.

USE A TYPE 4A FLEXIBLE AND MOISTURE INSENSITIVE EPOXY COATING IN ACCORDANCE WITH SECTION 1081 OF THE STANDARD SPECIFICATIONS. PROVIDE A TYPE 3 MATERIAL CERTIFICATION IN ACCORDANCE WITH ARTICLE 106-3 SHOWING THAT THE EPOXY MEETS TYPE 4A REQUIREMENTS.

			A	
	DESIGN DIS FREQUENCY DESIGN HIG DRAINAGE A BASE DISCH	OF DESIGN FLOOD H WATER ELEVATION REA	1400 CFS 25 YRS.	
	OVER	TOPPING FLOOD	DATA	
	FREQUENCY	IG DISCHARGE OF OVERTOPPING FLOOD IG FLOOD ELEVATION	4982 CFS 500+ YRS. 25.1 FT.	
	SAG STA.		18+50.83 -L-	
		project no. <u> </u> PITT	2004.1	— Y
—Signed by:		STATION: 22+		
-F9DF1280241648C	H CAROLINA	SHEET 2 OF 2		
	SEAL 054452	STATE OF NORTH DEPARTMENT OF TI RALEIC	RANSPORTATION	
4/16/2025	ER CAMPO	GENERAL I FOR BRIDGE (J. C. GALLOWA COW SWAMP BE (PARAMORE RD. (BLACKJACK GRI	ON SR 1756 Y RD.) OVER [WEEN SR 1771) AND SR 1777	
301 FAYETTEN	OCIATES /ILLE ST., SUITE 1500 27601 (919) 882-7839 NSE: C-1506	REVISIONS NO. BY: DATE: NO. BY: 1 3 3 3 2 4 4 3	DATE: SHEET	} S

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											ST	RENGTH		STATE						SERVIC	Ce III LI	MIT STA	ГЕ	
				$\langle \# \rangle$					M	10ME	NT			S	HEAR					M	OMENT	-		
		VEHICLE	WEIGHT (W) (TONS)	CONTROLLING LOAD RATING	MINIMUM RATING FACTORS (RF)	TONS = W x RF	LIVE-LOAD FACTORS (7 LL)	DISTRIBUTION FACTORS (DF)	RATING FACTOR	SPAN	GIRDER LOCATION	DISTANCE FROM LEFT END OF SPAN (ft)	DISTRIBUTION FACTORS (DF)	RATING FACTOR	SPAN	GIRDER LOCATION	DISTANCE FROM LEFT END OF SPAN (ft)	LIVE-LOAD FACTORS (7 LL)	DISTRIBUTION FACTORS (DF)	RATING FACTOR	SPAN	GIRDER LOCATION	DISTANCE FROM LEFT END OF SPAN (ft)	
	_	HL-93 (INVENTORY)	N/A		1.394		1.75	0.276	1.57	50'	EL	24.5	0.531	1.39	50'	EL	2.45	0.80	0.276	1.44	50'	EL	24.5	
DESIC		HL-93 (OPERATING)	N/A		1.807		1.35	0.276	2.03	50'	EL	24.5	0.531	1.81	50'	EL	2.45	N/A						
LOAI		HS-20 (INVENTORY)	36.000	2	1.667	60.007	1.75	0.276	1.95	50'	EL	24.5	0.531	1.67	50'	EL	2.45	0.80	0.276	1.79	50'	EL	24.5	_
		HS-20 (OPERATING)	36.000		2.161	77.787	1.35	0.276	2.52	50'	EL	24.5	0.531	2.16	50'	EL	2.45	N/A						
		SNSH	13.500		3.635	49.079	1.4	0.276	4.95	50'	EL	24.5	0.531	4.70	50'	EL	2.45	0.80	0.276	3.64	50'	EL	24.5	
	Щ	SNGARBS2	20.000		2.871	57.420	1.4	0.276	3.91	50'	EL	24.5	0.531	3.42	50'	EL	2.45	0.80	0.276	2.87	50'	EL	24.5	
	HICLE	SNAGRIS2	22.000		2.778	61.109	1.4	0.276	3.78	50'	EL	19.6	0.531	3.21	50'	EL	2.45	0.80	0.276	2.78	50'	EL	24.5	
	LE VEH (SV)	SNCOTTS3	27.250		1.814	49.418	1.4	0.276	2.47	50'	EL	24.5	0.531	2.36	50'	EL	2.45	0.80	0.276	1.81	50'	EL	24.5	
		SNAGGRS4	34.925		1.577	55.063	1.4	0.276	2.15	50'	EL	24.5	0.531	2.01	50'	EL	2.45	0.80	0.276	1.58	50'	EL	24.5	
		SNS5A	35.550		1.537	54.657	1.4	0.276	2.09	50'	EL	24.5	0.531	2.07	50'	EL	2.45	0.80	0.276	1.54	50'	EL	24.5	
		SNS6A	39.950		1.438	57.430	1.4	0.276	1.96	50'	EL	24.5	0.531	1.91	50'	EL	2.45	0.80	0.276	1.44	50'	EL	24.5	
LEGAL		SNS7B	42.000		1.370	57.540	1.4	0.276	1.87	50'	EL	24.5	0.531	1.91	50'	EL	2.45	0.80	0.276	1.37	50'	EL	24.5	
LOAD		TNAGRIT3	33.000		1.761	58.118	1.4	0.276	2.40	50'	EL	24.5	0.531	2.25	50'	EL	2.45	0.80	0.276	1.76	50'	EL	24.5	
	<u>۲</u>	TNT4A	33.075		1.777	58.759	1.4	0.276	2.42	50'	EL	24.5	0.531	2.17	50'	EL	2.45	0.80	0.276	1.78	50'	EL	24.5	
	LER	TNT6A	41.600		1.480	61.558	1.4	0.276	2.01	50'	EL	24.5	0.531	2.08	50'	EL	2.45	0.80	0.276	1.48	50'	EL	24.5	
	RAI ST)	TNT7A	42.000		1.502	63.087	1.4	0.276	2.05	50'	EL	24.5	0.531	1.94	50'	EL	2.45	0.80	0.276	1.50	50'	EL	24.5	
		TNT7B	42.000		1.566	65.773	1.4	0.276	2.13	50'	EL	24.5	0.531	1.84	50'	EL	2.45	0.80	0.276	1.57	50'	EL	24.5	
	FRUCK TRACTOR SEMI-TRAILER (TTST)	TNAGRIT4	43.000		1.486	63.902	1.4	0.276	2.02	50'	EL	24.5	0.531	1.77	50'	EL	2.45	0.80	0.276	1.49	50'	EL	24.5	
		TNAGT5A	45.000		1.388	62.470	1.4	0.276	1.89	50'	EL	24.5	0.531	1.80	50'	EL	2.45	0.80	0.276	1.39	50'	EL	24.5	\square
		TNAGT5B	45.000	3	1.360	61.206	1.4	0.276	1.85	50'	EL	24.5	0.531	1.68	50'	EL	2.45	0.80	0.276	1.36	50'	EL	24.5	
EMERG	ENCY	EV2	28.750		2.154	61.929	1.3	0.276	2.97	50'	EL	24.5	0.531	2.50	50'	EL	5.50	0.80	0.276	2.15	50'	EL	24.5	
VEHICL	.E (EV)	EV3	43.000	$\langle 4 \rangle$	1.392	59.852	1.3	0.276	1.92	50'	EL	24.5	0.531	1.69	50'	EL	5.50	0.80	0.276	1.39	50'	EL	24.5	



LRFR SUMMARY FOR SPANS A & B

DRAWN BY : MAA 1/08 Checked by : GM/d1 2/08	REV. 11/12/08RR REV. 10/1/11 REV. 04/23		MAA/GM MAA/GM BNB/AAI
DRAWN BY : SCOTT A.	BETZ	DATE :	07/2023
CHECKED BY : LAURA E.			07/2023
DESIGN ENGINEER OF RECORD:	SCOTT A. BETZ	DATE :	07/2023

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	FINAL UNLESS ALL	_
	SIGNATURES COMPLE	TE[
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DESIGN LOAD RATING FACTORS	LIMIT STATE	γDC	γdW
	STRENGTH I	1.25	1.50
	SERVICE III	1.00	1.00

NOTES:

MINIMUM RATING FACTORS ARE BASED ON THE STRENGTH I AND SERVICE III LIMIT STATES.

ALLOWABLE STRESSES FOR SERVICE III LIMIT STATE ARE AS REQUIRED FOR DESIGN.

COMMENTS:

- 1.
- 2.
- 3.
- 4.

$\langle \# \rangle$ CONTROLLING LO	DAD RATING
$\langle 1 \rangle$ design load rating (HL-93)
$\langle 2 \rangle$ design load rating (HS-20)
$\langle 3 \rangle$ LEGAL LOAD RATING * ³	*
4 EMERGENCY VEHICLE L	OAD RATING * *
* * SEE CHART FOR VEHICI	LE TYPE
GIRDER LOCA	TION
I - INTERIOR GIRDER EL - EXTERIOR LEFT GIRDEI ER- EXTERIOR RIGHT GIRD	
F9DF1280241648C	PROJECT NO. BP2.R004.1 PITT COUNTY STATION: 22+05.00 -L-
A/16/2025 SEAL 054452 A/16/2025 KISINGER CAMPO & ASSOCIATES 301 FAYETTEVILLE ST., SUITE 1500	DEPARTMENT OF TRANSPORTATION RALEIGH STANDARD LRFR SUMMARY FOR 50' CORED SLAB UNIT 90° SKEW (NON-INTERSTATE TRAFFIC) REVISIONS NO. BY: DATE: NO. BY: DATE: SHEET NO. S-4
RALEIGH, NC 27601 (919) 882-7839 ED NC FIRM LICENSE: C-1506	1 3 TOTAL SHEETS 2 4 20
	STD. NO. 21LRFR1_90S_50L

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							STRENGTH I LIMIT STATE SERVICE III LIMIT STATE MOMENT SHEAR								MIT STA	ГЕ									
									Μ	IOME	NT			S	HEAR	k i				M	OMENT]	
	-	VEHICLE	WEIGHT (W) (TONS)	CONTROLLING LOAD RATING	MINIMUM RATING FACTORS (RF)	TONS = W × RF	LIVE-LOAD FACTORS (DISTRIBUTION FACTORS (DF)	RATING FACTOR	SPAN	GIRDER LOCATION	DISTANCE FROM LEFT END OF SPAN (ft)	DISTRIBUTION FACTORS (DF)	RATING FACTOR	SPAN	GIRDER LOCATION	DISTANCE FROM LEFT END OF SPAN (ft)	LIVE-LOAD FACTORS (DISTRIBUTION FACTORS (DF)	RATING FACTOR	SPAN	GIRDER LOCATION	DISTANCE FROM LEFT END OF SPAN (ft)		
		HL-93 (INVENTORY)	N/A		1.055		1.75	0.275	1.23	55'	EL	27	0.523	1.23	55'	EL	5.4	0.80	0.275	1.05	55'	EL	27		
DESI		HL-93 (OPERATING)	N/A		1.591		1.35	0.275	1.59	55'	EL	27	0.523	1.59	55'	EL	5.4	N/A							
LOA	D	HS-20 (INVENTORY)	36.000	2	1.322	47.585	1.75	0.275	1.54	55'	EL	27	0.523	1.47	55'	EL	5.4	0.80	0.275	1.32	55'	EL	27		
		HS-20 (OPERATING)	36.000		1.900	68.396	1.35	0.275	1.99	55'	EL	27	0.523	1.90	55'	EL	5.4	N/A							
		SNSH	13.500		2.776	37.476	1.4	0.275	4.04	55'	EL	27	0.523	4.17	55'	EL	5.4	0.80	0.275	2.78	55'	EL	27		
	Щ	SNGARBS2	20.000		2.155	43.095	1.4	0.275	3.14	55'	EL	27	0.523	3.02	55'	EL	5.4	0.80	0.275	2.15	55'	EL	27		
		SNAGRIS2	22.000		2.079	45.734	1.4	0.275	3.03	55'	EL	27	0.523	2.83	55'	EL	5.4	0.80	0.275	2.08	55'	EL	27		
		VEH SV)	SNCOTTS3	27.250		1.384	37.708	1.4	0.275	2.01	55'	EL	27	0.523	2.09	55'	EL	5.4	0.80	0.275	1.38	55'	EL	27	
		(S)	SNAGGRS4	34.925		1.189	41.527	1.4	0.275	1.73	55'	EL	27	0.523	1.77	55'	EL	5.4	0.80	0.275	1.19	55'	EL	27	
	SING	SNS5A	35.550		1.160	41.255	1.4	0.275	1.69	55'	EL	27	0.523	1.82	55'	EL	5.4	0.80	0.275	1.16	55'	EL	27		
		SNS6A	39.950		1.079	43.102	1.4	0.275	1.57	55'	EL	27	0.523	1.68	55'	EL	5.4	0.80	0.275	1.08	55'	EL	27		
LEGAL		SNS7B	42.000		1.028	43.175	1.4	0.275	1.50	55'	EL	27	0.523	1.67	55'	EL	5.4	0.80	0.275	1.03	55'	EL	27		
LOAD		TNAGRIT3	33.000		1.320	43.556	1.4	0.275	1.92	55'	EL	27	0.523	1.98	55'	EL	5.4	0.80	0.275	1.32	55'	EL	27		
	ц Ц	TNT4A	33.075		1.330	43.979	1.4	0.275	1.94	55'	EL	27	0.523	1.91	55'	EL	5.4	0.80	0.275	1.33	55'	EL	27		
	LER	TNT6A	41.600		1.101	45.811	1.4	0.275	1.60	55'	EL	27	0.523	1.83	55'	EL	5.4	0.80	0.275	1.10	55'	EL	27		
	JCK TRACTOR EMI-TRAILER (TTST)	TNT7A	42.000		1.114	46.804	1.4	0.275	1.62	55'	EL	27	0.523	1.71	55'	EL	5.4	0.80	0.275	1.11	55'	EL	27		
		TNT7B	42.000		1.163	48.848	1.4	0.275	1.69	55'	EL	27	0.523	1.62	55'	EL	5.4	0.80	0.275	1.16	55'	EL	27		
	SEN	TNAGRIT4	43.000		1.101	47.330	1.4	0.275	1.60	55'	EL	27	0.523	1.56	55'	EL	5.4	0.80	0.275	1.10	55'	EL	27		
		TNAGT5A	45.000		1.031	46.405	1.4	0.275	1.50	55'	EL	27	0.523	1.58	55'	EL	5.4	0.80	0.275	1.03	55'	EL	27		
		TNAGT5B	45.000	3	1.013	45.582	1.4	0.275	1.47	55'	EL	27	0.523	1.48	55'	EL	5.4	0.80	0.275	1.01	55'	EL	27		
EMERG	SENCY	EV2	28.750		1.617	46.483	1.3	0.275	2.37	55'	EL	27	0.523	2.27	55'	EL	5.4	0.80	0.275	1.62	55'	EL	27		
VEHICL	E(EV)	EV3	43.000	$\langle 4 \rangle$	1.049	45.107	1.3	0.275	1.54	55'	EL	27	0.523	1.53	55'	EL	5.4	0.80	0.275	1.05	55'	EL	27		

LRFR SUMMARY

FOR SPAN C

DRAWN BY : MAA 1/08 CHECKED BY : GM/DI 2/08	REV.II/12/08RR REV.IO/1/II REV.04/23	MAA/GM MAA/GM BNB/AAI
DRAWN BY : SCOTT A.	BETZ DAT	E: 07/2023
CHECKED BY : LAURA E. S	DAT	E : 07/2023
DESIGN ENGINEER OF RECORD:	COTT A. BETZ DAT	E : <u>07/2023</u>

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SIGNATU	res	COM	PLET	ЕD

LOAD FACTORS:

DESIGN LOAD RATING FACTORS	LIMIT STATE	γDC	γDW
	STRENGTH I	1.25	1.50
	SERVICE III	1.00	1.00

NOTES:

MINIMUM RATING FACTORS ARE BASED ON THE STRENGTH I AND SERVICE III LIMIT STATES.

ALLOWABLE STRESSES FOR SERVICE III LIMIT STATE ARE AS REQUIRED FOR DESIGN.

COMMENTS:

1.

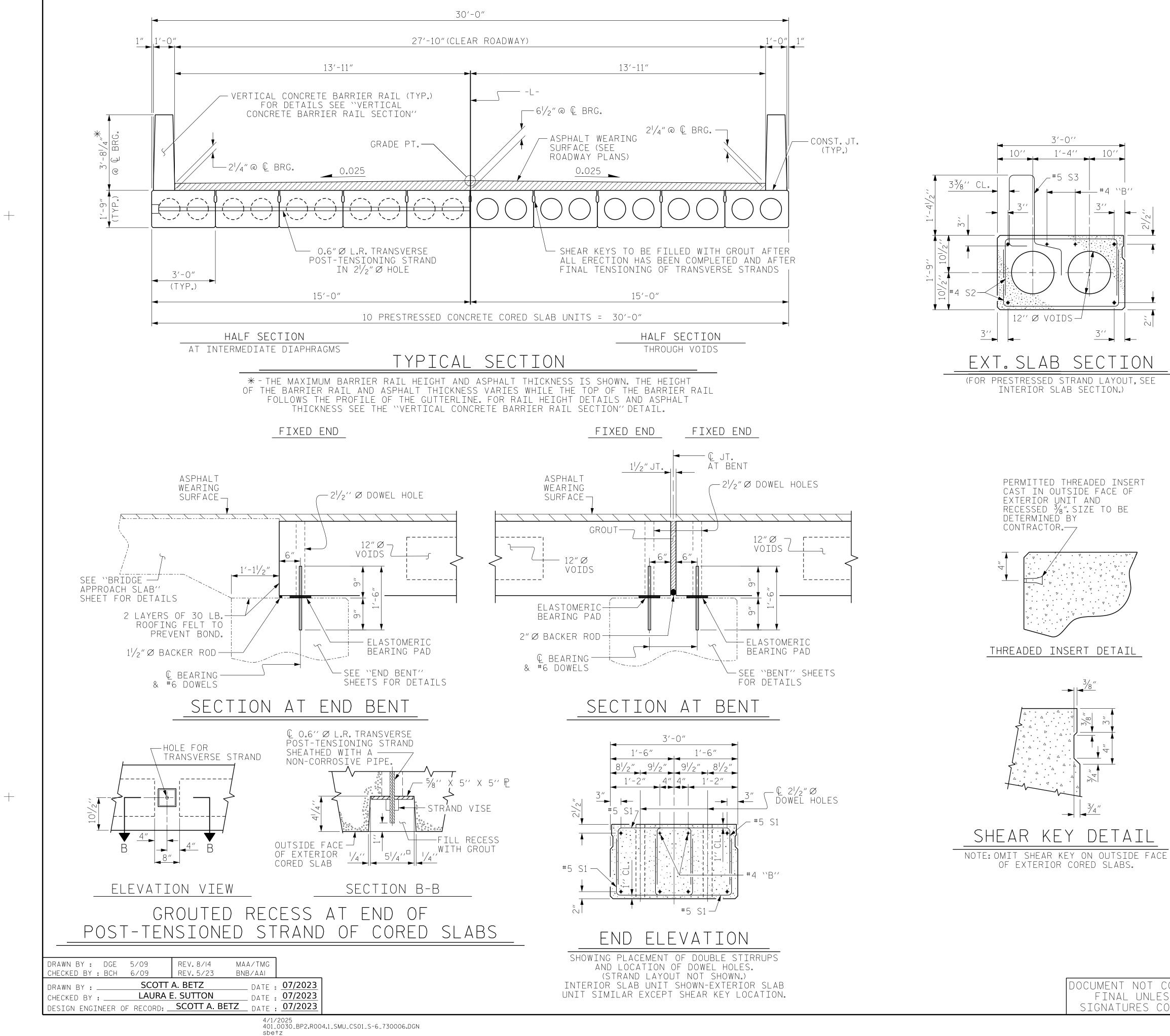
2.

3.

4.

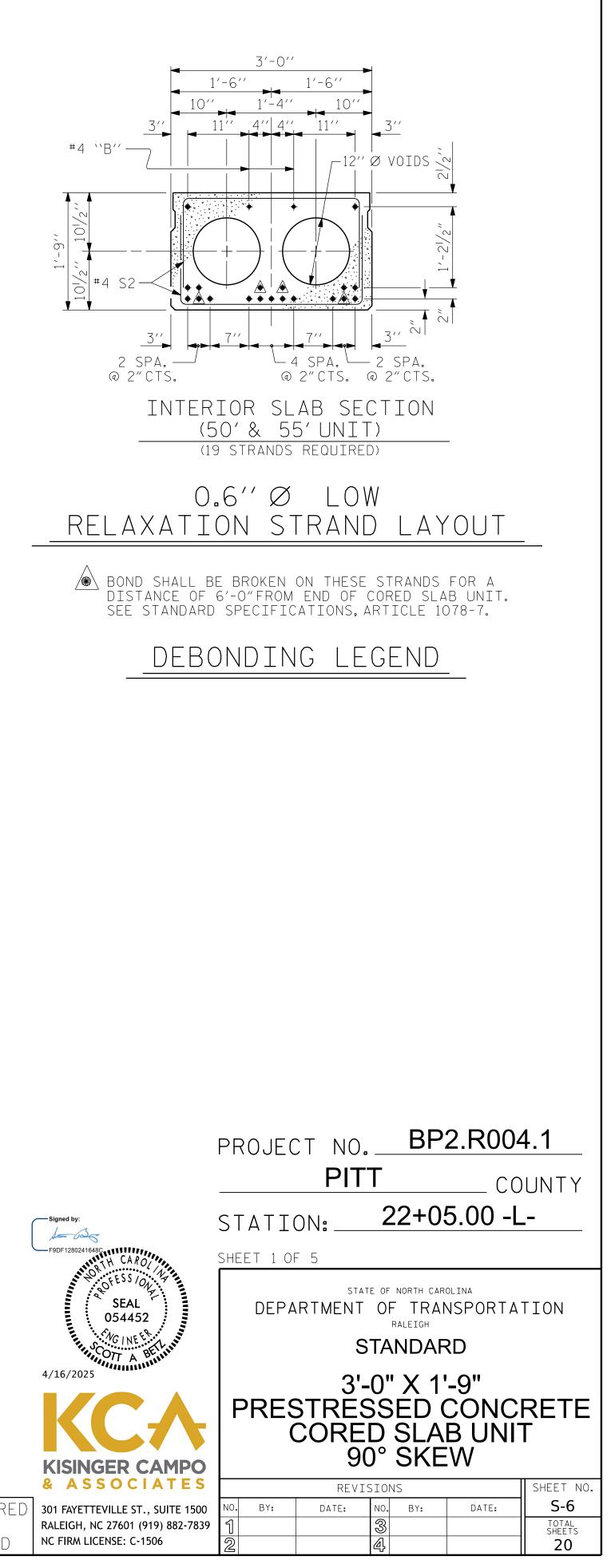
(#) CONTROLLING LOAD RATING
1 DESIGN LOAD RATING (HL-93)
2 DESIGN LOAD RATING (HS-20)
3 LEGAL LOAD RATING * *
4 EMERGENCY VEHICLE LOAD RATING **
* * SEE CHART FOR VEHICLE TYPE
GIRDER LOCATION
I - INTERIOR GIRDER
EL - EXTERIOR LEFT GIRDER
ER- EXTERIOR RIGHT GIRDER
PROJECT NO. BP2.R004.1 PITT COUNTY STATION: 22+05.00 -L-
SEAL 054452
4/16/2025 STANDARD
55' CORED SLAB UNIT
90° SKEW
KISINGER CAMPO 90° SKEW & ASSOCIATES (NON-INTERSTATE TRAFFIC) REVISIONS SHEET NO.
(NON-INTERSTATE TRAFFIC)

STD. NO. 21LRFR1_90S_55L



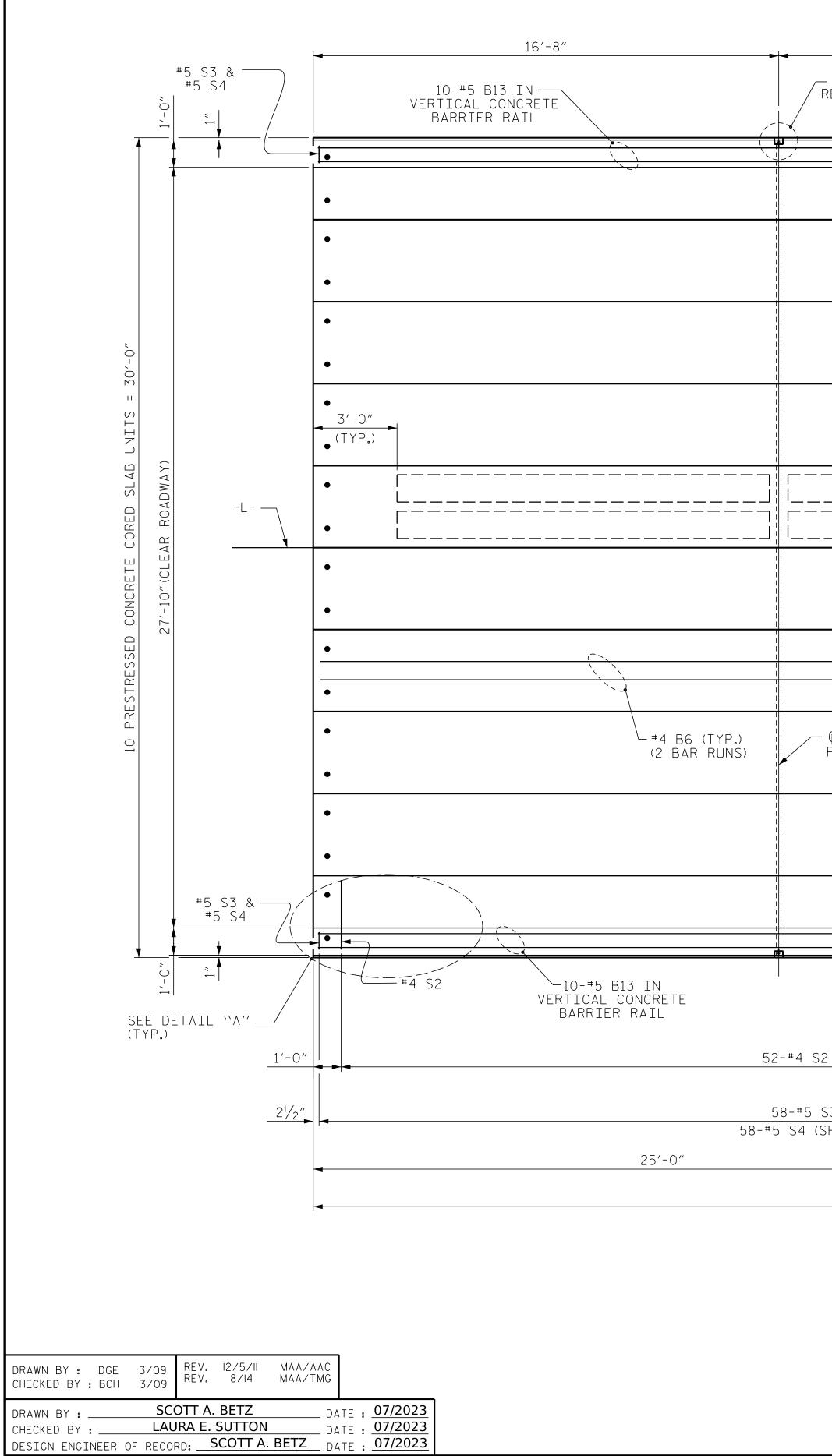
DOCUMENT NOT CONSIDERE FINAL UNLESS ALL SIGNATURES COMPLETED

10′′



STD. NO. 21" PCS2_30_90S

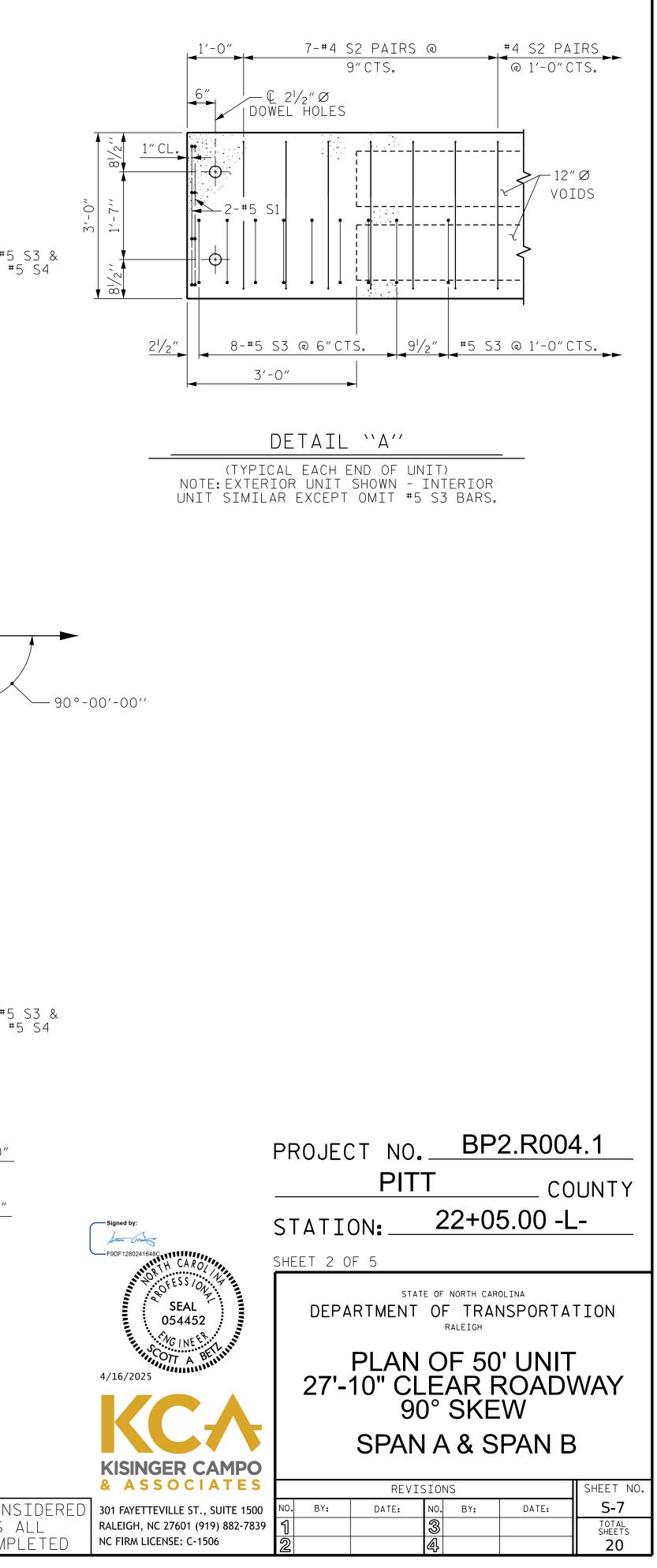
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16'-8"	▶ ◄	►	
- SEE GROUTED RECESS DETAILS (TYP.)	10-#5 B13 IN VERTICAL CONCRETE BARRIER RAIL		
GUTTERLINE -		 	−⊂ #Ę
		•	
		•	
		•	
12'' Ø VOIDS (TYP.EA.SLAB UNIT)	4" (TYP.)		
(TYP.EA. SLAB UNIT) (TYP.)		•	
		•	
SPLICE		•	
		•	
		•	
€ 0.6″ Ø L.R. TRANSVERSE POST-TENSIONING STRAND IN 2½″ Ø HOLE (TYP.)		•	
		•	
GUTTERLINE		•	
			#Ę
Q /2" EXP.JT. MAT'L.IN RAIL (TYP.)	#4 S2 —) — 10-#5 B13 IN VERTICAL CONCRETE BARRIER RAIL		
2 PAIRS (SPACE AS SHOWN IN DETAIL ``A'')(TYP.EA.UN	NIT)	. ← ►	1'-0"
53 (SPACE AS SHOWN IN DETAIL ``A'')(TYP.EA.EXT.UN] Spaced to match s3 in vertical concrete barrier	RAIL)		2 ¹ /2″
50'-0"	25'-0"		

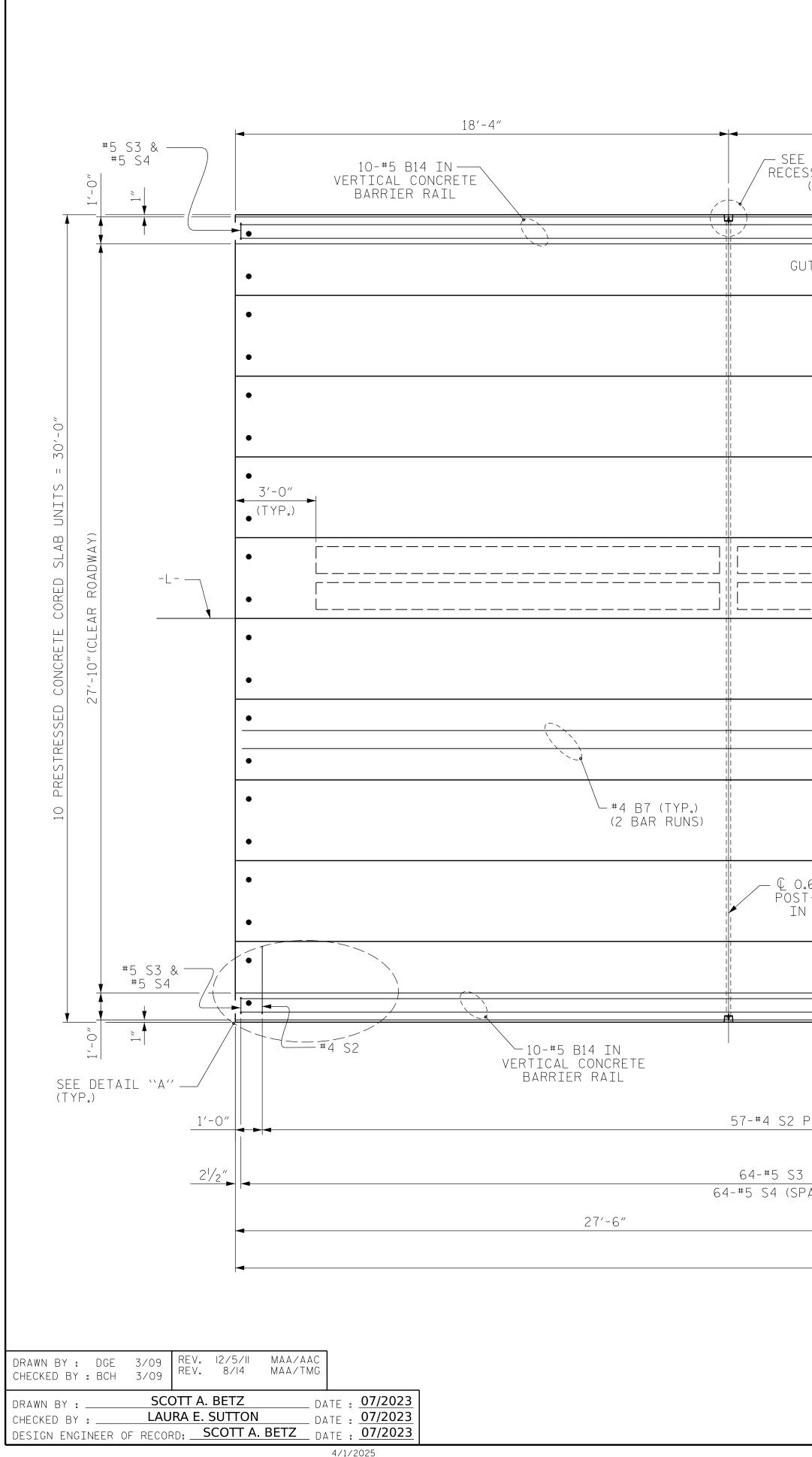
<u>Plan of unit</u>

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FINAL	UNL	ESS
SIGNATU	RES	COMF



STD. NO. 21" PCS_30_90S_50L

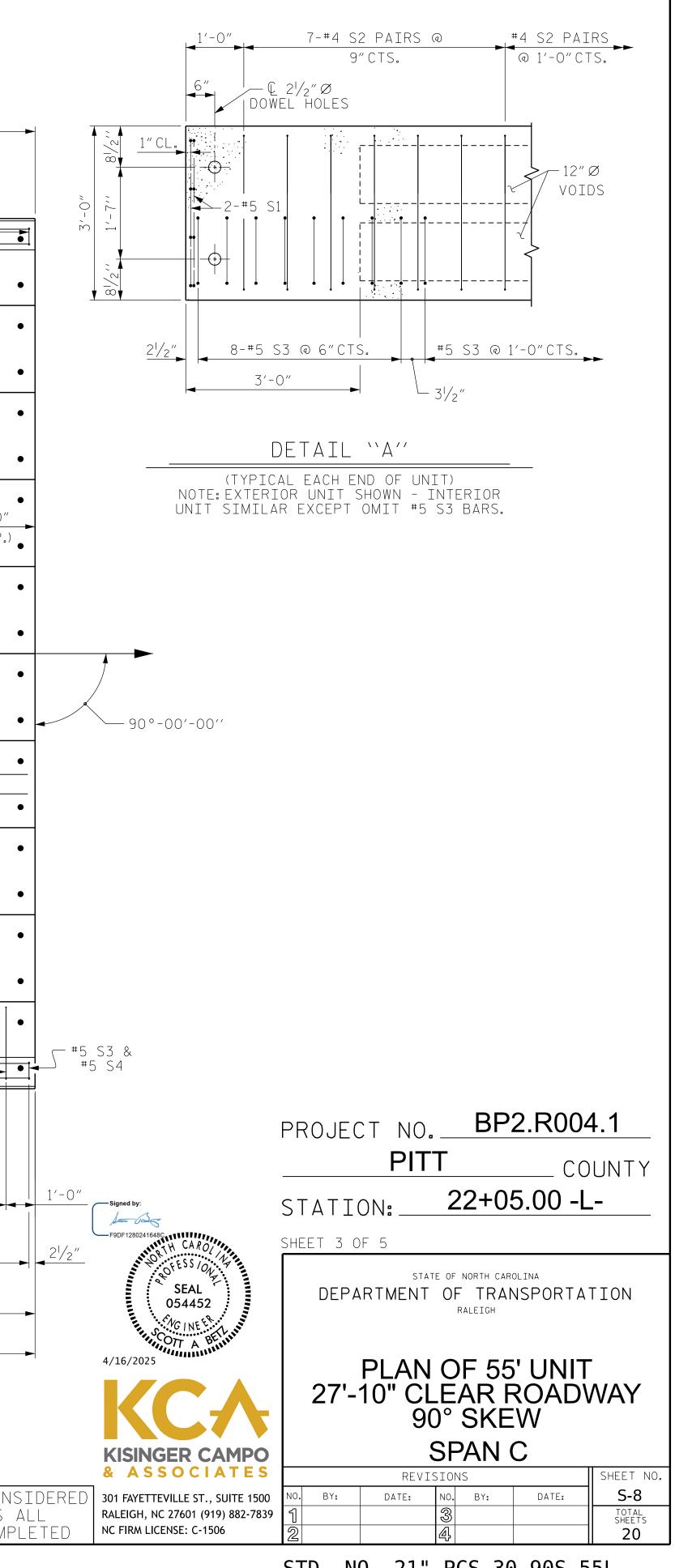
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4/1/2025 401_0040_BP2.R004.1_SMU_CS03_S-8_730006.DGN sbe†z

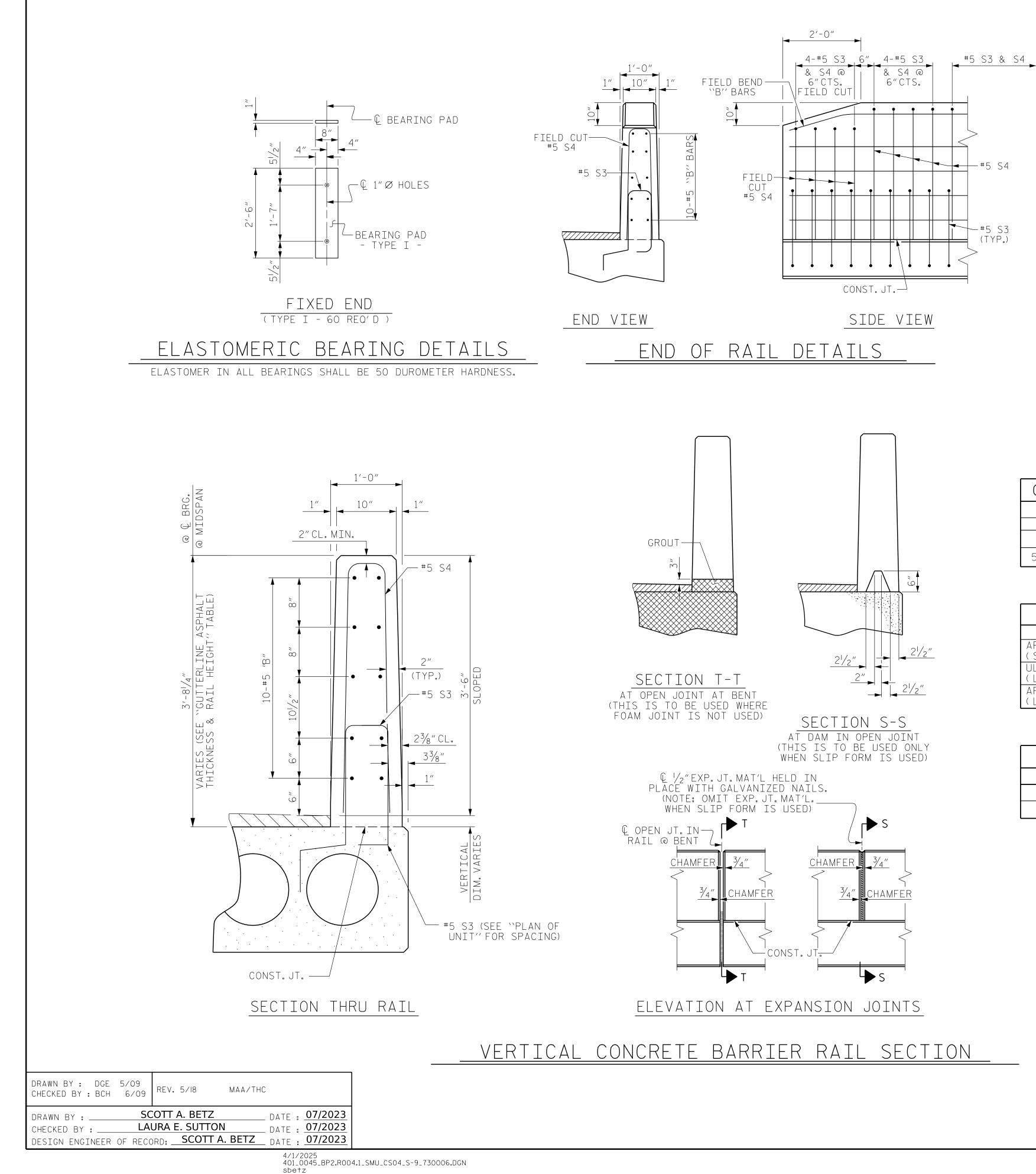
18'-4"	18'-4"
E GROUTED ESS DETAILS (TYP.)	10-#5 B14 IN VERTICAL CONCRETE BARRIER RAIL
GUTTERLINE -	#5 S3 &
	#5 S4
12'' Ø VOIDS 4"	→ ⁴ (TYP.)
12'' Ø VOIDS (TYP.EA.SLAB UNIT) (TYP.)	
SPLICE	
SPLICE	
0.6′′ØL.R. TRANSVERSE	
0.6″ Ø L.R. TRANSVERSE ST-TENSIONING STRAND IN 2 ¹ / ₂ ″ Ø HOLE (TYP.)	
GUTTERLINE	
	#4 S2
└──Û/2''EXP.JT. MAT'L.IN RAIL (TYP.)	BARRIER RAIL
PAIRS (SPACED AS SHOWN IN DETAIL ``A'')(TYP.EA.UNI	T)
3 (SPACED AS SHOWN IN DETAIL ``A'')(TYP.EA.EXT.UNIT SPACED TO MATCH S3 IN VERTICAL CONCRETE BARRIER R	
	27'-6"
55'-0"	
PLAN OF UNIT	

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FINAL	UNL	ESS	ŀ
SIGNATU	res	COMF	



STD. NO. 21" PCS_30_90S_55L

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CONCRETE RELEA	ASE STRENGTH
UNIT	PSI
50′& 55′UNITS	4900

GRADE 270 S	TRANDS
	0.6″Ø L.R.
AREA (SQUARE INCHES)	0.217
ULTIMATE STRENGTH (LBS.PER STRAND)	58,600
APPLIED PRESTRESS (LBS.PER STRAND)	43,950

GUTTERLINE ASPH	HALT THICKNESS & RAI	L HEIGHT	
	ASPHALT OVERLAY THICKNESS	RAIL HEIGHT	
	@ MID-SPAN	@ MID-SPAN	
50′& 55′UNITS	11/8″	3′-7 / ₈ ″	
		F9DF1280241648C	PROJECT NO. BP2.R004.1 PITT COUNTY STATION: 22+05.00-L- SHEET 4 OF 5
		SEAL 054452 4/16/2025	STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH STANDARD
_			3'-0" X 1'-9" PRESTRESSED CONCRETE CORED SLAB UNIT
	DOCUMENT NOT CONSIDE FINAL UNLESS ALL SIGNATURES COMPLETE	RALEIGH, NC 27601 (919) 882-7839	

T THICKNESS & RAI	L HEIGHT	
PHALT OVERLAY THICKNESS	RAIL HEIGHT	
@ MID-SPAN	@ MID-SPAN	
11/8″	3'-7 /8"	
	Signed by: F9DF1280241648C F9DF128024164 F9DF1280241648C F9DF128024164 F9DF128024164 F9DF128024164 F9DF1280240 F9DF12802 F9DF1280240 F9DF12802 F9DF1280240 F9DF12802 F9DF1280 F9DF1280 F9DF1280 F9DF1280 F9DF1280 F9DF1280 F9DF1280 F9DF1280 F9DF1280 F9DF1280 F9DF1280 F9DF1280 F9DF1280 F9DF1280 F9DF1280 F9DF1280 F9DF1280 F9DF1280 F9DF1280 F9DF	PROJECT NO. BP2.R004.1 PITT COUNTY STATION: 22+05.00 -L- SHEET 4 OF 5 STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH STANDARD 3'-0" X 1'-9" PRESTRESSED CONCRETE CORED SLAB UNIT
	& ASSOCIATES	REVISIONS SHEET NO.
DOCUMENT NOT CONSIDE FINAL UNLESS ALL SIGNATURES COMPLETE	RALEIGH, NC 27601 (919) 882-7839	

NOTES

ALL PRESTRESSING STRANDS SHALL BE 7-WIRE LOW RELAXATION GRADE 270 STRANDS AND SHALL CONFORM TO AASHTO M203 EXCEPT FOR SAMPLING REQUIREMENTS WHICH SHALL BE IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS.

ALL REINFORCING STEEL CAST WITH THE CORED SLAB SECTIONS SHALL BE GRADE 60 AND SHALL BE INCLUDED IN THE UNIT PRICE BID FOR PRESTRESSED CONCRETE CORED SLABS.

RECESSES FOR TRANSVERSE STRANDS SHALL BE GROUTED AFTER THE TENSIONING OF THE STRANDS.

THE $2^{1}/_{2}$ " Ø DOWEL HOLES AT FIXED ENDS OF SLAB SECTIONS SHALL BE FILLED WITH NON-SHRINK GROUT.

THE BACKER RODS SHALL CONFORM TO THE REQUIREMENTS OF TYPE M BOND BREAKER. SEE SECTION 1028 OF THE STANDARD SPECIFICATIONS.

WHEN CORED SLABS ARE CAST, AN INTERNAL HOLD-DOWN SYSTEM SHALL BE EMPLOYED TO PREVENT VOIDS FROM RISING OR MOVING SIDEWAYS.AT LEAST SIX WEEKS PRIOR TO CASTING CORED SLABS, THE CONTRACTOR SHALL SUBMIT TO THE ENGINEER FOR REVIEW AND COMMENT, DETAILED DRAWINGS OF THE PROPOSED HOLD-DOWN SYSTEM. IN ADDITION TO STRUCTURAL DETAILS, LOCATION AND SPACING OF THE HOLD-DOWNS SHALL BE INDICATED.

ALL REINFORCING STEEL IN THE VERTICAL CONCRETE BARRIER RAIL SHALL BE EPOXY COATED.

PRESTRESSING STRANDS SHALL BE CUT FLUSH WITH THE CORED SLAB UNIT ENDS.

APPLY EPOXY PROTECTIVE COATING TO CORED SLAB UNIT ENDS.

GROOVED CONTRACTION JOINTS, $\frac{1}{2}$ " IN DEPTH, SHALL BE TOOLED IN ALL EXPOSED FACES OF THE BARRIER RAIL AND IN ACCORDANCE WITH ARTICLE 825-10(B) OF THE STANDARD SPECIFICATIONS. A CONTRACTION JOINT SHALL BE LOCATED AT EACH THIRD POINT BETWEEN BARRIER RAIL EXPANSION JOINTS. ONLY ONE CONTRACTION JOINT IS REQUIRED AT MIDPOINT OF BARRIER RAIL SEGMENTS LESS THAN 20 FEET IN LENGTH AND NO CONTRACTION JOINTS ARE REQUIRED FOR THOSE SEGMENTS LESS THAN 10 FEET IN LENGTH.

FLAME CUTTING OF THE TRANSVERSE POST-TENSIONING STRAND IS NOT ALLOWED.

THE TRANSFER OF LOAD FROM THE ANCHORAGES TO THE CORED SLAB UNIT SHALL BE DONE WHEN THE CONCRETE HAS REACHED A COMPRESSIVE STRENGTH OF NOT LESS THAN THE REQUIRED STRENGTH SHOWN IN THE "CONCRETE RELEASE STRENGTH" TABLE.

FOR GROUT FOR STRUCTURES, SEE SPECIAL PROVISIONS.

THE PERMITTED THREADED INSERTS ARE DETAILED AS AN OPTION FOR THE CONTRACTOR TO ATTACH FALSEWORK AND FORMWORK DURING CONSTRUCTION.

THE PERMITTED THREADED INSERTS IN THE EXTERIOR UNITS SHALL BE SIZED BY THE CONTRACTOR. SPACED AT 4'-O"CENTERS AND GALVANIZED IN ACCORDANCE WITH SECTION 1076 OF THE STANDARD SPECIFICATIONS. STAINLESS STEEL THREADED INSERTS MAY BE USED AS AN ALTERNATE.

THE PERMITTED THREADED INSERTS SHALL BE GROUTED BY THE CONTRACTOR IMMEDIATELY FOLLOWING REMOVAL OF THE FALSEWORK.

THE COST OF THE PERMITTED THREADED INSERTS SHALL BE INCLUDED IN THE PRICE BID FOR THE PRECAST UNITS.

FOR BILL OF MATERIALS, BAR TYPES, AND OTHER TABLES, SEE SHEET 5 OF 5.

STD. NO. 21" PCS3_30_90S

CORED SLABS REQUIRED						
	NUMBER	LENGTH	TOTAL LENGTH			
50'UNIT						
EXTERIOR C.S.	4	50'-0"	200'-0"			
INTERIOR C.S.	16	50'-0"	800'-0"			
TOTAL	20		1000'-0"			

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BII	BILL OF MATERIAL FOR VERTICAL CONCRETE BARRIER RAIL						
BAR	BARS PER PAIR OF EXTERIOR UNITS	TOTAL NO.	SIZE	TYPE	LENGTH	WEIGHT	
	50'UNIT						
₩B13	40	80	#5	STR	24'-7"	2051	
米 S4	116	232	#5	2	7'-2"	1734	
* EPOX	* EPOXY COATED REINFORCING STEEL LBS. 3785						
CLASS AA CONCRETE CU.YDS.						25.6	
TOTAL VERTICAL CONCRETE BARRIER RAIL LIN.FT.					200.50		

DEAD LOAD DE

50' & 55' CORED CAMBER (SLAB ALO DEFLECTION DUE TO SUPERIMPOSED DEAD FINAL CAMBER

** INCLUDES FUTURE WEARING SURFACE

	BILL OF MATERIAL FOR ONE 50' CORED SLAB UNIT							
	EXTERIOR UNIT INTERIOR UNIT							
BAR	NUMBER	SIZE	TYPE	LENGTH	WEIGHT	LENGTH	WEIGHT	
B6	4	#4	STR	25′-9″	69	25′-9″	69	
S1	8	#5	3	4'-3"	35	4'-3"	35	
S2	104	# 4	3	5′-4″	371	5′-4″	371	
米 S3	58	#5	1	5′-7″	338			
REINF(ORCING	STEEL	LBS	5.	475		475	
* EPOXY COATED REINFORCING STEEL LBS. 338								
6500 F	P.S.I.CO	NCRETE	CU.YDS) .	7.2		7.1	
0.6″Ø	L.R. STR	ANDS	Nc).	19		19	

TABLES FOR 50' UNITS SPANS A & B

DRAWN BY :DGE 5/09
CHECKED BY : BCH 6/09REV. 1/15MAA/THCDRAWN BY :SCOTT A. BETZ
LAURA E. SUTTONDATE :07/2023
07/2023
DATE :DESIGN ENGINEER OF RECORD:SCOTT A. BETZ
O7/2023DATE :07/2023
07/2023

CORED	SLABS	s req	UIRED
	NUMBER	LENGTH	TOTAL LENGTH
55′UNIT			
EXTERIOR C.S.	2	55'-0"	110'-0"
INTERIOR C.S.	8	55'-0"	440'-0"
TOTAL	10		550'-0"

BIL	BILL OF MATERIAL FOR VERTICAL CONCRETE BARRIER RAIL							
BAR	BARS PER PAIR OF EXTERIOR UNITS	TOTAL NO.	SIZE	TYPE	LENGTH	WEIGHT		
	55' UNIT							
₩ B14	40	40	#5	STR	27'-1"	1130		
米 S4	128	128	#5	2	7'-2"	957		
★ EPOX	Y COATED REINFORCING STEEL			LBS.		2087		
CLASS AA CONCRETE CU.YDS.						14.1		
TOTAL	VERTICAL CONCRETE BARRIER RAIL			LIN.FT	٥	110.25		

FLECTION AN	ND CAMBER
	3'-0"× 1'-9"
SLAB UNIT	0.6″ØL.R. Strand
ONE IN PLACE)	1 ∕₂″ ♦
) D load **	3∕8″ ♦
	1 ∕8″ ♦
RE WEARTNG SURE	

	BILL OF MATERIAL FOR ONE 55' CORED SLAB UNIT						
				EXTERI	OR UNIT	INTERI	OR UNIT
BAR	NUMBER	SIZE	TYPE	LENGTH	WEIGHT	LENGTH	WEIGHT
Β7	4	#4	STR	28'-3"	75	28'-3"	75
S1	8	#5	3	4'-3"	35	4'-3"	35
S2	114	#4	3	5'-4"	406	5'-4"	406
* S3	64	#5	1	5′-7″	373		
REINFO	ORCING 2	STEEL	LBS	5.	516		516
	(Y COATE						
REINFORCING STEEL LBS. 373							
6500 F	P.S.I.CO	NCRETE	CU. YDS		7.9		7.8
0.6″Ø	L.R. STR	ANDS	Nc).	19		19

TABLES FOR 55' UNIT

DOCUMENT NOT CONSIDERED	301 FAYETTEVILLE ST., SUITE 1500
FINAL UNLESS ALL	RALEIGH, NC 27601 (919) 882-7839
SIGNATURES COMPLETED	NC FIRM LICENSE: C-1506

— Signed by:

4/16/2025

TH CAROL

ື SEAL ໌ 054452

KISINGER CAMPO & ASSOCIATES

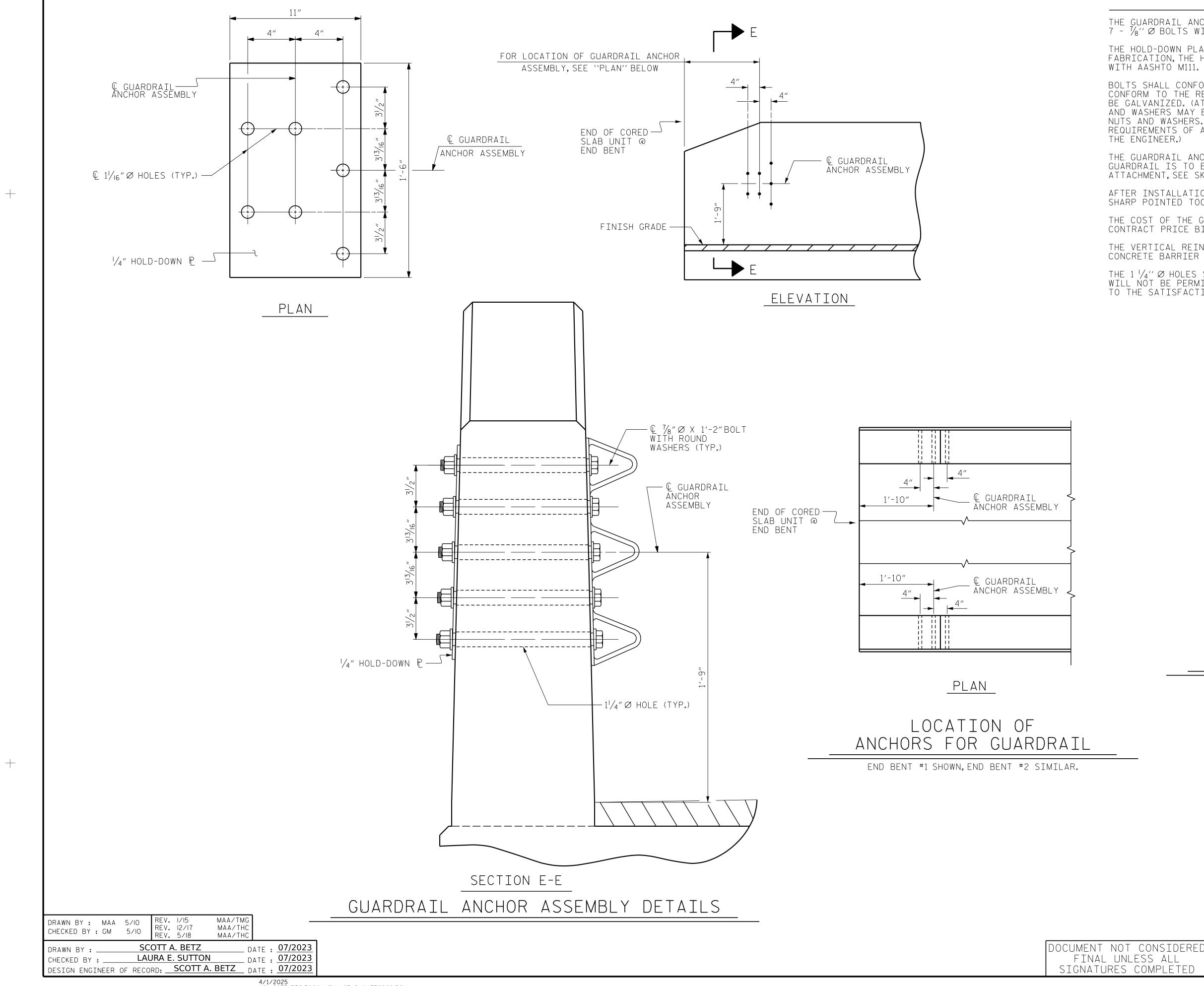




3'-0" X 1'-9" PRESTRESSED CONC CORED SLAB UNI 90° SKEW	
REVISIONS	SHEET NO.

		SHEET NO.				
NO.	BY:	DATE:	NO.	BY:	DATE:	S-10
1			3			TOTAL SHEETS
2			4			20

STD. NO. 21" BOM INF0_90_30S



NOTES

THE GUARDRAIL ANCHOR ASSEMBLY SHALL CONSIST OF A $^{1\!/}_{4}{}^{\prime\prime}$ hold down plate and 7 - $^{\prime\prime}_{8}{}^{\prime\prime}$ Ø Bolts with nuts and washers.

THE HOLD-DOWN PLATE SHALL CONFORM TO AASHTO M270 GRADE 36.AFTER FABRICATION, THE HOLD-DOWN PLATE SHALL BE HOT-DIP GALVANIZED IN ACCORDANCE WITH AASHTO M111.

BOLTS SHALL CONFORM TO THE REQUIREMENTS OF ASTM A307 AND NUTS SHALL CONFORM TO THE REQUIREMENTS OF AASHTO M291. BOLTS, NUTS AND WASHERS SHALL BE GALVANIZED. (AT THE CONTRACTOR'S OPTION, STAINLESS STEEL BOLTS, NUTS AND WASHERS MAY BE USED AS AN ALTERNATE FOR THE $\frac{7}{8}$ " Ø GALVANIZED BOLTS, NUTS AND WASHERS. THEY SHALL CONFORM TO OR EXCEED THE MECHANICAL REQUIREMENTS OF ASTM A307. THE USE OF THIS ALTERNATE SHALL BE APPROVED BY THE ENGINEER.)

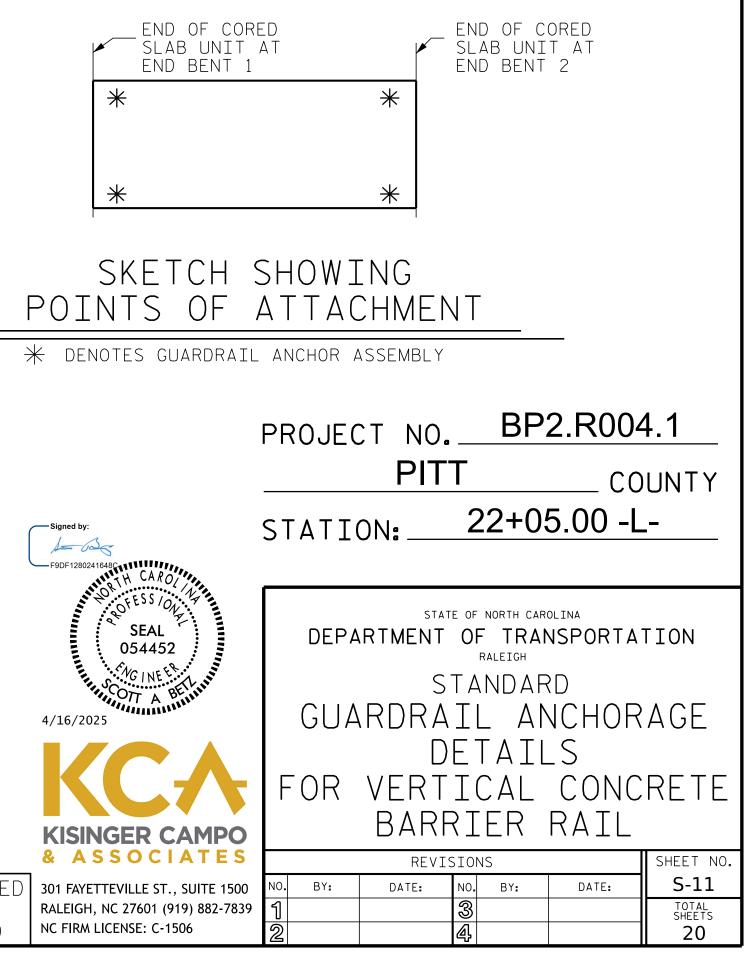
THE GUARDRAIL ANCHOR ASSEMBLY IS REQUIRED AT ALL POINTS WHERE APPROACH GUARDRAIL IS TO BE ATTACHED TO THE END OF BARRIER RAIL.FOR POINTS OF ATTACHMENT, SEE SKETCH.

AFTER INSTALLATION, THE EXPOSED THREAD OF THE BOLT SHALL BE BURRED WITH A SHARP POINTED TOOL.

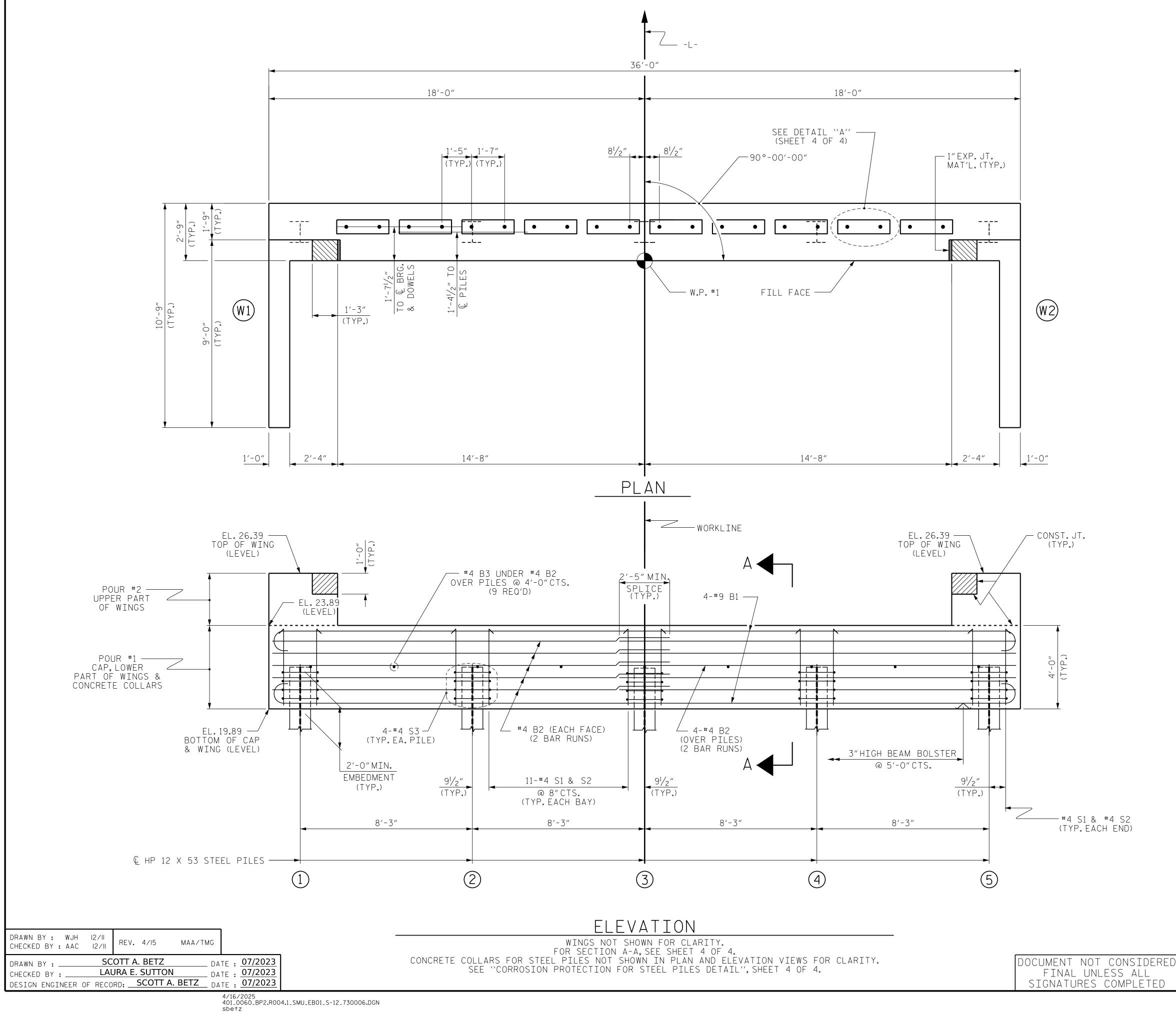
THE COST OF THE GUARDRAIL ANCHOR ASSEMBLY SHALL BE INCLUDED IN THE UNIT CONTRACT PRICE BID FOR VERTICAL CONCRETE BARRIER RAIL.

THE VERTICAL REINFORCING BARS MAY BE SHIFTED SLIGHTLY IN THE VERTICAL CONCRETE BARRIER RAIL TO CLEAR ASSEMBLY BOLTS.

THE 1 $\frac{1}{4}$ " Ø HOLES SHALL BE FORMED OR DRILLED WITH A CORE BIT. IMPACT TOOLS WILL NOT BE PERMITTED. ANY CONCRETE DAMAGED BY THIS WORK SHALL BE REPAIRED TO THE SATISFACTION OF THE ENGINEER.



STD. GRA3 SHT. 1



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NOTES

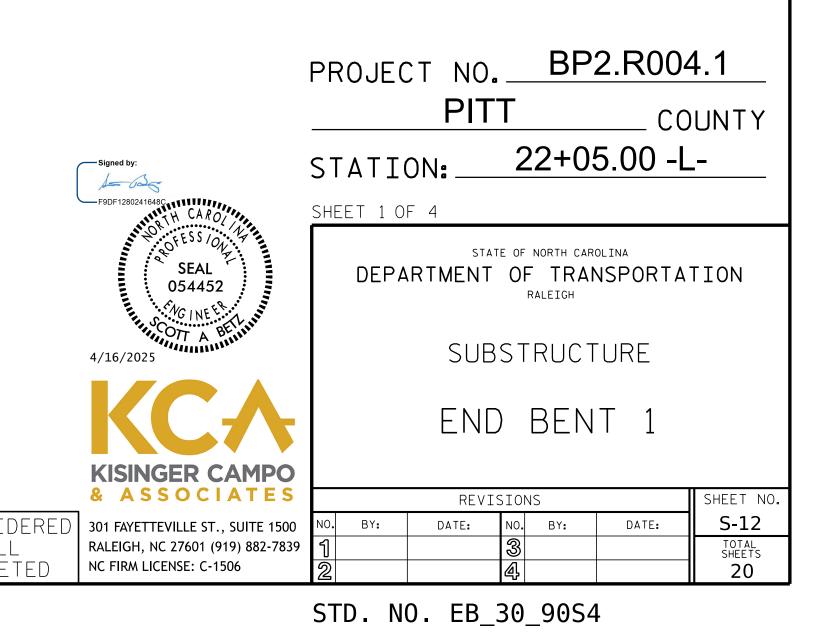
STIRRUPS IN CAP MAY BE SHIFTED AS NECESSARY TO CLEAR DOWELS.

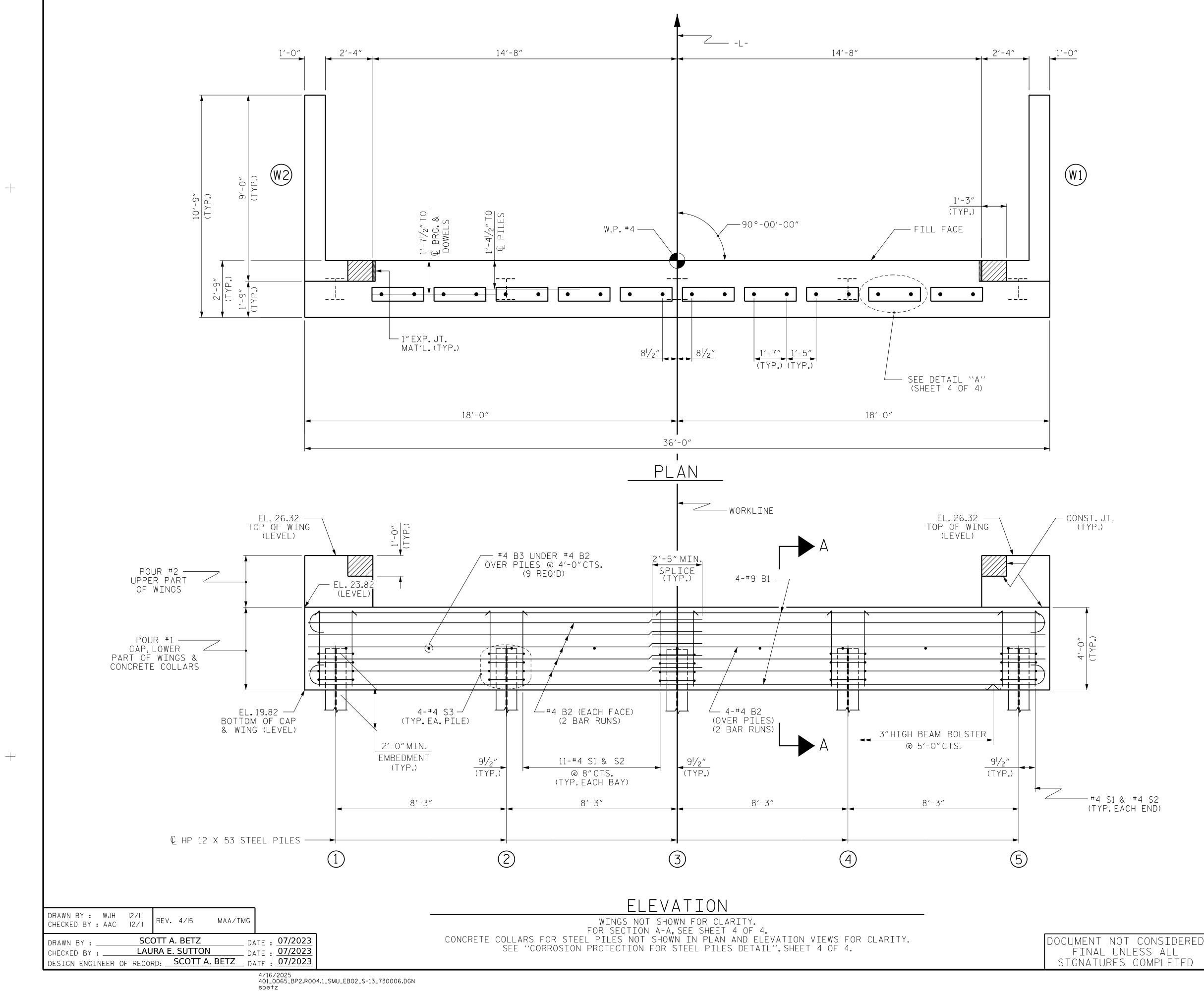
THE CONCRETE IN THE SHADED AREA OF THE WING SHALL BE POURED AFTER THE VERTICAL CONCRETE BARRIER RAIL IS CAST IF SLIP FORMING IS USED.

FOR PILE SPLICE DETAILS, SEE SHEET 4 OF 4.

FOR WING DETAILS, SEE SHEET 3 OF 4.

INSTALL THE 6"Ø DRAIN PIPE THROUGH THE WINGWALL AS REQUIRED FOR REINFORCED BRIDGE APPROACH FILLS. SEE ROADWAY PLANS. REINFORCING STEEL IN THE WINGWALL MAY BE SHIFTED AS NECESSARY TO CLEAR THE DRAIN PIPE.





NOTES

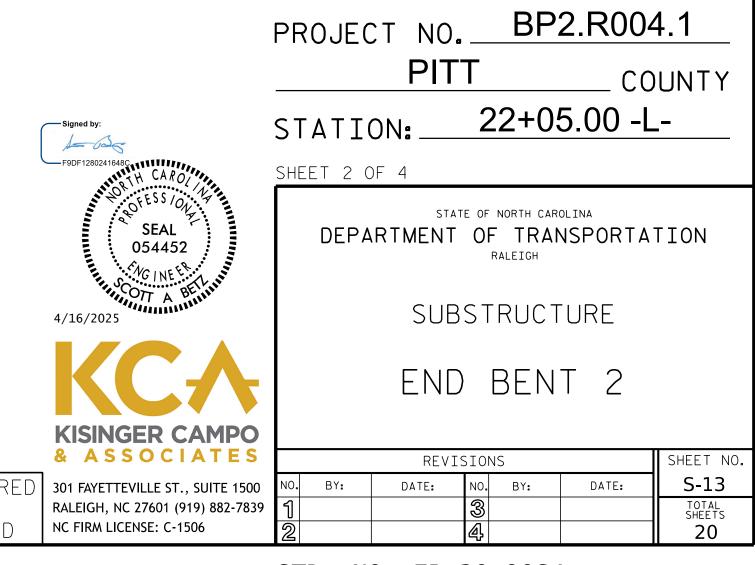
STIRRUPS IN CAP MAY BE SHIFTED AS NECESSARY TO CLEAR DOWELS.

THE CONCRETE IN THE SHADED AREA OF THE WING SHALL BE POURED AFTER THE VERTICAL CONCRETE BARRIER RAIL IS CAST IF SLIP FORMING IS USED.

FOR PILE SPLICE DETAILS, SEE SHEET 4 OF 4.

FOR WING DETAILS, SEE SHEET 3 OF 4.

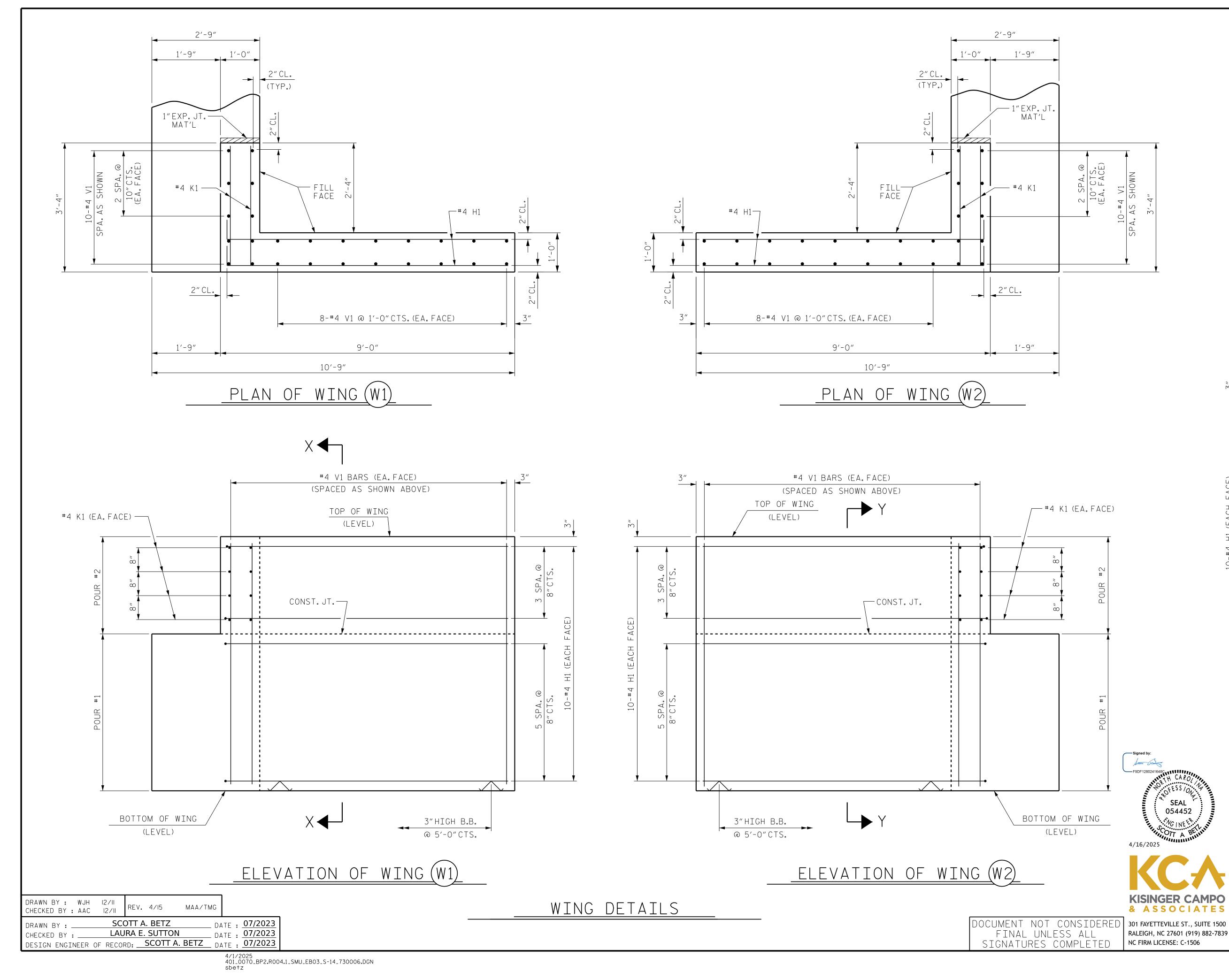
INSTALL THE 6" Ø DRAIN PIPE THROUGH THE WINGWALL AS REQUIRED FOR REINFORCED BRIDGE APPROACH FILLS. SEE ROADWAY PLANS. REINFORCING STEEL IN THE WINGWALL MAY BE SHIFTED AS NECESSARY TO CLEAR THE DRAIN PIPE.

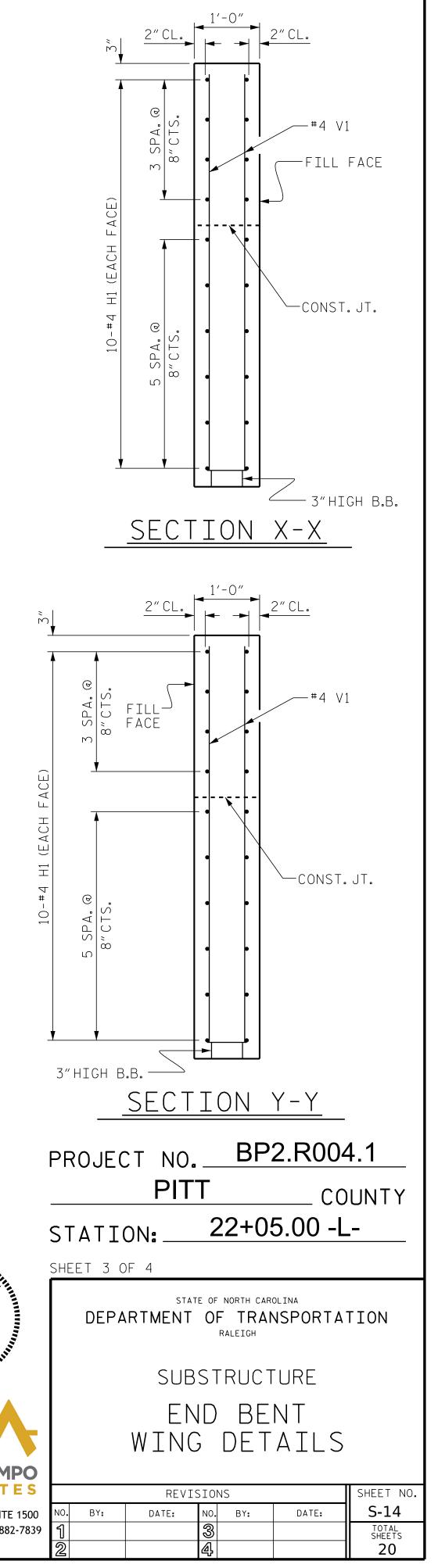


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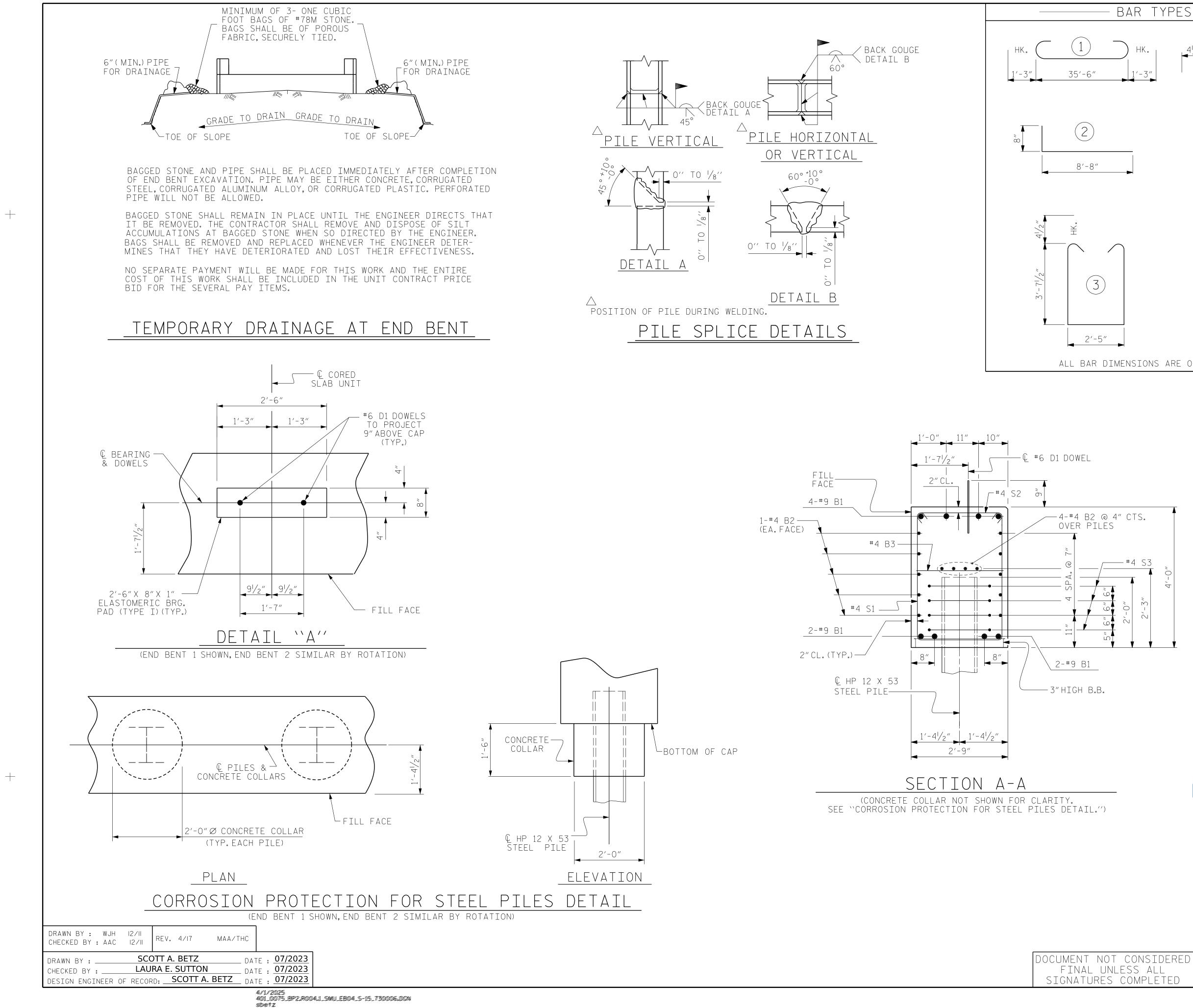
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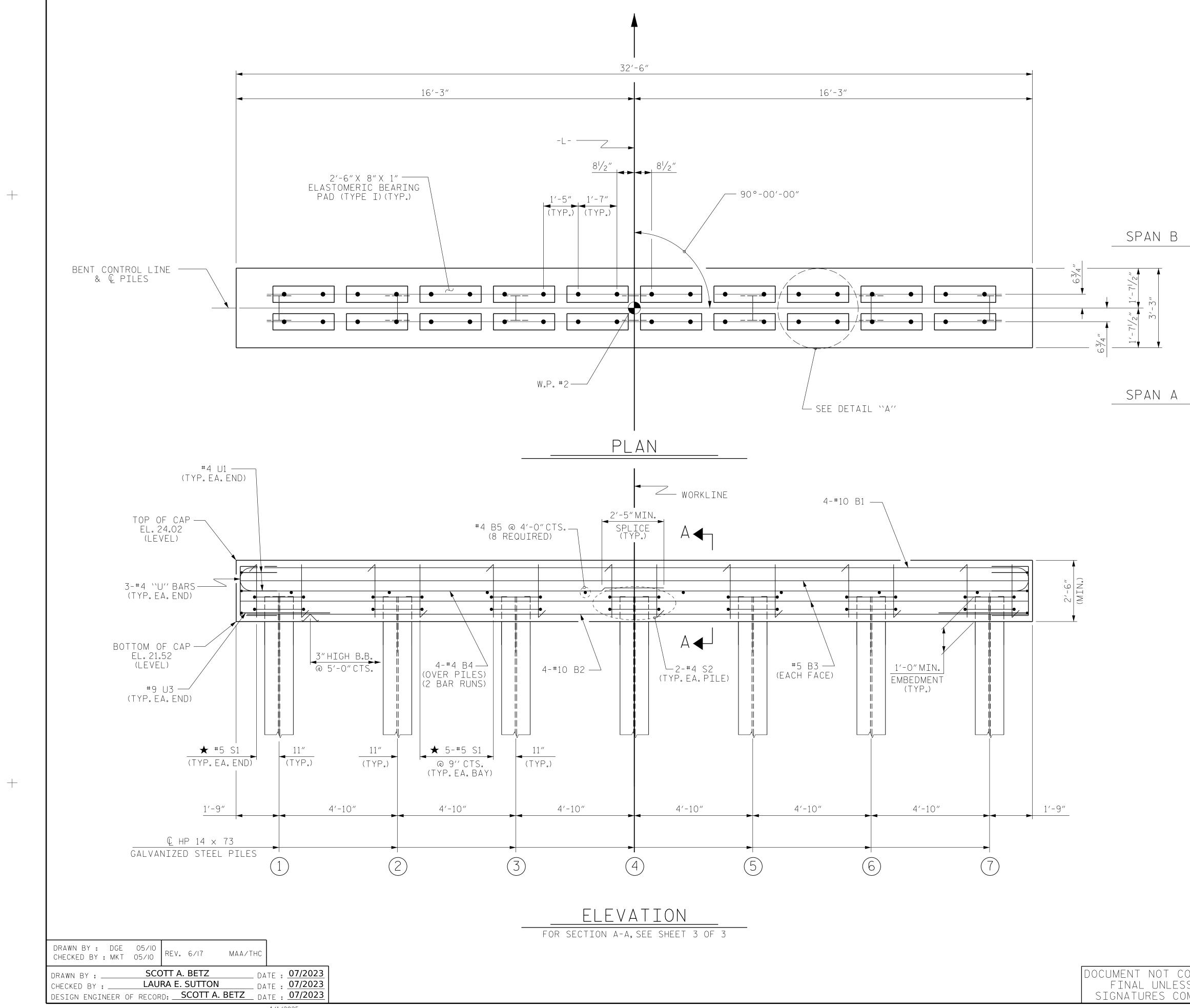
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r types		ΒT	LL O	F MA	ATERIA	4
	FOR ONE END BENT					
	BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT
HK. $4^{1/2}$ $2'-5''$ $4^{1/2}$	B1	8	#9	1	38'-0"	1034
	B2	28	#4	STR	19'-1"	357
<u>′-3″</u> НК. (4) НК. Г	Β3	9	#4	STR	2'-5"	15
	D1	20	#6	STR	1'-6"	45
1'-3'' LAP	H1	40	#4	2	9'-4"	249
	К1	16	#4	STR	2'-11"	31
	S1	46	#4	3	10'-5"	320
((5))	S2	46	#4	4	3'-2"	97
	S3	20	#4	5	6'-6"	87
1'-8"Ø	V1	52	#4	STR	6'-2"	214
			NG STE			
	(FOR	one e	ND BEN	IT)		2449 LBS.
			ONCRET ONE ENI		AKDOWN F)	
	POUR		AP,LOW F WINC		RT Collars	17.9 C.Y.
IONS ARE OUT TO OUT.	POUR		PPER P INGS	art o	F	2.1 C.Y.
	TOTAL	CLAS	SS A C	oncre ⁻	ΓE	20.0 C.Y.

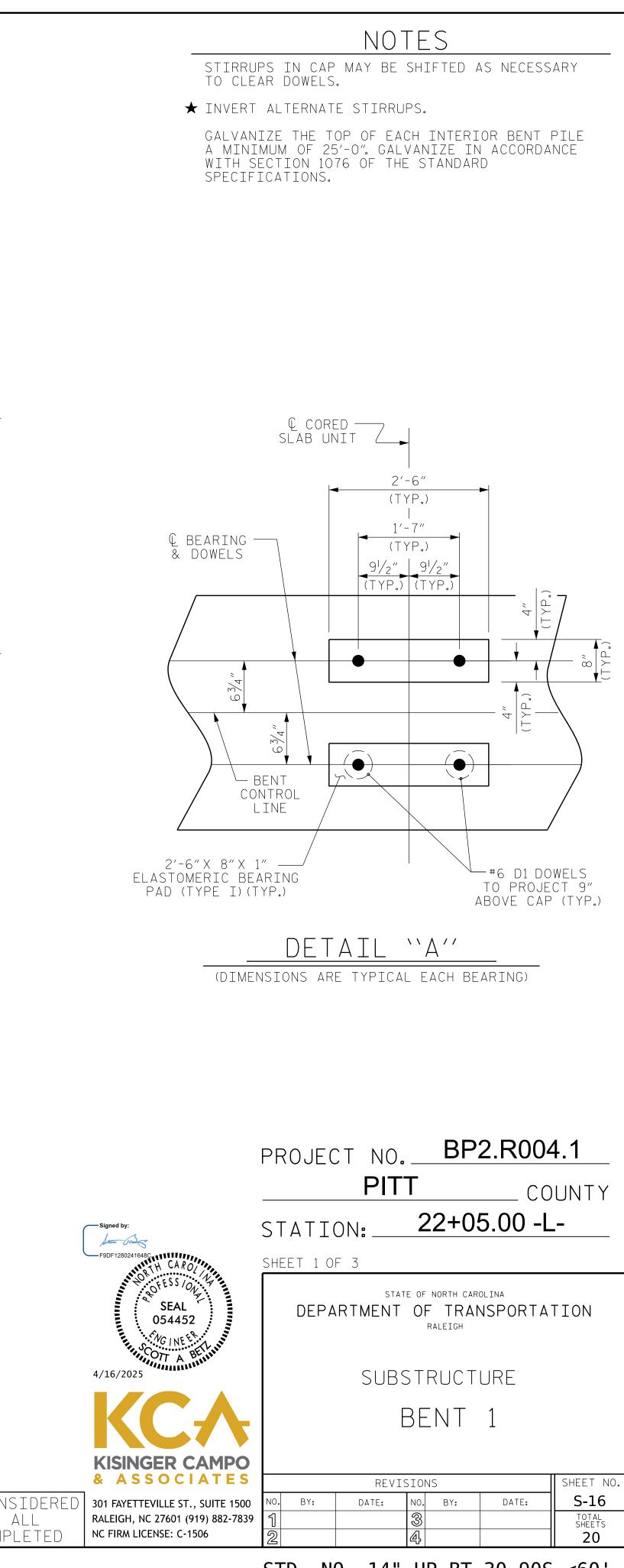
F9DF1280241648C	PROJECT NO. BP2.R004.1 PITT COUNTY STATION: 22+05.00 -L- SHEET 4 OF 4
SEAL 054452 4/16/2025	STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH SUBSTRUCTURE
KISINGER CAMPO	END BENT 1 & 2 Details
 ASSOCIATES 301 FAYETTEVILLE ST., SUITE 1500 RALEIGH, NC 27601 (919) 882-7839 NC FIRM LICENSE: C-1506 	REVISIONSSHEET NO.NO.BY:DATE:NO.BY:DATE:S-1513TOTAL SHEETS2420

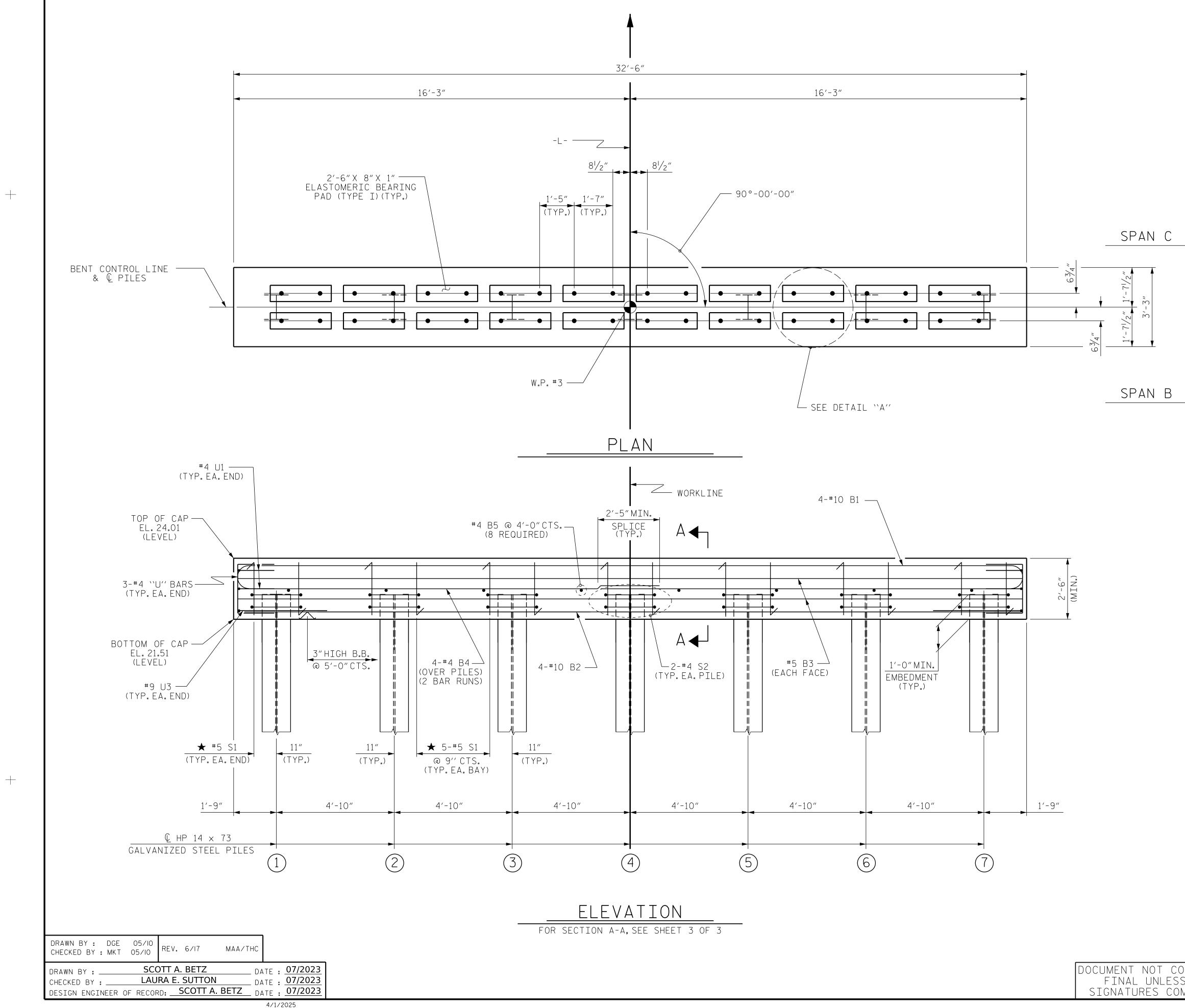
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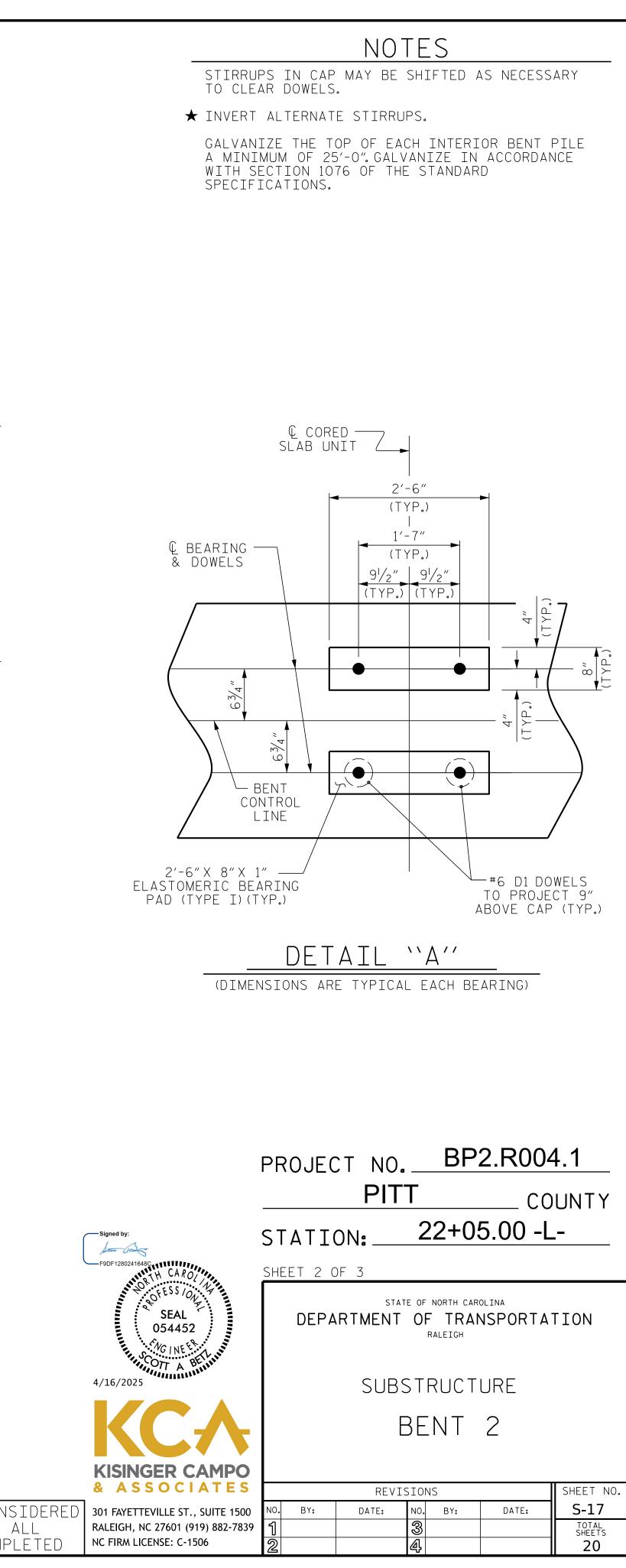
4/1/2025 401_0080_BP2.R004.1_SMU_B01_S-16_730006.DGN sbetz

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SIGNATURES COMPLETED

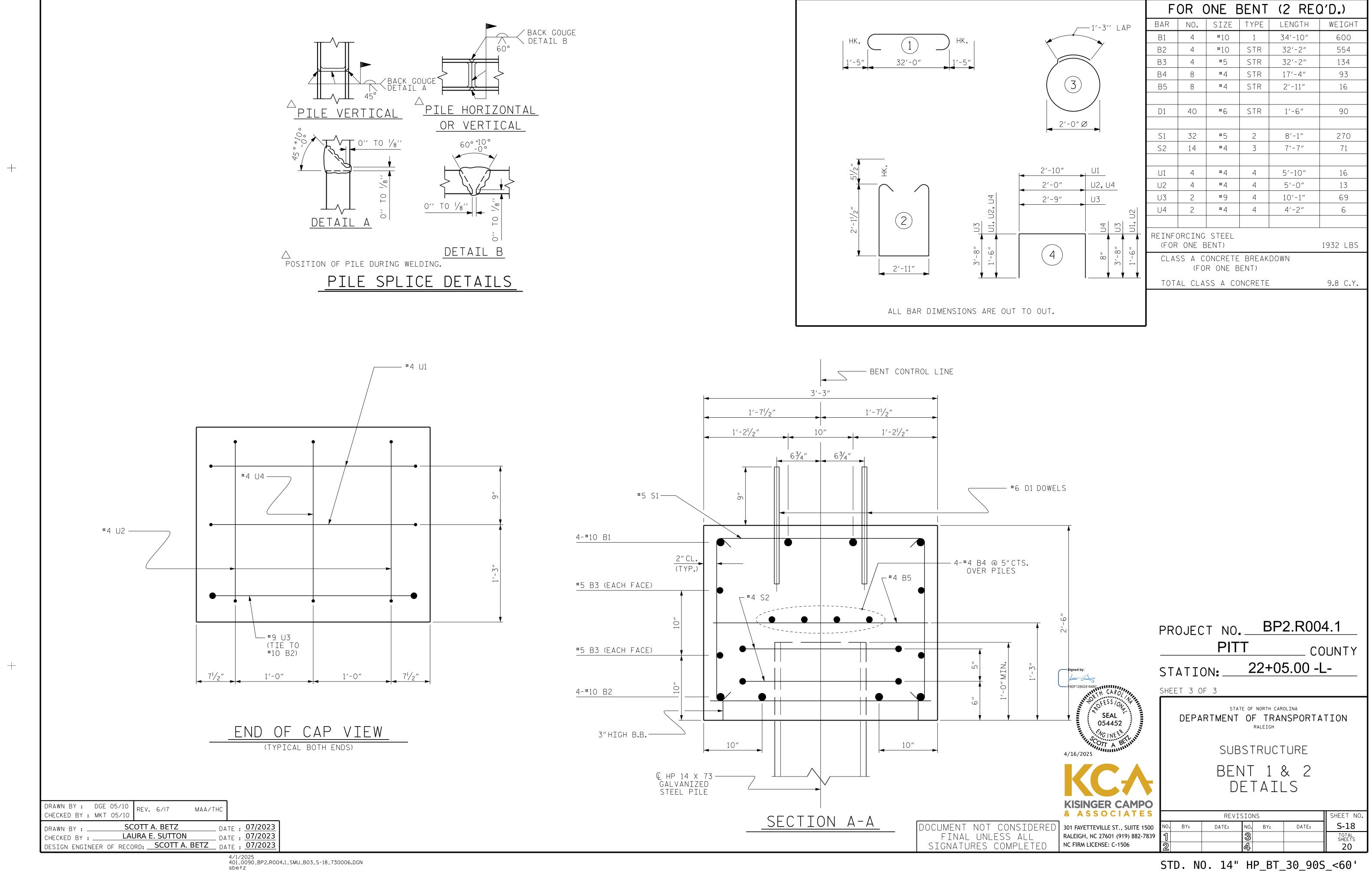




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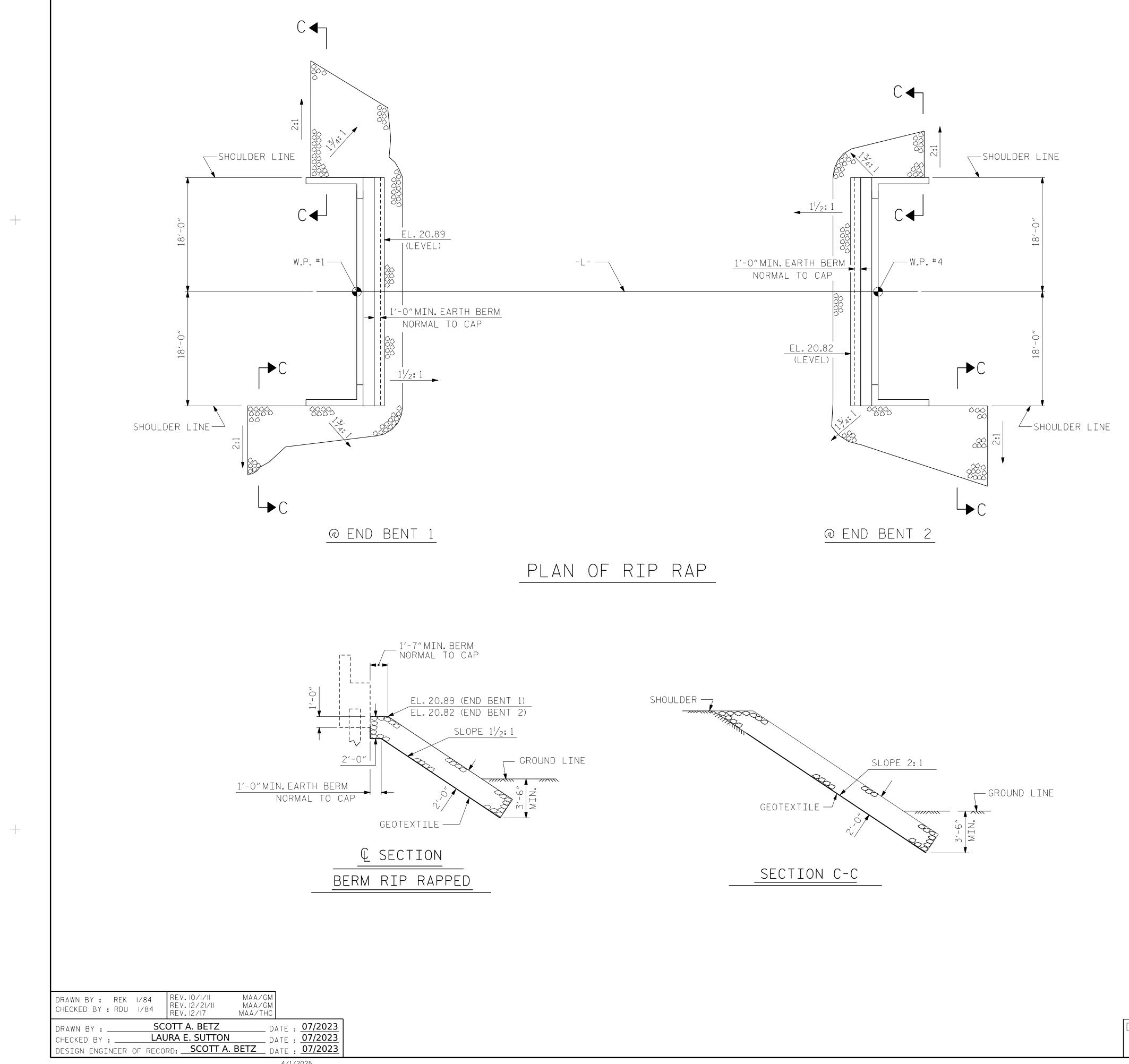
STD. NO. 14" HP_BT_30_90S_<60'





		BILL OF MATERIAL					
		F	OR (ONE E	BENT	(2 RE(Q'D.)
	1'-3'' LAP	BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT
		B1	4	#10	1	34'-10"	600
	7	B2	4	#10	STR	32'-2"	554
	/ 	B3	4	#5	STR	32'-2"	134
	\mathbf{i}	Β4	8	#4	STR	17'-4"	93
((3)		B5	8	#4	STR	2'-11"	16
		D1	40	#6	STR	1'-6"	90
2'-0"Ø							
	-1	S1	32	#5	2	8'-1"	270
		S2	14	#4	3	7'-7"	71
2'-10"	U1	U1	4	#4	4	5'-10"	16
2'-0" l	U2,U4	U2	4	#4	4	5'-0"	13
2'-9"	U3	U3	2	#9	4	10'-1"	69
	n2	U4	2	#4	4	4'-2"	6
	U4 U3 U1, L						
			ORCING ONE E	STEEL BENT)			1932 LBS
	8" 3′-8" 1′-6"	CLASS A CONCRETE BREAKDOWN (FOR ONE BENT)					
_1 _	<u>v v v</u>	TOTAL CLASS A CONCRETE 9.8 C.Y.					9.8 C.Y.
TO OUT.							

STD. NO. 14" HP_BT_30_90S_<60'



4/1/2025 401_0095_BP2.R004.1_SMU_RR_S-19_730006.DGN sbetz

ESTIMATED QUANTITIES							
RIP RAP CLASS II (2'-0"THICK)	GEOTEXTILE For drainage						
TONS	SQUARE YARDS						
114	233						
127	258						
	RIP RAP CLASS II (2'-O" THICK) TONS 114						

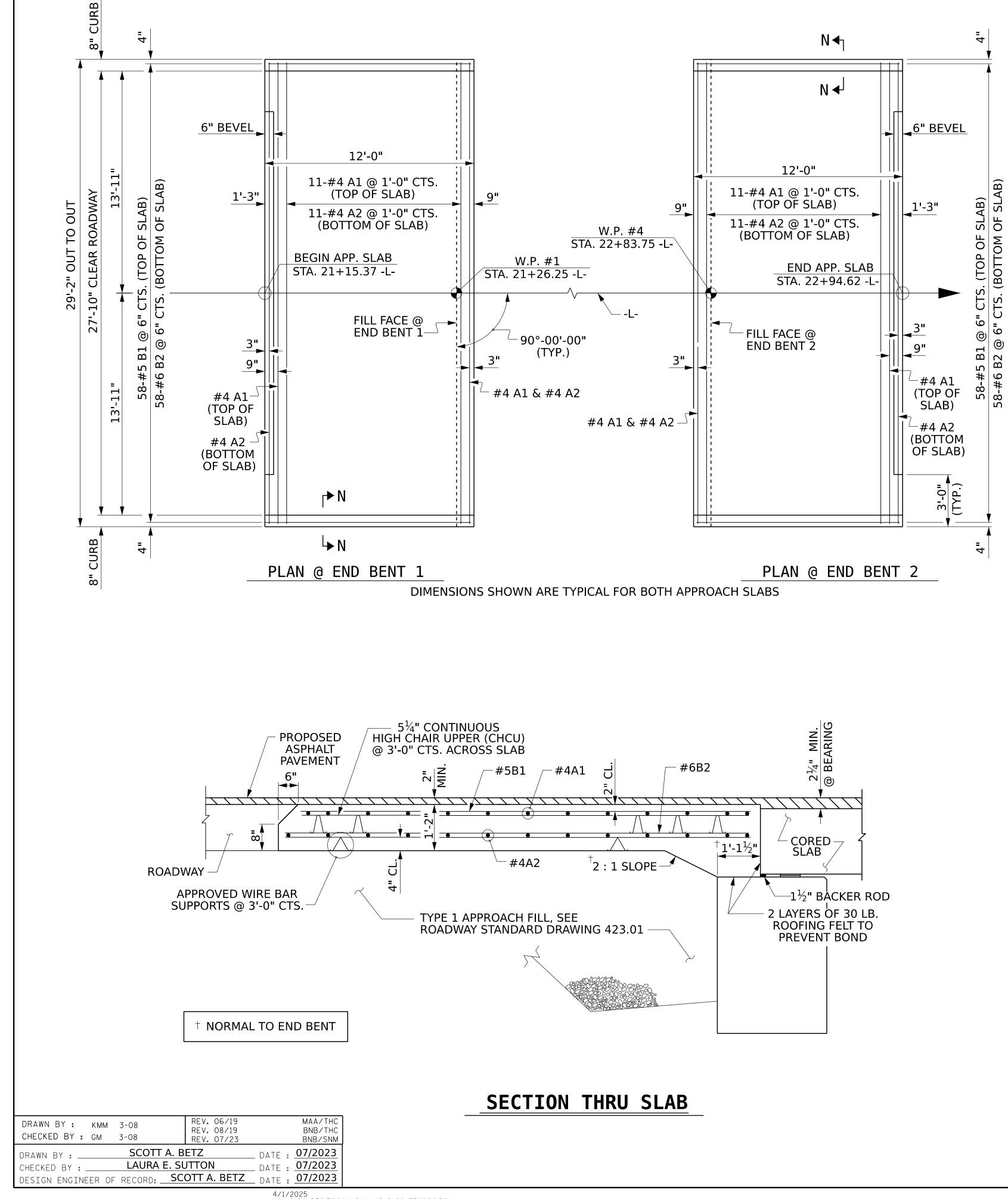
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SIGNATU	res	COMP	LETED

NO	tes	a a				
FOR	BERM	WIDTH	DIMENSIONS,	SEE	GENERAL	DRAWING.

F9DF1280241648C	PROJECT NO. BP2.R004.1 PITT COUNT STATION: 22+05.00 -L-	 Y			
SEAL 054452 4/16/2025	STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH STANDARD				
KISINGER CAMPO	RIP RAP DETAILS				
& ASSOCIATES	REVISIONS SHEET NO.				
RED 301 FAYETTEVILLE ST., SUITE 1500 RALEIGH, NC 27601 (919) 882-7839 NC FIRM LICENSE: C-1506	NO. BY: DATE: NO. BY: DATE: S-1 1 3 3 TOTA SHEE TOTA SHEE 2 2 2 2 2 2 2 2 2 2 2 2 2 2 3 1 3	AL Ets			

STD. NO. RR1

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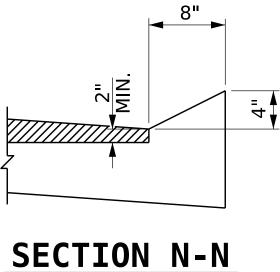
4/1/2025 401_0100_BP2.R004.1_SMU_AS_S-20_730006.DGN sbetz

NOTES

FOR BRIDGE APPROACH FILL, SEE ROADWAY PLANS.

AREA BETWEEN THE WINGWALL AND APPROACH SLAB SHALL BE GRADED TO DRAIN THE WATER AWAY FROM THE FILL FACE OF THE BRIDGE AND SHALL BE PAVED. SEE ROADWAY PLANS.

APPROACH SLAB GROOVING IS NOT REQUIRED.





DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED

BILL OF MATERIAL APPROACH SLAB AT EB 1													
ļ	\PPR	0ACH	SLA	B AT EE	3 1								
BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT								
*A1	13	#4	STR	28'-10"	250								
A2	13	#4	STR	28'-10"	250								
*B1	58	#5	11'-2"	676									
B2	58	#6	STR	11'-8"	1016								
					5. 1266								
	REINFORCING STEEL LE												
* EPO REIN	5. 926												
REINFORCING STEEL LBS.													
CLASS AA CONCRETE C. Y.													
	۱PPR	OACH	B AT EE	3 2									
BAR	NO.	SIZE	LENGTH	WEIGHT									
*A1	13	#4	STR	28'-10"	250								
A2	13	#4	STR	28'-10"	250								
*B1	58	#5	STR	11'-2"	676								
B2	58	#6	STR	11'-8"	1016								
BZ 58 #6 SIR 11-8" 10													
		NG STE	EL	LBS	5. 1266								
	NY CO NFORC		LBS	5. 926									
REINFORCING STEEL LBS.													
CLAS	<u>S AA C</u>	CONCRI	ETE	<u> </u>	<u>′ </u>								

SP	LICE LE	ENGTHS
BAR SIZE	EPOXY COATED	UNCOATED
#4	1'-11"	1'-7"
#5	2'-5"	2'-0"
#6	3'-7"	2'-5"

	PROJECT NO. BP2.R004.1
	PITT COUNTY
Signed by:	STATION: 22+05.00 -L-
F9DF1280241648C	
SEAL 054452	STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH
TT A BELLIN	
4/16/2025	BRIDGE APPROACH SLAB
KCA	CONCRETE CORED
KISINGER CAMPO	(SUB-REGIONAL TIER)
& ASSOCIATES	REVISIONS SHEET NO.
301 FAYETTEVILLE ST., SUITE 1500	NO. BY: DATE: NO. BY: DATE: S-20
RALEIGH, NC 27601 (919) 882-7839 NC FIRM LICENSE: C-1506	1 3 TOTAL SHEETS 2 4 20

STD. NO. BAS_30_90S

DESIGN DATA:

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SPECIFICATIONS		AASHTO (CURRENT)
LIVE LOAD		SEE PLANS
IMPACT ALLOWANCE .		SEE AASHTO
STRESS IN EXTREME STRUCTURAL STEEL	FIBER OF - AASHTO M270 GRADE 36	20,000 LBS. PER SQ. IN.
	- AASHTO M270 GRADE 50W	27,000 LBS. PER SQ. IN.
	- AASHTO M270 GRADE 50	27,000 LBS. PER SQ. IN.
REINFORCING STEEL	IN TENSION - GRADE 60	24,000 LBS. PER SQ. IN.
CONCRETE IN COMPR	ESSION	1,200 LBS. PER SQ. IN.
CONCRETE IN SHEAR		SEE AASHTO
STRUCTURAL TIMBER	- TREATED OR UNTREATED EXTREME FIBER STRESS	1,800 LBS. PER SQ. IN.
COMPRESSION PERPE	NDICULAR TO GRAIN OF TIMBER	375 LBS. PER SQ. IN.
EQUIVALENT FLUID P	RESSURE OF EARTH	30 LBS. PER CU. FT. (MINIMUM)

MATERIAL AND WORKMANSHIP:

EXCEPT AS MAY OTHERWISE BE SPECIFIED ON PLANS OR IN THE SPECIAL PROVISIONS, ALL MATERIAL AND WORKMANSHIP SHALL BE IN ACCORDANCE WITH THE 2024 "STANDARD SPECIFICATIONS FOR ROADS AND STRUCTURES" OF THE N. C. DEPARTMENT OF TRANSPORTATION.

STEEL SHEET PILING FOR PERMANENT OR TEMPORARY APPLICATIONS SHALL BE HOT ROLLED.

CONCRETE:

UNLESS OTHERWISE REQUIRED ON PLANS, CLASS A CONCRETE SHALL BE USED FOR ALL PORTIONS OF ALL STRUCTURES WITH THE EXCEPTION THAT: CLASS AA CONCRETE SHALL BE USED IN BRIDGE SUPERSTRUCTURES, ABUTMENT BACKWALLS, AND APPROACH SLABS; AND CLASS B CONCRETE SHALL BE USED FOR SLOPE PROTECTION AND RIP RAP.

CONCRETE CHAMFERS:

UNLESS OTHERWISE NOTED ON THE PLANS, ALL EXPOSED CORNERS ON STRUCTURES SHALL BE CHAMFERED $\frac{3}{4}$ " WITH THE FOLLOWING EXCEPTIONS: TOP CORNERS OF CURBS MAY BE ROUNDED TO $1\frac{1}{2}$ " RADIUS WHICH IS BUILT INTO CURB FORMS; CORNERS OF TRANSVERSE FLOOR EXPANSION JOINTS SHALL BE ROUNDED WITH A $\frac{1}{4}$ " FINISHING TOOL UNLESS OTHERWISE REQUIRED ON PLANS, AND CORNERS OF EXPANSION JOINTS IN THE ROADWAY FACES AND TOPS OF CURBS AND SIDEWALKS SHALL BE ROUNDED TO A $\frac{1}{4}$ " RADIUS WITH A FINISHING STONE OR TOOL UNLESS OTHERWISE REQUIRED ON PLANS.

DOWELS:

-

DOWELS WHEN INDICATED ON PLANS AS FOR CULVERT EXTENSIONS, SHALL BE EMBEDDED AT LEAST 12" INTO THE OLD CONCRETE AND GROUTED INTO PLACE WITH 1:2 CEMENT MORTAR.

BRIDGES SHALL BE BUILT ON THE GRADE OR VERTICAL CURVE SHOWN ON PLANS. SLABS, CURBS AND PARAPETS SHALL CONFORM TO THE GRADE OR CURVE.

ALL DIMENSIONS WHICH ARE GIVEN IN SECTION AND ARE AFFECTED BY DEAD LOAD DEFLECTIONS ARE DIMENSIONS AT CENTER LINE OF BEARING UNLESS OTHERWISE NOTED ON PLANS. IN SETTING FORMS FOR STEEL BEAM BRIDGES AND PRESTRESSED CONCRETE GIRDER BRIDGES, ADJUSTMENTS SHALL BE MADE DUE TO THE DEAD LOAD DEFLECTIONS FOR THE ELEVATIONS SHOWN. WHERE BLOCKS ARE SHOWN OVER BEAMS FOR BUILDING UP TO THE SLAB, THE VERTICAL DIMENSIONS OF THE BLOCKS SHALL BE ADJUSTED BETWEEN BEARINGS TO COMPENSATE FOR DEAD LOAD DEFLECTIONS, VERTICAL CURVE ORDINATE, AND ACTUAL BEAM CAMBER. WHERE BOTTOM OF SLAB IS IN LINE WITH BOTTOM OF TOP FLANGES. DEPTH OF SLAB BETWEEN BEARINGS SHALL BE ADIUSTED TO COMPENSATE FOR DEAD LOAD DEFLECTION, VERTICAL CURVE ORDINATE, AND ACTUAL BEAM CAMBER.

IN SETTING FALSEWORK AND FORMS FOR REINFORCED CONCRETE SPANS, AN ALLOWANCE SHALL BE MADE FOR DEAD LOAD DEFLECTIONS, SETTLEMENT OF FALSEWORK, AND PERMANENT CAMBER WHICH SHALL BE PROVIDED FOR IN ADDITION TO THE ELEVATIONS SHOWN. AFTER REMOVAL OF THE FALSEWORK, THE FINISHED STRUCTURES SHALL CONFORM TO THE PROFILE AND ELEVATIONS SHOWN ON THE PLANS AND CONSTRUCTION ELEVATIONS FURNISHED BY THE ENGINEER.

REINFORCING STEEL:

ALL REINFORCING STEEL SHALL BE DEFORMED. DIMENSIONS RELATIVE TO PLACEMENT OF REINFORCING ARE TO CENTERS OF BARS UNLESS OTHERWISE INDICATED IN THE PLANS. DIMENSIONS ON BAR DETAILS ARE TO CENTERS OF BARS OR ARE OUT TO OUT AS INDICATED ON PLANS.

STRUCTURAL STEEL:

AT THE CONTRACTOR'S OPTION, HE MAY SUBSTITUTE $\frac{7}{8}$ " \oslash Shear studs for the $\frac{3}{4}$ " Ø STUDS SPECIFIED ON THE PLANS. THIS SUBSTITUTION SHALL BE MADE AT THE RATE OF 3 - $\frac{7}{8}$ " \oslash STUDS FOR 4 - $\frac{3}{4}$ " \oslash STUDS, AND STUD SPACING CHANGES SHALL BE MADE AS NECESSARY TO PROVIDE THE SAME EQUIVALENT NUMBER OF $\frac{7}{8}$ " \oslash STUDS ALONG THE BEAM AS SHOWN FOR $\frac{3}{4}$ " Ø STUDS BASED ON THE RATIO OF 3 - $\frac{7}{8}$ "Ø STUDS FOR 4 - $\frac{3}{4}$ " Ø STUDS. STUDS OF THE LENGTH SPECIFIED ON THE PLANS MUST BE PROVIDED. THE MAXIMUM SPACING SHALL BE 2'-0".

EXCEPT AT THE INTERIOR SUPPORTS OF CONTINUOUS BEAMS WHERE THE COVER PLATE IS IN CONTACT WITH BEARING PLATE, THE CONTRACTOR MAY, AT HIS OPTION, SUBSTITUTE FOR THE COVER PLATES DESIGNATED ON THE PLANS COVER PLATES OF THE EOUIVALENT AREA PROVIDED THESE PLATES ARE AT LEAST 5/16" IN THICKNESS AND DO NOT EXCEED A WIDTH EQUAL TO THE FLANGE WIDTH LESS 2" OR A THICKNESS EQUAL TO 2 TIMES THE FLANGE THICKNESS. THE SIZE OF FILLET WELDS SHALL CONFORM TO THE REOUIREMENTS OF THE CURRENT ANSI/AASHTO/AWS "BRIDGE WELDING CODE". ELECTROSLAG WELDING WILL NOT BE PERMITTED.

STANDARD NOTES

ALLOWANCE FOR DEAD LOAD DEFLECTION, SETTLEMENT, ETC. IN CASTING SUPERSTRUCTURES:

DETAILED DRAWINGS FOR FALSEWORK OR FORMS FOR BRIDGE SUPERSTRUCTURE AND ANY STRUCTURE OR PARTS OF A STRUCTURE AS NOTED ON THE PLANS SHALL BE SUBMITTED TO THE ENGINEER FOR APPROVAL BEFORE CONSTRUCTION OF THE FALSEWORK OR FORMS IS STARTED.

WIRE BAR SUPPORTS SHALL BE PROVIDED FOR REINFORCING STEEL WHERE INDICATED ON THE PLANS. WHEN BAR SUPPORT PIECES ARE PLACED IN CONTINUOUS LINES, THEY SHALL BE SO PLACED THAT THE ENDS OF THE SUPPORTING WIRES SHALL BE LAPPED TO LOCK LEGS ON ADJOINING PIECES.

WITH THE SOLE EXCEPTION OF EDGES AT SURFACES WHICH BEAR ON OTHER SURFACES, ALL SHARP EDGES AND ENDS OF SHAPES AND PLATES SHALL BE SLIGHTLY ROUNDED BY SUITABLE MEANS TO A RADIUS OF APPROXIMATELY $\frac{1}{16}$ " OR EQUIVALENT FLAT SURFACE AT A SUITABLE ANGLE PRIOR TO PAINTING, GALVANIZING, OR METALLIZING.

HANDRAILS AND POSTS:

METAL STANDARDS AND FACES OF THE CONCRETE END POSTS FOR THE METAL RAIL SHALL BE SET NORMAL TO THE GRADE OF THE CURB, UNLESS OTHERWISE SHOWN ON PLANS. THE METAL RAIL AND TOPS OF CONCRETE POSTS USED WITH THE ALUMINUM RAIL SHALL BE BUILT PARALLEL TO THE GRADE OF THE CURB.

METAL HANDRAILS SHALL BE IN ACCORDANCE WITH THE PLANS. RAILS SHALL BE AS MANUFACTURED FOR BRIDGE RAILING. CASTINGS SHALL BE OF A UNIFORM APPEARANCE. FINS AND OTHER DEFORMATIONS RESULTING FROM CASTING OR OTHERWISE SHALL BE REMOVED IN A MANNER SO THAT A UNIFORM COLORING OF THE COMPLETED CASTING SHALL BE OBTAINED. CASTINGS WITH DISCOLORATIONS OR OF NON-UNIFORM COLORING WILL NOT BE ACCEPTED. CERTIFIED MILL REPORTS ARE REQUIRED FOR METAL RAILS AND POSTS.

SPECIAL NOTES:

GENERALLY, IN CASE OF DISCREPANCY, THIS STANDARD SHEET OF NOTES SHALL GOVERN OVER THE SPECIFICATIONS, BUT THE REMAINDER OF THE PLANS SHALL GOVERN OVER NOTES HEREON, AND SPECIAL PROVISIONS SHALL GOVERN OVER ALL. SEE SPECIFICATIONS ARTICLE 105-4.

CONTENTS

730006

SF-

REFERENCE

<u>SHEET</u>	<i>NO</i> .
1	
2	
3	
4	
5-8	

DESCRIPTION TITLE SHEET

LEGEND (SOIL & ROCK)

SITE PLAN

PROFILE BORE LOGS

STATE OF NORTH CAROLINA

DEPARTMENT OF TRANSPORTATION **DIVISION OF HIGHWAYS GEOTECHNICAL ENGINEERING UNIT**

STRUCTURE SUBSURFACE INVESTIGATION

COUNTY PITT

PROJECT DESCRIPTION BRIDGE NUMBER 6 ON -L-(SR 1756) OVER COW SWAMP AT STA. 22+05

R004 **P**2. R PROJEC

STATE	STATE PROJECT REFERENCE NO.	SHEET NO.	TOTAL SHEETS
N.C	SF-730006	1	8

CAUTION NOTICE

THE SUBSURFACE INFORMATION AND THE SUBSURFACE INVESTIGATION ON WHICH IT IS BASED WERE MADE FOR THE PURPOSE OF STUDY, PLANNIKG AND DESIGN, AND NOT FOR CONSTRUCTION OR PAY PURPOSES. THE VARIOUS FIELD BORING LOGS, ROCK CORES AND SOLI TEST DATA AVAILABLE MAY BE REVEWED OR INSPECTED IN RALEICH BY CONTACTING THE N.C. DEPARTMENT OF TRANSPORTATION, GEOTECHNICAL ENGINEERING UNIT AT 1991 707-6860. THE SUBSIFACE PLANS AND REPORTS, FIELD BORING LOGS, ROCK CORES AND SOIL TEST DATA ARE NOT PART OF THE CONTRACT.

GENERAL SOIL AND ROCK STRATA DESCRIPTIONS AND INDICATED BOUNDARIES ARE BASED ON A GEOTECHNICAL INTERPRETATION OF ALL AVAILABLE SUBSURFACE DATA AND MAY NOT NECESSARILY REFLECT THE ACTUAL SUBSURFACE CONDITIONS BETWEEN BORINGS OR BETWEEN SAMPLED STRATA WITHIN THE BOREHOLE. THE LABORATORY SAMPLE DATA AND THE IN SITU UNPELACED TEST DATA CAN BE RELIED ON ONLY TO THE DECREE OF RELIABILITY INHERENT IN THE STANDARD TEST METHOD. THE OBSERVED WATER LEVELS OR SOLI MOISTURE CONDITIONS INDICATED IN THE SUBSURFACE INVESTIGATIONS ARE AS RECORDED AT THE TIME OF THE INVESTIGATION. THESE WATER LEVELS OR SOLI MOISTURE CONDITIONS MAY YARY CONSIDERABLY WITH TIME ACCORDING TO CLIMATIC CONDITIONS INCLUDING TEMPERATURES, PRECIPITATION AND WIND, AS WELL AS OTHER NON-CLIMATIC FACTORS.

THE BIDDER OR CONTRACTOR IS CALIFORED THAT DAMAGE AS NOWN ON THE SUBSURFACE PLANS ARE PRELIMINARY ONLY AND IN MANY CASES THE FINAL DESIGN DETAILS ARE DIFFERENT, FOR BIDDING AND CONSTRUCTION PURPOSES, REFER TO THE CONSTRUCTION PLANS AND DOCUMENTS FOR FINAL DESIGN INFORMATION ON THIS PROJECT. THE DEPARTMENT DOES NOT MARRANT OR GUARANTEE THE SUFFICIENCY OR ACCURACY OF THE INVESTIGATION MADE, NOR THE INTERPRETATIONS MADE, OR OPNION OF THE DEPARTMENT AS TO THE TYPE OF MATERIALS AND CONDITIONS TO BE ENCOUNTERED. THE BIDDER OR CONTRACTOR IS CAUTIONED TO PERFORM INDEPENDENT SUBSURFACE INVESTIGATIONS AND MAKE INTERPRETATIONS AS NECESSARY TO CONFIRM CONDITIONS ENCOUNTERED ON THE PROJECT. THE CONTRACTOR SHALL HAVE NO CLAIM FOR ADDITIONAL COMPENSATION OR FOR AN EXTENSION OF TIME FOR ANY REASON RESULTING FROM THE ACTUAL CONDITIONS ENCOUNTERED AT THE SITE DIFFERING FROM THOSE INDICATED IN THE SUBSURFACE INFORMATION.

NOTES:

- TES: THE INFORMATION CONTAINED HEREIN IS NOT IMPLIED OR GUARANTEED BY THE N.C. DEPARTMENT OF TRANSPORTATION AS ACCURATE NOR IS IT CONSIDERED PART OF THE PLANS, SPECIFICATIONS OR CONTRACT FOR THE PROJECT. BY HAVING REDUESTED THIS INFORMATION, THE CONTRACTOR SPECIFICALLY WAIVES ANY CLAIMS FOR INCREASED COMPENSATION OR SITEMENSION OF TIME BASED ON DIFFERENCES BETWEEN THE CONDITIONS INDICATED HEREIN AND THE ACTUAL CONDITIONS AT THE PROJECT SITE. 2.

PERSONNEL

D.N. ARGENBRIGHT

S.N. ZIMARINO

T.W. MILLER

C.M. WALKER

INVESTIGATED BY _____ BOTTOMS DRAWN BY _ T.W. MILLER CHECKED BY ______. D.N. ARGENBRIGHT SUBMITTED BY ______. ARGENBRIGHT DATE _______ 2023

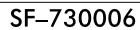


NORTH CAROLINA DEPARTMENT OF TRANSPORTATION DIVISION OF HIGHWAYS GEOTECHNICAL ENGINEERING UNIT SUBSURFACE INVESTIGATION

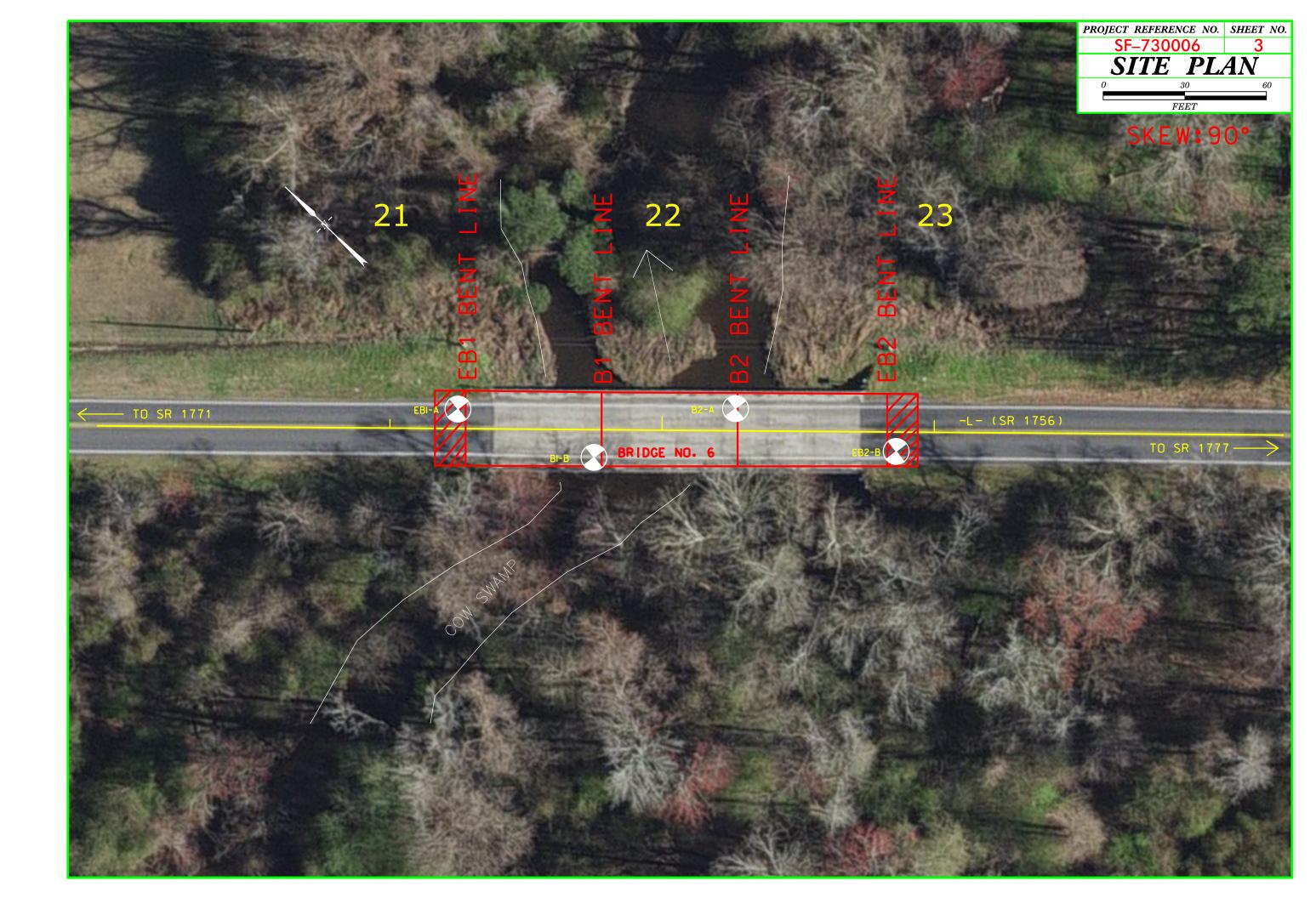
SOIL AND ROCK LEGEND, TERMS, SYMBOLS, AND ABBREVIATIONS

	SOIL DESCRIPTION IL IS CONSIDERED UNCONSOLIDATED, SEMI-CONSOLIDATED, OR WEATHERED EARTH MATERIALS PENETRATED WITH A CONTINUOUS FLIGHT POWER AUGER AND YIELD LESS THAN 100 BLOWS										G	RADATION		ROCK DESCRIPTION					
BE PENET ACCORDII IS B CONSISTE	RATED WITH NG TO THE ASED ON TH NCY, COLOR,	H A CONTINUC STANDARD PI HE AASHTO S TEXTURE,MO	DUS FLIGHT PON ENETRATION TE YSTEM. BASIC (ISTURE, AASHTO	VER AUGER AN ST (AASHTO 1 DESCRIPTIONS CLASSIFICA1	ND YIELD LESS 206, ASTM DI GENERALLY IN ION, AND OTHE	F THAN 100 586). SOII CLUDE TH R PERTINE	0 BLOWS P L CLASSIFI E FOLLOWI NT FACTOR	ER FOOT CATION NG: RS SUCH	<u>WELL GRADED</u> - INDICAT <u>UNIFORMLY GRADED</u> - IN <u>GAP-GRADED</u> - INDICATE	NDICATE	ES THAT SOIL IXTURE OF UN	PARTICLES ARE AL	LL APPROXIMA IZES OF TWO	ATELY THE SAME SIZE.	ROCK LINE I SPT REFUSAI BLOWS IN N REPRESENTED	INDICATI NL IS PE NON-COA	ES THE LEV ENETRATION STAL PLAIN ZONE OF W	EL AT WHICH NON-C BY A SPLIT SPOON MATERIAL, THE T EATHERED ROCK.	OASTAL PLAIN MATERIAL WOULD YIELD SAMPLER EQUAL TO OR LESS THAN Ø. RANSITION BETWEEN SOIL AND ROCK
AS V	5 MINERALO VERY STIFF.G	GICAL COMPO RAY, SILTY CLAY	SITION, ANGULAR , <i>MOIST WITH INT</i>	RITY, STRUCTU ERBEDDED FIN	RE, PLASTICITY E SAND LAYERS	,ETC. FO <i>HIGHLY PLA</i>	R EXAMPLE	•				F SOIL GRAINS IS D	ESIGNATED E	BY THE TERMS:	WEATHERED	IHLS H		Y DIVIDED AS FOLL	UWS: AIN MATERIAL THAT WOULD YIELD SP
			end and	AASHTO	CLASSIFI	CATION	l		ANGULAR, SUBAN			ICAL COMPOS			ROCK (WR)				FOOT IF TESTED.
GENERAL CLASS.		GRANULAR MATE ≤ 35% PASSING			MATERIALS	OR	GANIC MATER	IALS	MINERAL NA			Z, FELDSPAR, MICA, 1		ETC.		Ξ			GRAIN IGNEOUS AND METAMORPHIC RC T REFUSAL IF TESTED. ROCK TYPE IN
GROUP	A-1	A-3	A-2		A-6 A-7	A-1, A-2	A-4, A-5		ARE USED IN	1 DESCF		IN THEY ARE CONSIL	JERED OF SI	GNIFICANCE.	ROCK (CR)		<u></u>	GNEISS, GABBRO,	GRAIN METAMORPHIC AND NON-COASTA
0	A-1-a A-1-b	A-2-4	A-2-5 A-2-6 A-2		A-7-5, A-7-6	A-3	A-6, A-7		SI IG		CUMF OMPRESSIBLE	RESSIBILITY	LL < 31		NON-CRYSTAL ROCK (NCR)	LINE		SEDIMENTARY RO	CK THAT WOULD YEILD SPT REFUSAL UDES PHYLLITE, SLATE, SANDSTONE, ETI
SYMBOL	000000000000000000000000000000000000000			X					MODE	RATELY	Y COMPRESSIE		LL = 31 · LL > 50	- 50	COASTAL PLA			COASTAL PLAIN	SEDIMENTS CEMENTED INTO ROCK, BUT OCK TYPE INCLUDES LIMESTONE, SANDS
	50 MX					GRANULAR	SILT- CLAY	MUCK,	x, PERCENTAGE OF MATERIAL									SHELL BEDS, ETC	
	30 MX 50 MX 15 MX 25 MX		35 MX 35 MX 35 M	1X 36 MN 36 MI	N 36 MN 36 MN	SOILS	SOILS	PEAT	ORGANIC MATERIAL		GRANULAR SOILS	SILT - CLAY <u>SOILS</u>	OTHE	R MATERIAL	FRESH	BUCK	FRESH CRYS		INTS MAY SHOW SLIGHT STAINING. ROCK
MATERIAL PASSING #40 LL	-		41 MN 40 MX 41 M				S WITH LE OR		TRACE OF ORGANIC M LITTLE ORGANIC MAT MODERATELY ORGANIC HIGHLY ORGANIC	ATTER TER		3 - 5% 5 - 12% 12 - 20% > 20%	TRACE LITTLE SOME HIGHLY	1 - 10% 10 - 20% 20 - 35% 35% AND ABOVE	VERY SLIGHT	HAMME ROCK CRYST	ER IF CRYSTA GENERALLY F TALS ON A BF	ALLINE. FRESH, JOINTS STAINE ROKEN SPECIMEN FACI	D, SOME JOINTS MAY SHOW THIN CLAY C E SHINE BRIGHTLY. ROCK RINGS UNDER H
PI GROUP INDEX	6 MX Ø	NP 10 MX 1 0 0	10 MX 11 MN 11 M 4 MX		C 16 MX NO MX		RATE ITS OF	HIGHLY ORGANIC				UND WATER			SLIGHT				D AND DISCOLORATION EXTENDS INTO RO
USUAL TYPES S	STONE FRAGS. GRAVEL, AND SAND	FINE SIL	TY OR CLAYEY	SILTY	CLAYEY	ORC	ANIC	SOILS							(SLI.)	1 INCH CRYST	H. OPEN JOIN TALS ARE DUU	TS MAY CONTAIN CLA LL AND DISCOLORED.	Y, IN GRANITOID ROCKS SOME OCCASIONA CRYSTALLINE ROCKS RING UNDER HAMMEF DISCOLORATION AND WEATHERING EFFECT
GEN. RATING		5.005.0 5.07.70		5.15		Fair to					SATURATED ZONE, OF		(MOD.)	GRANI	TOID ROCKS.	MOST FELDSPARS ARE	DULL AND DISCOLORED, SOME SHOW CLA		
AS SUBGRADE		EXCELLENT TO			TO POOR	POOR	POOR	UNSUITABLE						FRESH ROCK.	R HAMMER BLUWS AND	SHOWS SIGNIFICANT LOSS OF STRENGTH			
	1		BGROUP IS ≤ LL			> LL - 30					MISCELL	ANEOUS SYMBI		MODERATELY SEVERE				OR STAINED. IN GRANITOID ROCKS, ALL F KAOLINIZATION. ROCK SHOWS SEVERE L	
			TNESS OR	RANGE OF	STANDARD	RAN	GE OF UNC	ONFINED			25.4	00F			(MOD. SEV.)	AND C	CAN BE EXCAN	VATED WITH A GEOLO	GIST'S PICK. ROCK GIVES "CLUNK" SOUND
PRIMARY S	SUIL ITPE CONSISTENCY PENETRATION RESISTENCE COMPRESSIVE S VLLY VERY LOOSE < 4								L ROADWAY EMB	DIP & DIP DIF OF ROCK STRU		SEVERE (SEV.)	ALL R REDUC	ROCK EXCEPT CED IN STREM	IGTH TO STRONG SOIL	OR STAINED. ROCK FABRIC CLEAR AND E . IN GRANITOID ROCKS ALL FELDSPARS / STRONG ROCK USUALLY REMAIN.			
GRANULA MATERIA	AR LUUSE 4 TO 10 MEDIUM DENSE 10 TO 30 N/A									- 131 ///					YIELD SPT N VALUES				
	ERIAL DENSE 30 TO 50 4-COHESIVE) VERY DENSE > 50 <						i	THAN ROADWA	ANKMENT 🗸	→ AUGER BORING → CORE BORING	•	VERY SEVERE (V SEV.)	BUT M REMAI	MASS IS EFFE	CTIVELY REDUCED TO ITE IS AN EXAMPLE	OR STAINED. ROCK FABRIC ELEMENTS AF O SOIL STATUS, WITH ONLY FRAGMENTS O OF ROCK WEATHERED TO A DEGREE THAT			
GENERAL SILT-CLA MATERIA (COHESIV	AY IL	MEDIU	OFT M STIFF TIFF STIFF	4 8	TO 4 TO 8 FO 15 TO 30		0.25 TO 0.5 TO 1 1 TO 2 2 TO 4	1.0 ?				T MONITORING W → PIEZOMETER INSTALLATION	ELL 🔶	_ TEST BORING WITH CORE — SPT N-VALUE	COMPLETE	ROCK SCATT	REDUCED TO	SOIL. ROCK FABRIC	MAIN. <u>IF TESTED, WOULD YIELD SPT N N</u> NOT DISCERNIBLE, OR DISCERNIBLE ONLY NAY BE PRESENT AS DIKES OR STRINGERS
		н	ARD	>	30		> 4	•	ALLUVIAL SOI	INSTALLATION		ROCK HARDNESS							
			TEXTURE	OR GRAII	N SIZE				<u> </u>			NDATION SYME			VERY HARD	HARP PICK. BREAKING OF HAND SPECIMEN			
U.S. STD. SIE OPENING (MM			4 10 4.76 2.00	40 0.42	60 200 0.25 0.075	270 0.053					NCLASSIFIED I NSUITABLE WA		ACCEPT	SIFIED EXCAVATION - ABLE, BUT NOT TO BE	HARD	ST'S PICK. ONLY WITH DIFFICULTY. HARD HAMMER B			
BOULDEF	а со	BBLE	GRAVEL	COARSE	FINE		SILT	CLAY	SHALLOW UNDERCUT			EXCAVATION - EGRADABLE ROCK		N THE TOP 3 FEET OF MENT OR BACKFILL			TACH HAND		
(BLDR.) GRAIN MM	(C	(OB.) 75	(GR.) 2.0	SAND (CSE. SD.)	SAND (F SD. 0.25		(SL.) 0.005	(CL.)	AR - AUGER REFUSAL		MED.	REVIATIONS	VST	- VANE SHEAR TEST	MODERATELY HARD	EXCAV		RD BLOW OF A GEOLO	GOUGES OR GROOVES TO 0.25 INCHES DE GIST'S PICK. HAND SPECIMENS CAN BE D
SIZE IN.		3							BT - BORING TERMINATED	٥		- MICACEOUS - MODERATELY		- WEATHERED UNIT WEIGHT	MEDIUM HARD				ES DEEP BY FIRM PRESSURE OF KNIFE () PEICES 1 INCH MAXIMUM SIZE BY HARD
	MOISTURE ERBERG LII	SCALE	STURE - I FIELD MO DESCRI	DISTURE	GUIDE FOR F		STURE DES	SCRIPTION	CPT - CONE PENETRATIO CSE COARSE DMT - DILATOMETER TES		ORG	NON PLASTIC - ORGANIC - PRESSUREMETER T		DRY UNIT WEIGHT	SOFT	CAN B		R GOUGED READILY B	/ KNIFE OR PICK. CAN BE EXCAVATED IN ZE BY MODERATE BLOWS OF A PICK POIN
			- SATURA (SAT.)	TED -	USUALLY LIC FROM BELOW				DPT - DYNAMIC PENETRA e - VOID RATIO F - FINE		EST SAP SD	- SAPROLITIC SAND, SANDY SILT, SILTY	S - E SS -	BULK SPLIT SPOON SHELBY TUBE	VERY	PIECE: CAN B	s can be br Be carved wi	ROKEN BY FINGER PRE	SSURE. XCAVATED READILY WITH POINT OF PICK.
LL PLASTIC RANGE	- LIQUID LIMIT)	 FOSS FOSSILIFEROUS FRAC FRACTURED, FRAC FRAGS FRAGMENTS 	TURES	TCR -	SLIGHTLY - TRICONE REFUSAL MOISTURE CONTENT		ROCK RECOMPACTED TRIAXIAL - CALIFORNIA BEARING	SOFT	FINGE			BY FINGER PRESSURE. CAN BE SCRATCH		
(PI) PLL.		C LIMIT							HI HIGHLY		v - v			RATIO	TERM			SPACING	TERM
	OM OPTIMUM MOISTURE MOIST - (M) SOLID; AT OR NEAR OPTIMUM MOISTURE					DRILL UNITS: ADVANCING TOOLS: HAMMER TYPE:								TYPE:	VERY WID WIDE MODERATE CLOSE		OSE	RE THAN 10 FEET 3 TO 10 FEET 1 TO 3 FEET 0.16 TO 1 FOOT	VERY THICKLY BEDDED THICKLY BEDDED 1 THINLY BEDDED 0. VERY THINLY BEDDED 0.0
						10ISTURE CME-55 G*CONTINUOUS FLIGHT AUGER CORE SIZE:							VERY CLC	JSE		6 THAN 0.16 FEET	THICKLY LAMINATED 0.00 THINLY LAMINATED <		
	PLASTICITY						CME-550		8" HOLLOW A	NUGERS	∐- [®] –	Ц-н					JRATION ENING OF MATERIAL BY CEMENTING.HE		
SLIG	PLASTIC GHTLY PLAS		PLAST	0-5 6-15	(PI)	<u>Di</u>	RY STRENC VERY LOW SLIGHT		VANE SHEAR TEST		TUNGCARBI		HAND TO		FOR SEDIMEN		NUCK 3, INDUP	RUBBING WIT	H FINGER FREES NUMEROUS GRAINS; W BY HAMMER DISINTEGRATES SAMPLE.
	ERATELY PI ILY PLASTI			16-25 6 OR MORE			MEDIUM HIGH		PORTABLE HOIST] <u>2 ⁵//6' STEEL TEETH</u>		ST HOLE DIGGER ND AUGER	MODEF	RATELY	INDURATED	BREAKS EAS	BE SEPARATED FROM SAMPLE WITH ST
												TUNGCARB.		JNDING ROD	INDUR	ATED			DIFFICULT TO SEPARATE WITH STEEL O BREAK WITH HAMMER.
			OR OR COLOR T, DARK, STREA								CORE BIT			NE SHEAR TEST	EXTRE	EMELY !	INDURATED		ER BLOWS REQUIRED TO BREAK SAMPLE AKS ACROSS GRAINS.

PROJECT REFERENCE NO.



TERMS AND DEFINITIONS ED. AN INFERRED) SPT REFUSAL. 1 FOOT PER 60 IS OFTEN ALLUVIUM (ALLUV.) - SOILS THAT HAVE BEEN TRANSPORTED BY WATER. ADUIFER - A WATER BEARING FORMATION OR STRATA. ARENACEOUS - APPLIED TO ROCKS THAT HAVE BEEN DERIVED FROM SAND OR THAT CONTAIN SAND. ARGILLACEOUS - APPLIED TO ALL ROCKS OR SUBSTANCES COMPOSED OF CLAY MINERALS, OR HAVING A NOTABLE PROPORTION OF CLAY IN THEIR COMPOSITION, SUCH AS SHALE, SLATE, ETC. N VALUES > ARTESIAN - GROUND WATER THAT IS UNDER SUFFICIENT PRESSURE TO RISE ABOVE THE LEVEL AT IS ENCOUNTERED, BUT WHICH DOES NOT NECESSARILY RISE TO OR ABOVE THE GROUND CK THAT SURFACE. CLUDES GRANITE. CALCAREOUS (CALC.) - SOILS THAT CONTAIN APPRECIABLE AMOUNTS OF CALCIUM CARBONATE. AL PLAIN IF TESTED. COLLUVIUM - ROCK FRAGMENTS MIXED WITH SOIL DEPOSITED BY GRAVITY ON SLOPE OR AT BOTTOM OF SLOPE. MAY NOT YIELD STONE, CEMENTED CORE RECOVERY (REC.) - TOTAL LENGTH OF ALL MATERIAL RECOVERED IN THE CORE BARREL DIVIDED BY TOTAL LENGTH OF CORE RUN AND EXPRESSED AS A PERCENTAGE. $\underline{\text{DIKE}}$ - A TABULAR BODY OF IGNEOUS ROCK THAT CUTS ACROSS THE STRUCTURE OF ADJACENT ROCKS OR CUTS MASSIVE ROCK. RINGS UNDER DIP - THE ANGLE AT WHICH A STRATUM OR ANY PLANAR FEATURE IS INCLINED FROM THE HORIZONTAL . NATINGS IF OPEN. DIP DIRECTION (DIP AZIMUTH) - THE DIRECTION OR BEARING OF THE HORIZONTAL TRACE OF THE LINE OF DIP, MEASURED CLOCKWISE FROM NORTH. AMMER BLOWS IF FAULT - A FRACTURE OR FRACTURE ZONE ALONG WHICH THERE HAS BEEN DISPLACEMENT OF THE ІСК ИР ТО SIDES RELATIVE TO ONE ANOTHER PARALLEL TO THE FRACTURE. FELDSPAR FISSILE - A PROPERTY OF SPLITTING ALONG CLOSELY SPACED PARALLEL PLANES. BLOWS. $\underline{\mathsf{FLOAT}}$ - ROCK FRAGMENTS ON SURFACE NEAR THEIR ORIGINAL POSITION AND DISLODGED FROM PARENT MATERIAL. S. IN AY. ROCK HAS AS COMPARED FLOOD PLAIN (FP) - LAND BORDERING A STREAM, BUILT OF SEDIMENTS DEPOSITED BY THE STREAM. FORMATION (FM.) - A MAPPABLE GEOLOGIC UNIT THAT CAN BE RECOGNIZED AND TRACED IN THE FIELD. ELDSPARS DULL OSS OF STRENGTH WHEN STRUCK. JOINT - FRACTURE IN ROCK ALONG WHICH NO APPRECIABLE MOVEMENT HAS OCCURRED. LEDGE - A SHELF-LIKE RIDGE OR PROJECTION OF ROCK WHOSE THICKNESS IS SMALL COMPARED TO ITS LATERAL EXTENT. VIDENT BUT ARE KAOLINIZED LENS - A BODY OF SOIL OR ROCK THAT THINS OUT IN ONE OR MORE DIRECTIONS. MOTTLED (MOT.) - IRREGULARLY MARKED WITH SPOTS OF DIFFERENT COLORS. MOTTLING IN SOILS USUALLY INDICATES POOR AERATION AND LACK OF GOOD DRAINAGE. RE DISCERNIBLE PERCHED WATER - WATER MAINTAINED ABOVE THE NORMAL GROUND WATER LEVEL BY THE PRESENCE STRONG ROCK T ONLY MINOR VALUES < 100 BPF OF AN INTERVENING IMPERVIOUS STRATUM. RESIDUAL (RES.) SOIL - SOIL FORMED IN PLACE BY THE WEATHERING OF ROCK. ROCK OUALITY DESIGNATION (ROD) - A MEASURE OF ROCK QUALITY DESCRIBED BY TOTAL LENGTH OF ROCK SECMENTS EQUAL TO OR CREATER THAN 4 INCHES DIVIDED BY THE TOTAL LENGTH OF CORE IN SMALL AND SAPROLITE IS RUN AND EXPRESSED AS A PERCENTAGE. SAPROLITE (SAP.) - RESIDUAL SOIL THAT RETAINS THE RELIC STRUCTURE OR FABRIC OF THE PARENT S REQUIRES SILL - AN INTRUSIVE BODY OF IGNEOUS ROCK OF APPROXIMATELY UNIFORM THICKNESS AND RELATIVELY THIN COMPARED WITH ITS LATERAL EXTENT, THAT HAS BEEN EMPLACED PARALLEL TO LOWS REQUIRED THE BEDDING OR SCHISTOSITY OF THE INTRUDED ROCKS. <u>SLICKENSIDE</u> - POLISHED AND STRIATED SURFACE THAT RESULTS FROM FRICTION ALONG A FAULT OR SLIP PLANE. EEP CAN BE ETACHED STANDARD PENETRATION TEST (PENETRATION RESISTANCE) (SPT) - NUMBER OF BLOWS (N OR BPF) OF A 140 LB. HAMMER FALLING 30 INCHES REQUIRED TO PRODUCE A PENETRATION OF 1 FOOT INTO SOIL R PICK POINT WITH A 2 INCH OUTSIDE DIAMETER SPLIT SPOON SAMPLER. SPT REFUSAL IS PENETRATION EQUAL BLOWS OF THE TO OR LESS THAN 0.1 FOOT PER 60 BLOWS. $\underline{STRATA CORE RECOVERY (SREC.)}$ - TOTAL LENGTH OF STRATA MATERIAL RECOVERED DIVIDED BY TOTAL LENGTH OF STRATUM AND EXPRESSED AS A PERCENTAGE. FRAGMENTS IT. SMALL. THIN STRATA ROCK QUALITY DESIGNATION (SROD) - A MEASURE OF ROCK QUALITY DESCRIBED BY TOTAL LENGTH OF ROCK SEGMENTS WITHIN A STRATUM EQUAL TO OR GREATER THAN 4 INCHES DIVIDED BY THE TOTAL LENGTH OF STRATA AND EXPRESSED AS A PERCENTAGE. PIECES 1 INCH ED READILY BY TOPSOIL (TS.) - SURFACE SOILS USUALLY CONTAINING ORGANIC MATTER. BENCH MARK: BM-I THICKNESS N: 654031.3897 E: 2527951.7563 4 FEET ELEVATION: 24.78 FEET .5 - 4 FEET 16 - 1.5 FEET NOTES: 3 - Ø.16 FEET 08 - 0.03 FEET 0.008 FEET AT, PRESSURE, ETC. TEEL PROBE: PROBE;



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0	VE	RY SOFT TO) SOFT	GRAY	SILT	Y CLAY	1	©- 	 /ITH	3	SHELL		FRAGME	NTS 3+	WET (Y	OF
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WBS	BP2.F	R004.1	N BRIDGE NUMBER 6 ON -L- (SR 1756) OVER COW SWAMP							1		WBS	BP2.R	004.1			TIF	P SF-7300	006	COUNT	FY F						
				DGE N				6) OVER														GE NI		R 6 ON -L-	, ,) OVER (
		. EB1-/				TATION			_	FSET					NMENT -L-	-1	/A		NG NO.				_	ATION 2			OF
		EV. 25								ORTHING			<u> </u>		FING 2,527,944		.9		AR ELE								NC
				E GFG		XME-45C 909							UM	ud Rotary		IER TYPE Automatic	;					GFC	_	ME-45C 90%			
		Valker, C				TART DAT				omp. Da	SAMP.			SUR	FACE WATER DEPTH N	Ά										23 PER FOC	
ELEV (ft)	ELEV (ft)	DEPTH (ft)	0.5ft	W CO	0.5ft	0	25	50 PER FO	75	100	NO.	17	0		SOIL AND ROCK DES			ELEV (ft)	DRIVE ELEV (ft)	DEPTH (ft)	0.5ft	W COL		0		50 SER FUC	7 <u>5</u>
	(11)		0.010	0.010	0.010						110.		I G	ELEV. (ft)	DEPTH	1 (ft)		(11)		0.011	0.011	0.011		<u> </u>	<u> </u>	
20																		50							Mat	ch Line	
30		‡												-				-50	+		+		+	·	· · · · ·		·
		‡												-				-	-52.3 -	- 77.7 -	2	3	4				:
25	24.6 -	- 0.8				-+								- 25.4 - 24.6	GROUND SURF		0.0 0.8	-55	4						<u> </u>		÷
		‡	10	5	4	: ,∳ 9∶:	· · · · ·	• • • •	.	· · · ·				-	PAVEMENT TAN SILTY SAND WITH G				-57.3 -	82.7		0	_				:
20	21.4	<u>+ 4.0</u> +	2	1	2			- -						-	TO SATURAT	ED		-60	+		4	3	5	. •8			:
_20	-	ŧ				<mark></mark>								<u>19.4</u>			6.0	-00	+	-					+ · · · ·		
ŀ	17.7	<u>+ 7.7</u> +	1	1	1			• • • •	.	· · · ·				-	GRAY SILTY SAND W FRAGMENTS, SATU			-	-62.3 -	87.7 -	4	3	6	· · · · ·			:
15	-	ŧ												-	,			-65	+	-					· · · ·		·
	12.7	+ + 12.7_	woн	1		::::		• • • •	.	· · · · ·				-					-67.3	92.7			13				:
10		ŧ		1	2	• 3								-				-70	+		4	9	13	1117	22		:
		ŧ				 							N	<u>9.4</u>		<u> </u>	<u>16.0</u>	-10	+	-					· · · ·		
	7.7	<u>+ 17.7</u> +	1	1	1			· · · ·	.	· · · · ·				-	GRAY SILTY CLAY W FRAGMENTS, WET (Y				-72.3 -	<u>97.7</u>	4	4	6	· · · · ·			:
5	-	ŧ												-	FORMATIÓN	1)		-75	4	-				- 1	+		÷
	2.7	22.7	3	1	1			• • • •	.	· · · · ·				-				-	-77.3	102.7	5	5	8				:
0		Ŧ		'	'	Q ² · · · ·								-				-80	1			J	°				
	-2.3	27.7				1								-0.6	GRAY SILTY SAND WITH	SHELLS AND	<u>26.0</u>		-82.3	-					· · · ·		
	-2.3	<u>+ 2/./</u> +	3	2	2	• • • • • • • • • • • • • • • • • • •			.	· · · · ·				-	PHOSPHATE, SATU (YORKTOWN FORM				-82.3 -	· 107.7	6	7	11		8		
-5	-	Ŧ																-85	-	-							<u> </u>
	-7.3	32.7	7	10	5	I N. I		• • • •	.	· · · · ·				<u>6.6</u> 7.0	GRAY AND GREEN SILT		<u>32.0</u> 32.4	-	-87.3	112.7	29	37	63/0.4				·
-10		Ŧ	'											-9.1 -9.5	PHOSPHATE, SHELL FRA VERY HARD SANDSTO	NE LAYERS,	34.5 34.9	-90	1	-	20	01	00/0.4				:
	-123	I 37 7											1	-10.8 -11.3	SATURATED (PEEDEE F		36.2 36.7		_923	- . 117 7							•
	-12.0	<u>+ 37.7</u>	34	37	7			44						-13.9			39.3		-92.0		22	29	48				¢
-15	-	Ŧ					+/-							14.5		3	39.9			-							
	-17.3	42.7	4	3	30		. . / .							-					1								
-20	_	Ŧ					- 33		-					-18.8 -19.7			14.2 15.1		1	-							
	-22.3	47.7							· · ·	 . 				-22.0		2	17.4		+								
		ŧ	10	10	90/0.3					100/0.8	•			24.4		2	19.8		+								
-25	-	ŧ												-					+	-							
	-27.3	<u> </u>	14	18	32			-		· · · · ·				-					1								
	-	ŧ												-29.1		ŧ	54.5 55.6		1	-							
-30 -35	-32.3	+ + 57.7				· · · ·				· · · ·				-30.7 -31.0		t t	56.1 56.4		1								
		ŧ	7	5	5	• 10								-33.6	DARK GRAY SILTY C		59.0		+								
-35	-	ŧ												-	SANDSTONE LAYERS, W FORMATION	VET (PEEDEE			+	-							
-40	-37.3	<u> </u>	53	100/0.1	1	::::::	:+÷÷÷			<u> </u>				37.3 38.8	TORMATION	· · · ·	62.7 64.2		1								
-40	-	‡				· 1===								-30.0		Ċ	2.7.2		+	-							
	-42.3	+ - 67.7				::::		· · · ·	•••	 				-					+	.							
45		‡	18	4	8	12		.	.					-					+	.							
-45	-	‡												-					+	-							
,	-47.3	+ 72.7 +	3	4	7	 11 ·								-					+								
, ,																											

SHEET 5 OF 8

' PITT					GEOLOGIST Mil	ler, T. W.	-	
W SW	AMP						GROUN	D WTR (ft)
OFFSE	ET 7	7 ft LT			ALIGNMENT -L-		0 HR.	N/A
NORTH	HING	654,02	24		EASTING 2,527,	944	24 HR.	1.9
		DRILLM	ETHOD) ML			ER TYPE	Automatic
COMP	DA.	L TE 04/0			SURFACE WATE		2	
		SAMP.	////	L			`	
75	100	NO.	моі	O G	SOIL AN	ND ROCK DESC	CRIPTION	
1		110.		G				
		+						
					GRAY SA	NDY SILT, WE	T (PEEDE)	E CIR
	::				•			
· · ·						NDY CLAY, WE		E 81.0
						FORMATION)	
	•••							
	•••				•			
					- <u>-65.6</u>			91.0
	•••				GRAY MICA	CEOUS SILT, V FORMATION	VET (PEE)	DEE
					-			
	•••							
					-			
					- ·			
					- 			106.0
	•••				GREEN AN			TH
	•••				- PRUSPRA	TE, SATURATE FORMATION	ED (PEEDI)	
					-			
	•••				•			
. 100	0/0.9				•			
· · · ·	<u>/</u> .				_			
					- -93 8			119.2
		-				ninated at Eleva Y DENSE SILTY	tion -93.8 1	
				F	- VER	Y DENSE SILTY	' SAND	
				F	•			
					-			
					-			
				F				
				F				
					-			
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				F				
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					-			
				F	-			

site Bori Coli Drill	ng no. _ar ele .rig/haw	IPTION B1-B	BRID	DGE N		IP SF-7300 R 6 ON -L-		COUNT				0	EOLOGIST Argenbright, D. N			S BP2.R0	004.1		T	IP SF-7300	06	COUNTY	ΥI
Bori Coli Drill Drill Elev	ng no. _ar ele .rig/haw	B1-B	TION BRIDGE NU			MBER 6 ON -L- (SR 1756) OVER CO						GROUND WTR (f		(ft) WBS BP2.R004.1 SITE DESCRIPTION BRIDG									
Coli Drill Drill Elev	.AR ELE .RIG/HAN	NO. B1-B STATION 21 RELEV. 16.6 ft TOTAL DEPT			, ,	OVER CC						GROUND WTR (ft	·			BRIDG		R 6 ON -L- (SR 1756)	OVER CC	<u>w</u>		
drill Dril	. RIG/HAN	-V 16			S	TATION 2	1+75		OFFSET	10 ft RT		/	LIGNMENT -L-	0 HR. N//	BOF	ring no.	B1-B		S	TATION 21	+75		OF
DRIL Elev						OTAL DEPT		ft	NORTHING				ASTING 2,527,966	24 HR. N//		LAR ELE				OTAL DEPT		ť	NC
ELEV	ER Walker, C. M. START DAT DRIVE ELEV DEPTH BLOW COUNT Image: Count Count			E GFO	20075 (CME-45C 90%	511/21/2022			DRILL M	IETHOD	Mud R	tary HAMIV	NER TYPE Automatic	DRIL	L RIG/HAM	MER EFI	F./DATE	GF00075 (CME-45C 90% 1	11/21/2022		
ELEV (ft)					S	TART DATE	E 03/30/2	3	COMP. DA	TE 03/3	30/23	5	URFACE WATER DEPTH 2.	4ft	DRI	L LER Wa			S	TART DATE	03/30/23	3	c
(ft)	DRIVE ELEV	DEPTH			-		BLOWS F	PER FOOT	-	SAMP.		LO	SOIL AND ROCK DES	CRIPTION	ELE\				COUNT		BLOWS P	ER FOOT	
	(ft)	(ft)	0.5ft	0.5ft	0.5ft	0 2	25 5	50	75 100	NO.	моі		EV. (ft)	DEPTH	(ft)	(ft)	(ft)	0.5ft (0.5ft 0.5ft	0 2	5 5	0	75
20		L I										L	WATER SURFACE (03/30/23)	-60	↓		+			Match	n Line	
	-	+														-62.5 +	79.1			: : : : :	· · · · ·	· · · · ·	
15	16.6 -	- 0.0	WOR	WOR	WOR	0						- 16	ALLUVIAL		. <u>0</u> -65	‡		4	4 5	9.	· · · ·	· · · ·	
15		ŧ.											TAN SAND WITH WOOD SATURATEI		-05	1 1	-						+
	-	Ł										Ē				-67.5	84.1	5	43 27				
10	11.3	5.3	4	2	2							- 10	3	6	3 -70	Ŧ		5	43 21				70
	-	F				 						8.0		ð		1 7	.						T
	7.5 -	9.1	wон	2	1								GRAY SILTY CLAY W			-72.5 +	<u>89.1</u>	4	4 6			· · · ·	
5	-	Ł										N.	GRAY SILTY CLAY W FRAGMENTS, WET (Y FORMATION		-75					· •			
	-											St		•)		1							
	2.5 -	<u>+ 14.1</u>	3	1	1							N-				-77.5 +	· 94.1	4	5 6				
0	-	ŧ.				$\left \begin{array}{c} 1 \\ 1 \\ 1 \\ 1 \end{array} \right $	+ • • • •								0 -80		-			\			_
	-2.5 -	+ - 19.1											GRAY SILTY SAND, SA (YORKTOWN FORM			-82.5 +	99.1			::\:	· · · · ·	· · · ·	
-	-	1	2	3	5							-	Υ.	,				5	7 12	19			
-5	-	Ł					<u> </u>								-85	- +	-					~~	+
	-7.5 -	24.1	04	10	00/0 4							-	_			-87.5	104.1	05	40 57				·
-10	-	ŧ	21	10	60/0.1		\ <u></u>		60/0.1	·			GRAY AND GREEN SILT		¹ ⁶ -90	1	-	25	43 57				
-10	-	ŧ.					İ					-1		NDSTONE 27 D (PEEDEE 28	6	1 +	-						+
	-12.5 -	29.1	4	6	100	::::	[FORMATION	N) 30		-92.5	· 109.1	18	27 38		· · · ·		1
-15	-	Ł							100/0.5	'		-1		31	-95		_		-			965	
	-	F										F				I							
	-17.5 -	34.1	2	3	24	$\left \left \begin{array}{c} \cdot \cdot \cdot \cdot \cdot \\ \cdot \cdot \cdot \end{array} \right \right $	↓ · · · · ·					Ē				-97.5 +	114.1	16	26 27			.∕ ●53	
-20	-	ŧ.					Į						0	37	5	-	-						
	- -22.5 -	- 39 1										-2		37		1 1	.						
	-	-	7	9	60/0.1				60/0.1	,		-2	.5	40 40	1	1							
-25	-	F													-		-						
	-27.5 -	44.1	11	15	14			• • • •								Ŧ	.						
-30	-	F	11	15	14		• 29	: : : :				-				‡							
	-	ŧ					12227	1220					.7 .2	47 47	8	‡	-						
	-32.5 -	49.1	25	7	8			+	╡╤╤╤╒┥			-3	2.6	48 49	5 2	‡							
-35	-	t				P 15							- DARK GRAY SILTY CLAY,		4	1							
	-					$ \cdot \cdot \cdot \cdot \cdot$						N	FORMATION	4)		 							
	-37.5 -	54.1	11	11	12	1 ::: \	23					N				7	.						
-40	_	ŧ				/	+ • • • •					X-				‡	-						
	-42.5 -	- 59.1				::/:						1				‡							
	-		4	4	5] :••::						2				<u>†</u>							
-45		+					+	+			[\mathbf{Y}				-	-						
	- 47.5 -	- 64.1				_	K					V				‡	.						
_50	-	‡	7	23	12		€35					N				‡							
-50		‡							<u> </u>							1	-						
	-52.5 -	69.1	2	100/0.4				<u> </u>	<u> </u>			-5		69	6	+							
-55	-	+	_	00,0.2	1		+		100/0.4			-5				Ŧ	-						
	-	F											GRAY SANDY CLAY, W	ET (PEEDEE	<u> </u>	‡	-						
	-57.5 -	74.1	3	4	4		· · · · ·		· · · · ·				FORMATION	1)		‡	-						
-60	-					.● ⁸	· · · ·									<u> </u>							

SHEET 6 OF 8

PITT		GEOLOGIST Argenbrigh	nt, D. N.	
W SWAMP			GR	OUND WTR (ft)
OFFSET 10 ft RT		ALIGNMENT -L-	IR. N/A	
NORTHING 653,975		EASTING 2,527,966	24 H	IR. N/A
DRILL METH	IOD Muc	I Rotary	HAMMER TY	PE Automatic
COMP. DATE 03/30/2	3	SURFACE WATER DEPT	H 2.4ft	
SAMP.				
75 100 NO. M	OI G	SOIL AND ROC		
: : : : : : : [[-		GRAY SANDY CL FORMATIO	AY, WET (PE N) (continued)	EEDEE)
<u>+ • • • • •</u>		GRAY AND GREEN	SILT WET?	PEEDEE <u>82.0</u>
	₩E	FORM	IATION)	
	E			
	E			
	F			
+				
	F			
		80.4		07.0
· · · ·		GRAY AND GRE		
		SATURATED (PEI	EDEE FORM	41 ION)
<u> </u>	-			
100	-			
<u> </u>	-			
		-99.0 Boring Terminated a	at Elevation	115.6
			E SILTY SAN	
	E			
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	I E			
	F			
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	F			
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14/2017					D 05		-					0501 5										N of		0.01	
	BP2.R004.				IP SF-730							GEOLOG	GIST Zimarino, S. N				BP2.					P SF-73			
			DGEN			•	UVER C								GROUND WTR (ft)				BRIL	JGE N			- (SR 175	o) UVER	
	NG NO. B2-				TATION 2			OFFSET				ALIGNM			0 HR. N/A			. B2-A				ATION		4.6	0
	.AR ELEV. RIG/HAMMER							NORTHIN		950 Method			G 2,528,015		24 HR. N/A RTYPE Automatic			.EV. 15					PTH 114. %11/21/202		N
																				- 01					C
	LER Walker	-	ow co		TART DAT		PER FOO				1 L T	SURFAC	E WATER DEPTH	3.51	t			DEPTH		W CO			BLOW	S PER FO	
ELEV (ft)	ELEV (ft)	· · ·		0.5ft	0	25	50	75 100		MOI	0	ELEV. (ft)	SOIL AND ROCK D	DESCI	RIPTION DEPTH (ft	ELEV (ft)	ELEV (ft)	(ft)		0.5ft		0	25	50	75
i	(11)					1		1				ELEV. (π)			<u>DEPTH (π</u>		(11)						1		
20																-60							Ма	atch Line	
_20	_												WATER SURFAC	CE (03	8/29/23)	-00	+	+			+	- <u></u> ,			
1	‡																-62.4	<u>+ 77.9</u> +	3	7	15	: : : `	22		
15	15.6 + 0.0	wor	1 WOF	1	<u> </u>					-		15.6	GROUND SU ALLUVI		CE 0.0	-65		ŧ				· · · ·			
	‡				📉 : : :			· · · · · ·				12.6	BROWN CLAYEY		3.0		-67.4	+ 82.9		-		· · ·/ · · /			
10	11.4 4.2	3	4	6				· · · · · ·			0 0 0 0 0 0 0 0 0 0 0 0		GRAY SAND, SA	ATUR	ATED	-70		‡	4	6	6	· •12			
10	+										0 0 0 0 0 0 0 0 0 0 0 0	8.6			7.0			‡				<u>; ; ;</u>			+
-	<u>7.7 + 7.9</u> +	wor	I WOF	1				 			N		GRAY SILTY CLAY	' WITI	T]	-72.4	<u>+ 87.9</u> +	3	5	5		· · · · ·		· •
5	‡				T ['] ····				41		N-		FRAGMENTS, WET FORMAT	(YOF	RKTOWN	-75	.	‡				- T'-	.		
	2.7 + 12.9							· · · · · ·						1011)			-77.4	+ 92.9				: 1:		· · · ·	
_	+	2	2	2] •4 : : :			:										ţ	4	5	7	• • 12			
0	+				+							- <u>0.5</u>	GRAY AND BLUE	SILT	<u> / SAND 16.0</u>	-80		ŧ							
-	-2.4 + 17.9	2	3	3								SA	TURATED (YORKTC				-82.4	<u>+ 97.9</u> +	4	6	8			· · · ·	
-5	1						<u> </u>	·				- <u>4.7</u>	AN, GREEN, AND GR			-85		ŧ							
	-7.4 + 22.9				<u> </u>		+		4				WITH VERY HARD	CALC	AREOUS		-87.4	+ 102.9					· · · · ·	:	
		2	2	2] ₩4:::							I	SANDSTONE LA PHOSPHATE, SATUF		D (PEEDEE			ţ	16	26	41				6 7
-10	+				+								FORMAT	ION)		-90		ŧ							$\dot{\cdot}$
	-12.4 + 27.9	2	2	3													-92.4	+ 107.9	27	30	46				: '\
-15	Ŧ															-95		ŧ							
	-17.4 + 32.9																-97 /	+ 112.9							/
		7	14	17	1	•31												+	14	26	43				69
-20	+																-	ŧ							
	-22.4 + 37.9	8	9	65				· · · · ·										ŧ							
-25	Ŧ							· ¶ ⁷⁴ · · · ·			Ŀ							Ŧ							
1	-27.4 + 42.9							: : : : :										ŧ							
	<u></u>	14	20	52				• 72										Ŧ							
-30	Ŧ										F						.	Ŧ							
-00	-32.4 47.9	32	11	11														Ŧ							
-35	Ŧ											-35.5			51.0		.	1							
-35	-37.4 + 52.9				: : : i						Š	<u> </u>	GRAY SANDY CLAY, FORMAT		(PEEDEE	1		Ŧ							
		9	9	11	1 :::∔	20							FURIVIAT					ŧ							
-40	Ŧ																.	Ŧ							
	42.4 _ 57.9	27	8	12														Ŧ							
<u>-40</u> _45	Ŧ					20												Ŧ							
						<u> </u>]								.	Ŧ							
-50	<u>-47.4 + 62.9</u> +	5	11	15	1	26		-										Ŧ							
	Ŧ					4											.	Ŧ							
	-52.4 + 67.9	3	4	8	::/:													Ŧ							
-55	Ŧ		4	Ô	• 12													Ŧ							
-55									11		Þ	- <u>55.5</u>	GRAY CLAYEY SILT,		(PEEDEE	1	.	Ŧ							
	<u>-57.4 + 72.9</u> +	2	4	6									FORMAT	ION)				Ŧ							
-60	<u>t</u>				<u> - X -</u>													<u>t</u>							

SHEET 7 OF 8

' PITT		GEOLOGIST Zimarino,	S. N.		
W SWAMP				GROUNI	D WTR (ft)
OFFSET 8 ft LT		ALIGNMENT -L-		0 HR.	N/A
NORTHING 653,9	50	EASTING 2,528,015		24 HR.	N/A
DRILL	IETHOD Mud	l Rotary	HAMME	RTYPE	Automatic
COMP. DATE 03/2	29/23	SURFACE WATER DEPT	FH 3.5	ft	
75 100 NO.		SOIL AND ROC	K DESC	RIPTION	
	0	SOIL AND ROC SOIL AND ROC GRAY CLAYEY S FORMATIO FORMATIO SATURATED (PE Boring Terminated VERY DENS	GRAYS EDEE F	T (PEEDE inued)	iD, <u>96.0</u> N)

COLLAR ELV. 25.1 TOTAL DEFINI 14.3.1 NORTHNGE 65.807 LABTING 2.280,045 14.4.8 0.7 DRALEGNAMERY EF ADAR GRALEGNAMERY EF ADAR GRAL	WPC	802 000	1			T 17) CE 700	0006				.00			CE					WPC	י רחק	2004 4			-		E 700	006	COUNT		
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SHEET 8 OF 8

PITT				GEOLOG	ST Miller, T. \	N.		
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NORTHING	653,89	97		EASTING	2,528,045		24 HR.	6.7
	DRILL M	ethod) Mu	dRotary		Hamme	RTYPE	Automatic
COMP. DAT	E 04/0	04/23		SURFACE	WATER DEP	TH N/A	4	
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				G	RAY AND GREE HOSPHATE, SA			ITH
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